# ADDENDUM No. 3

# TO THE

# PURE WATER MONTEREY/GROUNDWATER REPLENISHMENT PROJECT ENVIRONMENTAL IMPACT REPORT

STATE CLEARINGHOUSE NO. 2013051094

# FOR THE ADVANCED WATER TREATMENT FACILITY EXPANDED CAPACITY PROJECT MODIFICATIONS

October 24, 2017

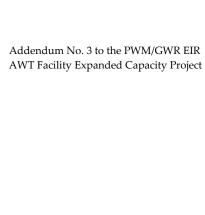
# Prepared for

Monterey Regional Water Pollution Control Agency (Monterey One Water)

Prepared by Denise Duffy and Associates







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# TABLE OF CONTENTS

I.	Introduction	1
II.	Project Location/Facilities	2
III.	Project Description Within Prior Environmental Documents	3
IV.	Proposed PWM/GWR Project Modifications	12
V.	Supplemental or Subsequent EIR Not Required	24
VI.		
VII	Environmental Analysis	26
	I. Report Preparation and References	
	EIR Preparers, Agencies and Persons Consulted	
Lis	ST OF FIGURES	
1.	PWM/GWR Overview Map	6
2.	Shared Facilities for PWM/GWR and RUWAP	7
3.	Advanced Water Treatment Facility at Existing Regional Treatment Plant	8
4.	RUWAP Facilities and Pipeline Alignment	9
5.	A. PWM/GWR Advanced Water Treatment Facility Site Plan	10
5.	B. Future Brine Mixing Facility Site Plan	11
6.	Diagram of Existing MRWPCA RTP and Future AWT Facility Treatment Process	15
7.	Eliminated RUWAP Facilities	16
8.	Blackhorse Reservoir Tank Location	22
9.	Blackhorse Reservoir Tank Site Plan	23
10.	RUWAP Cultural Resources Consultation Map	35
Lis	ST OF TABLES	
1.	Updated AWT Facilities Design Summary	17
2.	Updated AWT Facility Process Design Flow Assumptions	17
3.	Updated Overview of Proposed Project Electricity Demand	20
S-2	Summary of Cumulative Impacts and Mitigation Measures	53

# LIST OF APPENDICES

- A. Table of Project Description Text Changes to the PWM/GWR EIR
- B. Adopted PWM/GWR and RUWAP Mitigation Monitoring and Reporting Program
  - 1. PWM/GWR MMRP

# 2. RUWAP MMRP

- C. Trussell Tech September 2017 Ocean Plan Compliance Assessment for the PWM/GWR Project
- D. Trussell Tech September 2017 Comparison of Dilution Results
- E. Trussell Tech September 2017 Revised Ocean Plan Compliance Assessment for MPWSP and Project Variant
- F. 600 AFY RUWAP Recycled Water Urban Irrigation Use and Implications for CSIP Yields

# I. Introduction

This document is an Addendum to the Pure Water Monterey Groundwater Replenishment Project Final Environmental Impact Report (PWM/GWR EIR) (State Clearinghouse No. 2013051094), and has been prepared pursuant to the California Environmental Quality Act (CEQA; Pub. Resources Code, §21000 et seq). The PWM/GWR Project Final EIR was certified and the PWM/GWR Project was approved by the Monterey Regional Water Pollution Control Agency (MRWPCA) Board of Directors on October 8, 2015<sup>1</sup>.

The MRWPCA Board of Directors approved the PWM/GWR Project as modified by the Alternative Monterey Pipeline and the Regional Urban Water Augmentation Project (RUWAP) alignment for the Product Water Conveyance pipeline and booster pump station. Subsequent to the approval of the PWM/GWR Project, minor changes to components of the PWM/GWR Project were subject to discretionary action by responsible agencies. These actions included approval of the Hilby Pump Station and minor re-alignments to the Monterey Pipeline distribution system, these actions did not require discretionary approval by MRWPCA, thus, addenda to the EIR were prepared and approved by Monterey Peninsula Water Management District's (MPWMD) Board of Directors (acting as responsible agency) on March 6, 2017 and June 20, 2016, respectively. In addition, the Bureau of Reclamation prepared an Environmental Assessment (EA) to comply with the National Environmental Policy Act (NEPA) for Title XVI funds to be awarded to the PWM/GWR Project (April 2017). Figure 1 provides an overview of the PWM/GWR approved facilities.

MRWPCA has prepared this Addendum to enable the PWM/GWR EIR to address the effects associated with modification to the operational capacity of the approved Advanced Water Treatment Facility (AWT Facility²) that would increase the AWT Facility from a maximum capacity (product water flowrate) of 4.0 million gallons per day (mgd) to a maximum capacity of 5.0 mgd. This expanded capacity, also referred to as the Expanded Capacity AWT Facility would enable delivery of 600 acre feet per year (AFY) of purified recycled water to Marina Coast Water District (MCWD) for MCWD customers to use for urban landscape irrigation. This water delivery is a component of the Regional Urban Water Augmentation Project (RUWAP). The RUWAP is an urban recycled water project developed by MCWD.

This Addendum also covers the proposal for MRWPCA and MCWD to jointly use storage (Blackhorse Reservoir) and product water conveyance (Product Water Conveyance) facilities for both the RUWAP and the PWM/GWR Projects. This would eliminate redundant conveyance facilities, namely, a single pump station at the AWT Facility site that is part of the AWT Facility would convey water for both projects eliminating two previously proposed and approved booster pump stations (one pump station for each project: the PWM/GWR Booster Pump Station and the RUWAP booster pump station). The combined Product Water Conveyance facilities would also result in elimination of approximately 40,000 linear feet of redundant pipeline. The RUWAP EIR and the PWM/GWR EIR each evaluated construction of product water conveyances pipelines along the same alignment. By sharing Product Water Conveyance facilities, only one pipeline would be needed to carry the recycled water for both projects.

<sup>&</sup>lt;sup>1</sup> MRWPCA recently announced an organizational name change and is now referred to as Monterey One Water. This Addendum uses MRWPCA for consistency with the PWM/GWR EIR.

<sup>&</sup>lt;sup>2</sup> Throughout the PWM/GWR EIR and this Addendum, the term Advanced Water Treatment (or AWT) Facility is used for consistency. The name of this same project component may also be also referred to as the Advanced Water Purification Facility (or AWPF) in project documentation. The terms are interchangeable.

With the combined facilities, purified recycled water would be conveyed along the same Product Water Conveyance alignment that was identified in the PWM/GWR EIR as the RUWAP Alignment Product Water Conveyance. The pipeline would convey up to 3,700 AFY (4.0 mgd maximum) of purified recycled water place a portion of California American Water's (CalAm's) water supply as required by state orders (as evaluated in the PWM/GWR EIR) as well as up to 600 AFY (1.0 mgd maximum) of irrigation water for use by MCWD (the subject of this Addendum and as evaluated in the RUWAP EIR). The source water to provide the 600 AFY to MCWD customers would come entirely from MCWD's rights to the return of its municipal wastewater per existing agreements (as evaluated in the RUWAP EIR).

During consultations with regulatory agencies and completion of 100% design drawings, minor changes were incorporated into the physical components of the AWT Facility. Those changes are reflected in Appendix A, Table of Project Description Text Changes to the PWM/GWR EIR. The design and physical features of the AWT Facility currently under construction would not need to be further modified for the Expanded Capacity AWT Facility because the approved AWT Facility includes additional redundant equipment with adequate hydraulic capacity to meet urban irrigation demands of up to 600 AFY in accordance with the MCWD existing demands and associated recycled water rights. However, the proposed Expanded Capacity AWT Facility would change operations of the AWT Facility compared to operations assumed in the PWM/GWR EIR. Additional purified water would be produced under the Expanded Capacity AWT Facility, resulting in a higher flowrate of reverse osmosis concentrate disposal to the existing ocean outfall. In addition, operation of the Expanded Capacity AWT Facility would require more electricity use than the approved AWT Facility. These operational changes are addressed in this Addendum.

In sum, this Addendum evaluates the following changes in the PWM/GWR Project:

- Operation of the AWT Facility at a peak capacity of up to 5.0 mgd to provide up to 600 AFY to MCWD.
- Shared use of Product Water Conveyance facilities (including Blackhorse Reservoir and pipelines) with MCWD for delivery of purified recycled water to urban irrigation customers as part of the RUWAP.

This Project Description portion of this Addendum also identifies minor design changes to the AWT Facility, including the brine mixing facility and product water pump station that resulted from agency consultation and completion of 100% design drawings. In addition, the site of the previously proposed brine mixing facility has been changed because it is not needed for disposing reverse osmosis concentrate, it is only needed for brine from a potential future desalination plant; therefore, it has been located in closer proximity to a proposed brine return line and the outfall.

# II. PROJECT LOCATION/FACILITIES

The PWM/GWR Project facilities are located within unincorporated areas of the Salinas Valley in Monterey County and within the cities of Salinas, Marina, Monterey, Pacific Grove, and Seaside.

<sup>&</sup>lt;sup>3</sup> In 1995, the State Water Resources Control Board (SWRCB) issued Order No. WR 95-10, which found that CalAm was diverting more water from the Carmel River Basin than it was legally entitled to divert. The State Board ordered CalAm to implement actions to terminate its unlawful diversions from the Carmel River and to maximize use of the Seaside Groundwater Basin (to the extent feasible) to reduce diversions of Carmel River water.

The Expanded Capacity AWT Facility and shared Product Water Conveyance Facilities are located within the approved boundaries of the PWM/GWR Project and RUWAP as shown in Figure 1, PWM/GWR Overview Map, and Figure 2, Shared Facilities for PWM/GWR and RUWAP. MRWPCA's Regional Treatment Plant (RTP) is located two miles north of the City of Marina, on the south side of the Salinas River as shown on Figure 3, Advanced Water Treatment Facility at Existing Regional Treatment Plant. The project setting would not change as a result of the proposed Expanded Capacity AWT Facility and shared Product Water Conveyance Facilities.

# III. PROJECT DESCRIPTION WITHIN PRIOR ENVIRONMENTAL DOCUMENTS

# PWM/GWR Project Description (Facilities and Description from the PWM/GWR EIR)4

The PWM/GWR Project approved by the MRWPCA Board of Directors under Resolution 2015-24 is a water supply project that will provide purified recycled water for recharge of the Seaside Basin that serves as a drinking water supply, and to augment recycled water for the existing Castroville Seawater Intrusion Project's crop irrigation. The PWM/GWR Project is jointly sponsored by the MRWPCA and the MPWMD, and also includes participation by the City of Salinas, the MCWD, and the Monterey County Water Resources Agency (MCWRA). The PWM/GWR Project includes the collection of a variety of new source waters (agricultural wash water, urban storm water runoff, and surface waters) which are combined with existing raw wastewater inflows to MRWPCA's RTP for treatment and recycling. The water is then used for two purposes: replenishment of the Seaside Groundwater Basin with purified recycled water to replace some of CalAm's existing drinking water supplies; and provision of additional recycled water supply for agricultural irrigation in northern Salinas Valley, enabling a reduction in groundwater pumping in that area. Secondary-treated effluent that is not further treated to tertiary levels for agricultural irrigation is conveyed to the AWT Facility for purification and then conveyed to the Seaside Basin to be used for replenishment of the Seaside Basin through the injection of the water into a series of shallow and deep injection wells. Once injected, this purified recycled water is mixed with other water in the basin, stored, and available for future extraction by CalAm through CalAm's existing extraction wells for delivery to its customers, to offset use of Carmel River's alluvial aquifer. The PWM/GWR Project also includes construction of a new distribution pipeline, the Monterey Pipeline, to enable the water purveyor, CalAm, to deliver the water to its customers5.

The following facilities are included in the PWM/GWR Project6:

Source water diversion and storage – facilities to enable diversion of new source waters to the
existing municipal wastewater collection system and conveyance of those waters to the Regional
Treatment Plant to increase availability of wastewater for recycling. Modifications would also be
made to the existing Salinas Industrial Wastewater Treatment Facility to allow the use of the
existing treatment ponds for storage of excess winter source water flows and later delivery to the
RTP for recycling.

<sup>&</sup>lt;sup>4</sup> This section describes the facilities of the PWM/GWR Project; See **Section IV**, **Proposed PWM/GWR Project Modifications** for a description of the changes to the PWM/GW Project under consideration in this Addendum.

<sup>&</sup>lt;sup>5</sup> Although referred to as the "Alternative Monterey Pipeline" in the PWM/GWR Project EIR, the term Monterey Pipeline is used to identify this pipeline alignment in this Addendum to be consistent with current terminology for these conveyance facilities.

<sup>&</sup>lt;sup>6</sup> Per the PWM/GWR EIR Project Description, in Section 2.6.1 of the PWM/GWR EIR "Proposed Project Facilities Overview".

- Treatment facilities at RTP use of existing primary and secondary treatment facilities at the RTP, as well as new pre-treatment, advanced water treatment (AWT), product water stabilization, product water pump station, and concentrate disposal facilities, and modifications to the Salinas Valley Reclamation tertiary treatment plant.
- *Product water conveyance* new pipelines, booster pump station, appurtenant facilities along the RUWAP pipeline alignment to move the product water from the RTP to the Seaside Groundwater Basin injection well facilities<sup>7</sup>.
- Injection well facilities new deep and vadose zone wells to inject product water into the Seaside Groundwater Basin, along with associated back-flush facilities, pipelines, electricity/ power distribution facilities, and electrical/motor control buildings.
- Distribution of groundwater from Seaside Groundwater Basin new CalAm distribution system improvements to convey extracted groundwater and deliver it to CalAm customers.

The RUWAP Alignment Product Water Conveyance was evaluated in the certified PWM/GWR EIR as one of two optional alignments and was approved by the MRWPCA as the preferred alignment.

The RUWAP Alignment Product Water Conveyance was also evaluated in the RUWAP EIR and Addenda as discussed below. See **Figure 4**, **RUWAP Facilities and Pipeline Alignment**.

# RUWAP Project Description and Environmental Documentation Background

MCWD currently owns, operates and maintains the potable water distribution and wastewater collection systems in the City of Marina and former Fort Ord under a permit for water distribution. The RUWAP Recycled Water Project is an urban recycled water project, and was originally developed to help MCWD meet the overall needs of its service area, delivering tertiary-treated and disinfected recycled water produced at the existing Salinas Valley Reclamation Plant (SVRP) to urban users in the MCWD service area and former Fort Ord<sup>8</sup>.

The RUWAP was considered in previous environmental documentation and agency actions, and included conveyance facilities for a recycled water distribution system to provide up to 1,727 AFY of chlorinated, tertiary-treated water from MRWPCA to urban users in the MCWD service area and former Fort Ord. The RUWAP EIR and Addenda evaluated the recycled distribution system as well as connection to the SVRP; a new distribution system consisting of up to 127,000 linear feet of 4- to 20-inch diameter main and lateral pipelines primarily within existing roadway rights-of-way throughout the MCWD region; a 1.9 MG storage tank located at an MCWD existing tank site (Blackhorse Reservoir) and one pump station (up to 4 motors at 200 hp each) located near the intersection of 3rd Street and 5th Avenue in the City of Marina.

CEQA compliance for the RUWAP was provided in the RUWAP EIR (certified October 2004) and Addenda No. 1, 2 and 3 to the RUWAP EIR (adopted in October 2006, February 2007, and April 2017 respectively). In addition, an EA was prepared by the Bureau of Reclamation in compliance with NEPA. It included more detailed information and updated alignments. **Figure 4** shows the approved RUWAP

<sup>&</sup>lt;sup>7</sup> This component of the GWR Project is now proposed to be built by MCWD through agreement with MRWPCA to share the use and cost of the conveyance facilities to convey and store purified recycled water for both urban irrigation and for groundwater replenishment. The booster pump stations associated with the PWM/GWR Product Water Conveyance and the RUWAP project have been eliminated and a storage tank called the Blackhorse reservoir would be built by MCWD. See Section IV, Proposed PWM/GWR Project Modifications and Figure 2.

<sup>&</sup>lt;sup>8</sup> This project has been referred to as both the Regional Urban Recycled Water Project or RUWAP and the Recycled Water Project; this document uses the acronym RUWAP for consistency.

facilities and existing components. Addendum No. 2 refined the total pipeline length, specifically, several alternative alignments were eliminated and some pipeline segments have since been constructed.

Addendum No. 3 to the RUWAP EIR described and analyzed the RUWAP for shared use of facilities between the PWM/GWR Project and the RUWAP. On April 18, 2016, MCWD adopted Addendum No. 3 to the RUWAP EIR for the construction of one single transmission pipeline and related facilities to deliver purified recycled water from the AWT Facility to the Seaside Groundwater Basin for the PWM/GWR Project and to MCWD's irrigation customers for the RUWAP Project. The approved facilities under this approved shared Product Water Conveyance Facilities include:

- Use of existing recycled water pipeline measuring 14-20" in diameter built by the Fort Ord Reuse Authority and Marina Coast Water District;
- 40,000 linear feet of new 24" diameter pipeline along the RUWAP EIR alignment coincident with the RUWAP Product Water Conveyance Pipeline alignment from the PWM/GWR EIR;
- A connection to the AWT Facility at the MRWPCA fence line with Armstrong Ranch;
- One 2 million gallon welded steel storage tank reservoir at an existing MCWD storage tank site referred to as the Blackhorse Reservoir; and
- An intermediate booster pump station located in the City of Marina referred to as the 5th Avenue Pump Station and pressure reducing valves and appurtenances<sup>9</sup>.

# Combined PWM/GWR and RUWAP CEQA Discussion

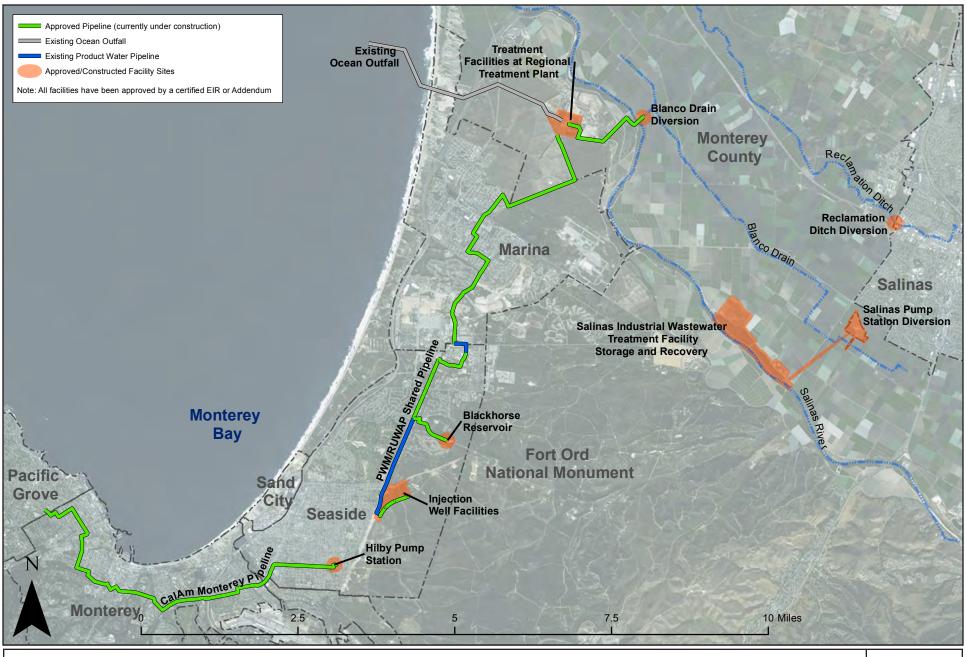
Both projects have certified environmental documentation, the RUWAP EIR and approved Addenda (Marina Coast Water District, 2009) and the PWM/GWR EIR (2015) all considered constructing a transmission pipeline along a common alignment, originating at the MRWPCA RTP and proceeding south through unincorporated Monterey County, Marina, California State University at Monterey Bay (CSUMB) and Seaside. Both EIRs and MCWD's Addenda described and evaluated a main pump station at the RTP and a booster pump station. The RUWAP and PWM/GWR EIR and associated Addenda each considered and evaluated an individual pump station within the City of Marina Corporation Yard and pipeline. The RUWAP EIR also considered the Blackhorse Reservoir, all lateral distribution pipeline alignments, originating at the transmission pipeline and extending outward to convey recycled water to existing and planned urban irrigation demands.

# **Incorporation by Reference**

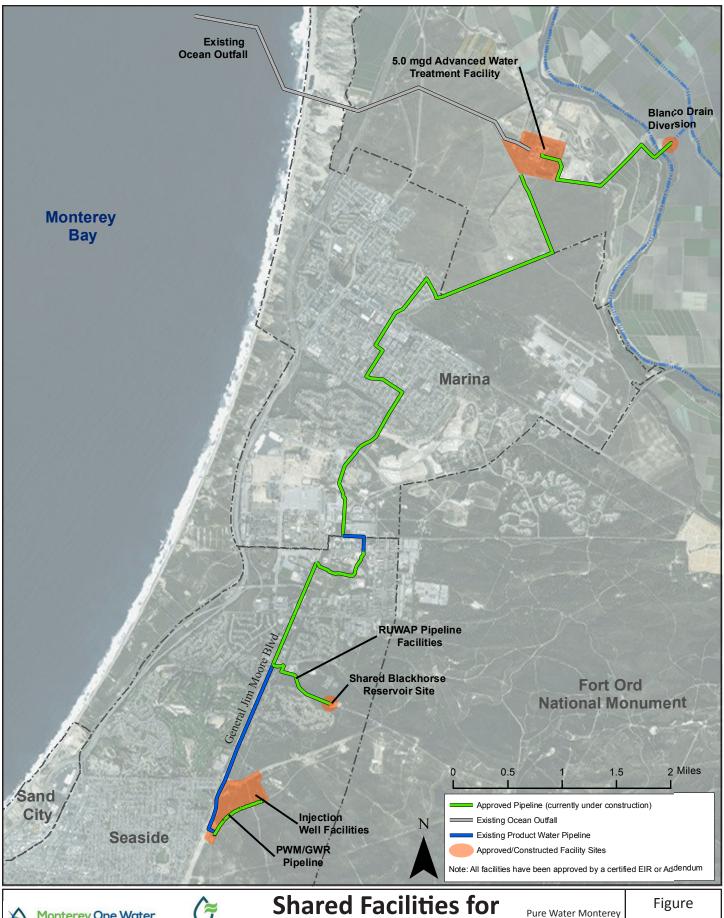
Consistent with Section 15150 of the CEQA Guidelines, the following documents were used in the preparation of this Addendum and are incorporated herein by reference:

- Draft and Final Environmental Impact Report Regional Urban Water Augmentation Project, (State Clearinghouse Number 2003081142) (MCWD 2004a)
- Addendum No. 1 (2006), Addendum No. 2 (2007) and Addendum No. 3 (2016) to the RUWAP EIR.
- Pure Water Monterey: Groundwater Replenishment Project EIR (State Clearing House Number 2013051094)
- Addendum No. 1 (2016) and Addendum No. 2 (2017) to the PWM/GWR EIR approved by the Monterey Peninsula Water Management District related to their approval of the Monterey Pipeline and Hilby Pump Station.

<sup>&</sup>lt;sup>9</sup> Under this PWM/GWR Addendum No. 3 Proposed Project, this booster pump station is eliminated. See **Section IV**, **Proposed PWM/GWR Project Modifications.** 









PWM/GWR and RUWAP

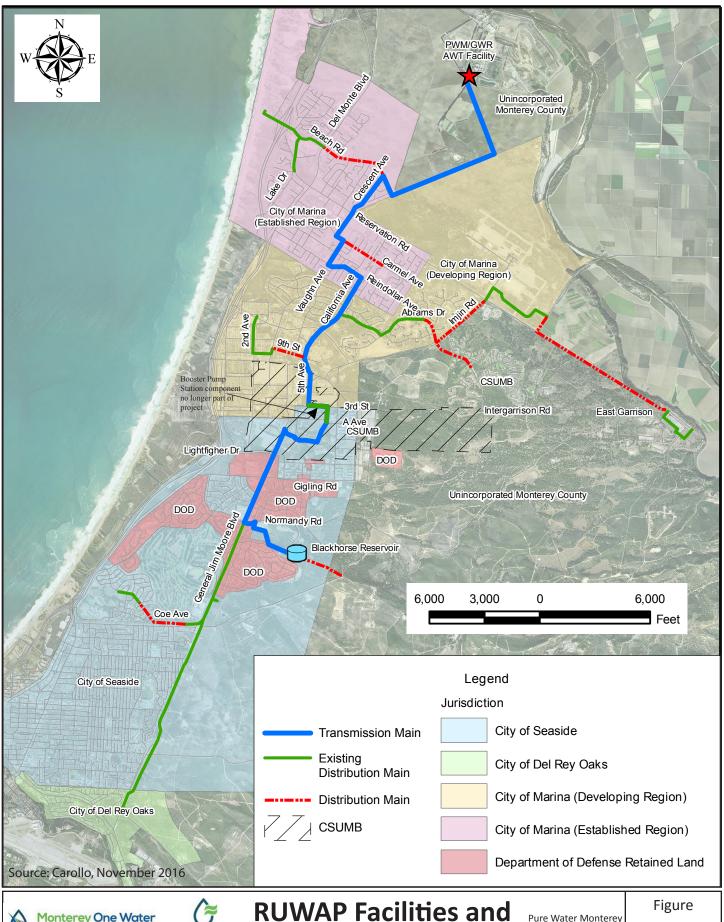
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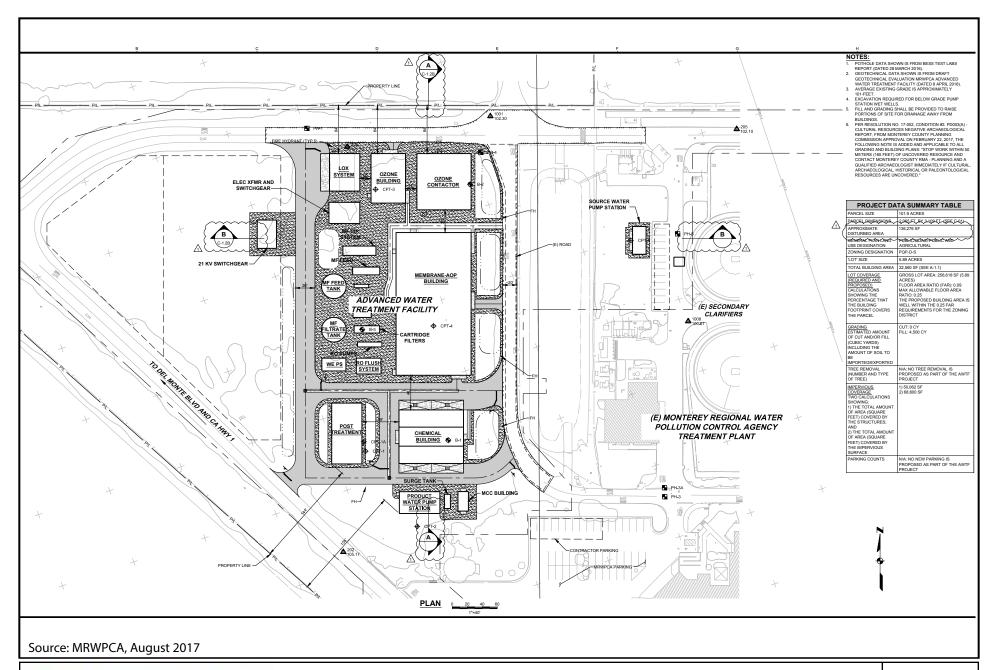


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# RUWAP Facilities and Pipeline Alignment

Pure Water Monterey GWR Project Addendum No. 3 October 2017



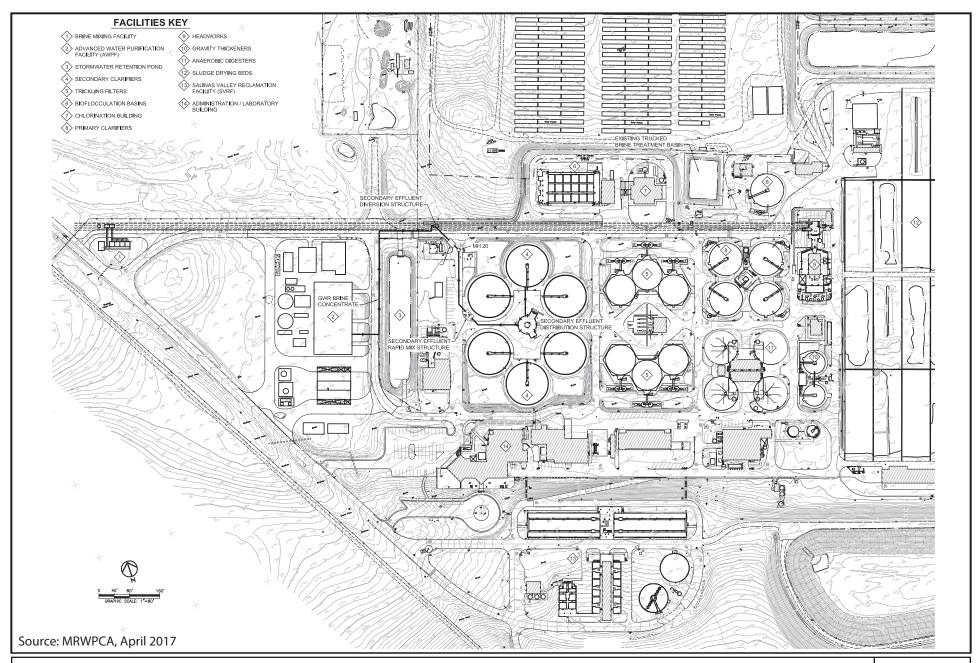


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**PWM/GWR Advanced Water Treatment Facility Site Plan** 

Pure Water Monterey **GWR Project** Addendum No. 3 October 2017 Figure **5**A







Future Brine Mixing Facility
Site Plan

Pure Water Monterey GWR Project Addendum No. 3 October 2017 Figure **5b** 

Information from these documents is incorporated by reference into this EIR Addendum and briefly summarized in the appropriate section(s).

- The full PWM/GWR Consolidated Final EIR is available for public review at the following address: <a href="http://purewatermonterey.org/reports-docs/cfeir">http://purewatermonterey.org/reports-docs/cfeir</a> or at the office of MRWPCA at 5 Harris Court, Building D, Monterey, CA 93940.
- The RUWAP EIR and Addenda are available for public review upon request at the office of the Marina Coast Water District at 11 Reservation Road, Marina, CA 93933-2099.

# IV. PROPOSED PWM/GWR PROJECT MODIFICATIONS

# **Project Overview**

Since certification of the Final PWM/GWR EIR in 2015, and approval of Addenda No. 1 and No. 2, MRWPCA has refined the design of the AWT Facility. The design refinements are identified in the bullets under the heading Modifications to the AWT Facility, below, and in **Appendix A**, **Table of Project Description Text Changes to the PWM/GWR EIR**. The design refinements include minor changes to the secondary effluent diversion structure to bring source water into the AWT Facility, the addition of booster pumping of the ozone effluent and pre-treated reverse osmosis feed, and the addition of waste water equalization and pump station. The biologically active filtration component has been approved for the project but may not be needed, and the brine mixing facility is not needed at the AWT Facility. The brine mixing facility may, however, be constructed elsewhere at the RTP. The AWT Facility currently is under construction.

MRWPCA is considering modifications to the approved PWM/GWR Project. First, MRWPCA is considering operation of the AWT Facility at a 5.0 mgd capacity to provide up to 600 AFY to MCWD for delivery to its customers as irrigation water. This involves approval from the Regional Water Quality Control Board (RWQCB).

In addition, MRWPCA is considering approval of sharing use of the Product Water Conveyance Facilities <sup>10</sup> with the MCWD for both the PWM/GWR and RUWAP projects. This would eliminate redundant infrastructure, as further described below.

# The Modified Project includes:

- 1) Approval of an increase in *peak or maximum* capacity of the AWT Facility from 4.0 mgd to 5.0 mgd and associated increase in water supply yield of 600 AFY of purified recycled water for urban irrigation. This requires application to the RWQCB and approval of an amended NPDES permit.
- 2) Minor physical modifications to the PWM/GWR Project as evaluated in the PWM/GWR and RUWAP EIRs):
  - a. <u>AWT Facility</u>: The detailed design review process, which included consultation and permitting with regulatory and partner agencies, has resulted in minor changes to AWT Facility equipment, buildings, and processes within the existing footprint (see below).

<sup>&</sup>lt;sup>10</sup> Pursuant to Addendum No. 3 to the RUWAP EIR, MCWD Board of Directors approved this shared facility use in April, 2016. Construction of the RUWAP facilities is underway.

- Due to redundant equipment within the 100% design currently being built, no further equipment changes would need to be constructed for the 5.0 mgd peak capacity plant.
- b. <u>Product Water Conveyance Facilities</u>: MRWPCA and the MCWD propose to jointly use a single pipeline for both the PWM/GWR Product Water Conveyance and the RUWAP recycled water delivery system. Product Water Conveyance Facilities are therefore modified based upon this shared use as follows:
  - i. Redundant equipment and facilities would be eliminated, including:
    - 1. two booster pump stations, one from each the RUWAP and PWM/GWR Projects.
    - 2. approximately 40,000 linear feet of pipeline would be eliminated (by sharing the Product Water Conveyance pipeline, only one Product Water Conveyance pipeline would be constructed as identified above).
  - ii. Portions of the transmission pipeline have been constructed. The shared pipeline initially would use these existing, built segments of RUWAP pipeline; in the future, these segments may need to be replaced with a 24" pipeline for energy efficiency.
  - iii. A 2.0 Million Gallon (MG) Blackhorse Reservoir (storage tank) site to be constructed on an MCWD existing tank site as part of the approved RUWAP would be shared by both the PWM/GWR Project and the RUWAP.

Please refer to Figure 5A PWM/GWR Advanced Water Treatment Facility Site Plan, Figure 6 Diagram of Existing MRWPCA RTP and Future AWT Facility Treatment Process and Figure 7 Eliminated RUWAP Facilities.

The following addresses the specific modifications to the AWT Facility and shared Product Water Conveyance Facilities as compared to the PWM/GWR EIR Project Description. For specific text changes to the PWM/GWR EIR and page references, please refer to **Appendix A**, **Table of Project Description Text Changes to the PWM/GWR EIR**.

# **Modifications to the AWT Facility**

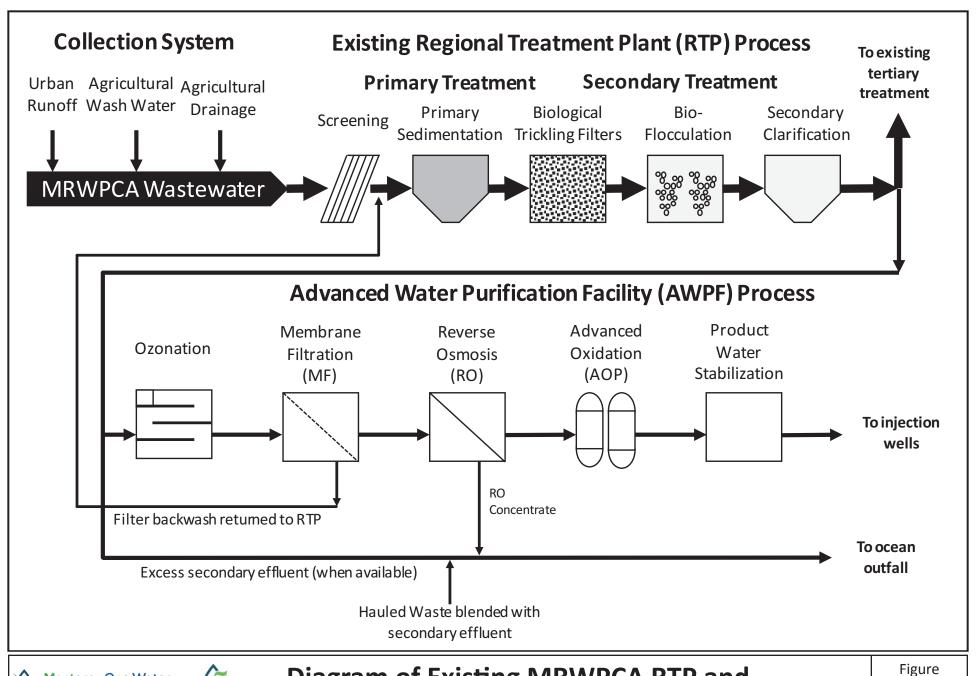
- Under the proposed Project modifications, an Expanded Capacity AWT Facility would be operated both to provide purified water for to enable injection into the Seaside Groundwater Basin (as described in the PWM/GWR EIR) and provision of up to 600 AFY of purified water to MCWD for urban landscape irrigation as part of the RUWAP.<sup>11</sup>
- The following is a list of the structures and facilities at the AWT Facility that have been modified during consultation with regulatory agencies and completion of 100% design drawings (see Figure 5A, PWM/GWR Advanced Water Treatment Facility Site Plan); text changes from the Final PWM/GWR EIR are shown in strike-out and underline to clarify the design modifications that have occurred:
  - <u>inlet source water secondary effluent</u> diversion structure, an <u>influent approximately 60-foot long</u>, 30-inch diameter pipeline to bring the secondary effluent to the source water pump station, the source water pump station, and an approximately 360 300-foot long, 24-inch diameter pipeline to bring secondary effluent to the <u>rest of the AWT Facility</u>;
  - advanced treatment process facilities, including

<sup>&</sup>lt;sup>11</sup> As described in previous sections, the approved PWM/GWR Project would divert additional water sources and convey those waters with municipal effluent to the Regional Treatment Plant, including urban and agricultural runoff, agricultural wash water flows, and excess/unused Regional Treatment Plant secondary-treated wastewater.

- o chloramination,
- o ozonation,
- o booster pumping of the ozone effluent,
- o biologically active filtration (approved for the Project, but it may not be required)
- o automatic straining,
- o membrane filtration treatment,
- o booster pumping of the membrane filtration filtrate,
- o cartridge filtration,
- o <u>reverse osmosis pre-treatment</u> chemical addition,
- o booster pumping of the pre-treated reverse osmosis feed,
- o reverse osmosis membrane treatment,
- o advanced oxidation using ultraviolet light and hydrogen peroxide (advanced oxidation),
- o side stream decarbonation, and
- o product-water stabilization with <del>calcium, alkalinity and pH adjustment</del> <u>liquid</u> <u>lime</u>;
- final product storage and distribution pumping water pump station; and
- brine mixing facilities; and
- waste water equalization and pump station.

Figure 6, Diagram of Existing MRWPCA RTP and Future AWT Facility Treatment Process, provides a simplified updated AWT Facility process flow diagram illustrating the treatment facilities. Concentrate from the reverse osmosis system would be combined with other effluent waste streams prior to final effluent sampling at a downstream wet well (sampling station) and disposal through the existing MRWPCA ocean outfall. As noted above, the brine mixing facility at the AWT Facility is not necessary for the PWM/GWR Project. While a new brine mixing facility is not currently under construction, it could still be constructed at the Regional Treatment Plant in the future for the purpose of mixing and monitoring seawater desalination plant brine (such as from the proposed Monterey Peninsula Water Supply Project desalination plant). If so, it would be located west of the AWT Facility (as shown on Figure 5B, Future Brine Mixing Facility Site Plan); this facility is within the area previously evaluated for construction.

Table 1, Updated AWT Facilities Design Summary and Table 2, Updated AWT Facility Process Design Flow Assumptions below were included in the PWM/GWR Project Description as Tables 2-18 and 2-19, respectively. They have been revised to reflect the changes made to the AWT Facility during the design process and to reflect the proposed Expanded Capacity AWT Facility.



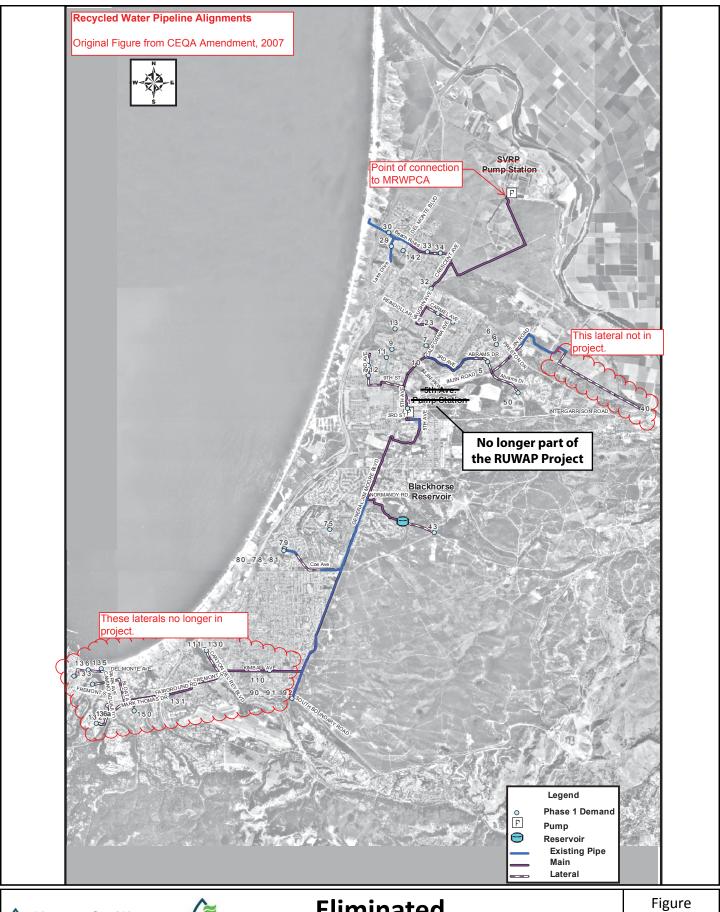




**Diagram of Existing MRWPCA RTP and Future AWP Facility Treatment Process** 

Pure Water Monterey GWR Project Addendum No. 3 October 2017

6







# **Eliminated RUWAP Facilities**

Pure Water Monterey **GWR Project** Addendum No. 3 October 2017

Table 1. Updated AWT Facilities Design Summary (Updated Table 2-18)

Component	Design Capacity (See Note a)
Pipeline from secondary treatment system outfall pipe to AWT Facility	N/A
AWT Facility Influent Wetwell	<del>0.2 mg</del>
Influent Pumping (see Note b) <u>Secondary Effluent Diversion Structure, Source Water Pump Station, and Chloramination</u>	2.7 to 5 <u>6</u> .9 mgd
Ozone System <del>(see Note b)</del>	<del>5</del> <u>6</u> .9 mgd
Biologically Active Filtration (may not beif required) (see Note c)	<del>5.5 mgd</del>
Membrane Filtration System	<u>46</u> .9 mgd
Reverse Osmosis System	<u>6</u> .2.2 to 4.9 mgd
Advanced Oxidation System, Product Water Stabilization and Pumping Product Water Pump Station	<u>45</u> .0 mgd

# Notes

Table 2 Updated AWT Facility Process Design Flow Assumptions

	Annual Flows <sup>1</sup>	Average Flow Conditions <sup>1</sup>	Maximum Flow Conditions <sup>2</sup>
AWT Facility Process	AFY	mgd	mgd
Source Water Pump Station and Ozone System Feed	5, <del>496</del> <u>898</u>	<del>4.9</del> <u>5.3</u>	<del>5</del> <u>6</u> .9
Biologically Active Filtration Feed	4,481	4.0	4.8
Biologically Active Filtration Backwash returned to Regional Treatment Plant Headworks	421	0.4	0.5
Biologically Active Filtration Bypass <sup>3</sup>	<del>1,015</del>	0.9	<del>1.1</del>
Membrane Filtration Feed	5, <del>075</del> <u>898</u>	4.5 <u>.3</u>	<del>5.5</del> <u>6.9</u>
Membrane Filtration Backwash retuned to Regional Treatment Plant Headworks	<del>508</del> <u>590</u>	0.5	0. <del>6</del> <u>7</u>
Reverse Osmosis Feed	4,567 <u>5,309</u>	4. <del>1</del> <u>7</u>	4.9 <u>6.2</u>
Reverse Osmosis Concentrate	<del>867</del> 1,009	0. <u>89</u>	<del>0.9</del> 1.2
Reverse Osmosis Product Water (AWT Facility Design Size)	<del>3,700</del> <u>4,300</u>	3. <del>3</del> <u>8</u>	<b>4</b> <u>5</u> .0
Advanced Oxidation Process, <u>Product Water</u> <u>Stabilization</u> , and <u>Product Water Pump Station</u>	<del>3,700</del> <u>4,300</u>	3. <del>3</del> <u>8</u>	<b>4</b> <u>5</u> .0

# Notes:

a. Capacities represent process feedwater flows; units are million gallons (mg) and million gallons per day (mgd).

b. For the case where biological filtration is not included, the range for the influent pumping would be 2.7 to 5.5 mgd, and the ozone system would be sized for 5.5 mgd.

c. The biologically active filtration would be sized to treat up to 80 percent of the process flow; the 5.5 mgd represents the total product flow when combined with the by pass. The Biologically Active Filtration is not included in the PWM/GWR Project; it may be constructed later at the AWTF if it is required as mitigation for California Ocean Plan compliance (for disposal of reverse osmosis concentrate to the MRWPCA ocean outfall).

<sup>1.</sup> Average annual flows reflect 3,700 4,300 AFY, typical annual production while building the drought reserve.

<sup>&</sup>lt;sup>2</sup>. Maximum flow condition reflects design peak production rate.

<sup>&</sup>lt;sup>3</sup>. 80% of the flow would pass through the Biologically Active Filtration, and 20% may bypass directly to the membrane filtration. Although Biologically Active Filtration will not be included in the PWM/GWR Project, it may be constructed later at the AWT Facility if it is required as mitigation for California Ocean Plan compliance for disposal of reverse osmosis concentrate to the MRWPCA ocean outfall.

As modified during the design process, the AWT Facility will include several structures up to 34 feet tall and totaling approximately 50,000 square feet (compared to 32 feet tall and 60,000 feet discussed in the PWM/GWR EIR).

The proposed Expanded Capacity AWT Facility would have a design capacity of 5.0 mgd of product water. A range of monthly source water flows has been estimated, depending upon the seasonal availability of source waters and demands. The facility would be operated to produce up to 3,700 AFY of purified recycled water for injection into the Seaside Basin and 600 AFY of purified recycled water to MCWD for urban landscape irrigation, which equates to an annual average production rate of 3.8 mgd. The 5.0 mgd facility size is required to allow for peak seasonal and daily operation and system down time. See **Appendix A, Table of Project Description Text Changes to the PWM/GWR EIR** for more information.

In producing an additional 600 AFY of highly purified water, the proposed Expanded Capacity AWT Facility would also produce higher quantities of two waste streams as compared to the quantities evaluated in the PWM/GWR EIR: membrane filtration backwash, and reverse osmosis concentrate. The membrane filtration backwash would be diverted back to the RTP headworks, while the reverse osmosis concentrate would be discharged through the existing ocean outfall. The Expanded Capacity AWT Facility is expected to produce water at up to 90% of design capacity, on average, due to some anticipated down time for membrane "clean in place" practices and repairs. The down time is assumed to be evenly distributed each month, though planned events would be scheduled for times when the least source water is available. The Expanded Capacity AWT Facility would need to be large enough to produce the required product water during the operational times (90% of each month). See **Appendix A, Table of Project Description Text Changes to the PWM/GWR EIR.** 

Based on these assumptions (including the 90% in-service, 81% reverse osmosis recovery, 90% microfiltration recovery), the Expanded Capacity AWT Facility design flow rate of 5.0 mgd would be required to provide up to 3,700 AFY of high quality water for groundwater injection and 600 AFY to MCWD for urban landscape irrigation.

In order to satisfy variations in the MCWD irrigation demand, the Expanded Capacity AWT Facility may operate with a range of production rates in order to meet variable demands of urban irrigation customers. The Expanded Capacity AWT Facility would be designed and constructed to allow production rates from 1.2 mgd (830 gpm) to 5.0 mgd (3,500 gpm). During a wet or normal year, the Expanded Capacity AWT Facility could operate at an average monthly rate of 4.0 mgd during the summer months (April to September). If the drought reserve is full (1,000 acre-feet additional purified recycled water has been stored in the Seaside Groundwater Basin), the winter production rate would remain 4.0 mgd. If the drought reserve is not full, the winter production rate would be increased to 4.2 mgd or greater as needed to allow the production of an additional 200 AFY. During certain dry years, the AWT Facility production rate would be decreased in the summer months, depending upon the amount of water stored in the drought reserve and the demands of the Castroville Seawater Intrusion Project (CSIP) irrigators. Like the previously approved AWT, the Expanded Capacity AWT Facility would produce enough purified recycled water in each year so that the amount of injected water plus the amount of "withdrawn" drought reserve or operational reserve equals the 3,500 AFY extracted by CalAm.

The Expanded Capacity AWT Facility power would be supplied through two new utility connections, one from Monterey Regional Waste Management District (MRWMD) and one from PG&E to the RTP. MRWMD utilizes biogas produced by the decomposition of waste material in the landfill to produce

electrical energy. Since certification of the PWM/GWR EIR, MRWMD and the MRWPCA have executed an agreement for MRWPCA to purchase 1,800 (kWh) or more of electrical energy which is surplus electrical energy available for export and sale to MRWPCA for operation of the AWT Facility. The energy demand needed for the Expanded Capacity AWT Facility was updated by Kennedy Jenks Engineers. The amount of renewable energy to be provided under the agreement with MRWMD is more than the total projected annual electrical demand for the Expanded Capacity AWT Facility. **Table 3, Overview of Proposed Project Electricity Demand** presented below provides the estimated energy demands for the Expanded Capacity AWT Facility; the demands are presented as mega-watt hours (MWh) per year,

# **NPDES Permit**

As identified in the PWM/GWR EIR, the current MRWPCA wastewater discharge is governed by NPDES permit R3-2014-0013 issued by the Central Coast Regional Water Quality Control Board (RWQCB). MRWPCA will need to obtain an amended permit or a new permit from the Central Coast RWQCB to discharge the reverse osmosis concentrate generated by the PWM/GWR Project through the existing outfall. The Central Coast RWQCB will consider compliance with the Water Quality Control Plan for Ocean Waters of California (Ocean Plan) which establishes water quality objectives and beneficial uses for waters of the Pacific Ocean adjacent to the California coast outside of estuaries, coastal lagoons, and enclosed bays. Additionally, the National Oceanic and Atmospheric Administration (NOAA) – Monterey Bay National Marine Sanctuary (MBNMS) will provide authorization and/or approval of the discharge permit. Because operation of the Expanded Capacity AWT Facility would generate a higher amount of reverse osmosis concentrate than the previously approved AWT Facility, Trussell Technologies has prepared updated Ocean Plan Compliance Assessments, which are included in the Appendices to this Addendum.

# Table 3 Overview of Proposed Project Electricity Demand

Updated Revised Table 2-11	4.0 MGD	5.0 MGD
Overview of Proposed Project Electricity Demand (all in megawatt-hours per year)	EIR 2015	Addendum
Source Water Diversion and Storage Sites (Source: Vinod Badani, E2 Consulting, October 2014, except as noted	1)	•
Existing MRWPCA Wastewater Collection System Pump Stations	1100	1100
(increased pumping for source water collection) (Source: Bob Holden, MRWPCA, October 2014)		
Proposed Salinas Pump Station Diversions	10	10
(lighting, SCADA, misc. electricity)[Note: this facility now operates almost exclusively using solar energy.]		
Proposed Salinas Industrial Wastew ater Treatment Plant Storage and Recovery Component	224	100
(pumping, lighting, SCADA, misc. electricity)		
Existing Salinas Treatment Facility and Stormwater Operations	-1875	-1875
(reduction of pumping, Ron Cole, February 2014 modified by MRWPCA staff October 2014)		
Proposed Reclamation Ditch Diversion	250	250
(pumping, lighting, SCADA, misc. electricity)		
Proposed Tembladero Slough Diversion	461	461
(pumping, lighting, SCADA, misc. electricity)		
Proposed Blanco Drain Diversion	731	731
(pumping, lighting, SCADA, misc. electricity)		
Proposed Lake El Estero Diversion	10	10
(lighting, SCADA, misc. electricity)		
Treatment Facilities at Regional Treatment Plant (Source: Bob Holden, October 2014)		
Existing Primary and Secondary Processes	3673	3673
(existing on-site cogeneration facility would provide a reduction in this value, see below)		
(9,900 AFY more w astew ater flows through treatment processes)		
Existing Salinas Valley Reclamation Plant	1300	1300
(existing plant operations use solar array electricity, which has reduced electricity demand by up to 1,400 mWhr/yr)		
(4,260 AFY more crop irrigation w ater produced)		
4.0 AWT Facility (2015 GWR EIR)  (new treatment facilities, not including product w ater pumping; assumes 3,700 AFY of w ater production to build drought	7007	0
reserve; demand will be less when Drought Reserve is at full capacity and when Drought Reserve is being used by CSIP)		
5.0 Expanded Capacity AWT Facility assumes 4,300 AFY of water production (Source: Kennedy Jenks September 2017)	0	12930
CSIP Supplemental Wells (Source: Bob Holden, MRWPCA, October 2014)		12330
Reduction of use of CSIP Supplemental Wells by 4,260 AFY	-1900	-1900
Product Water Conveyance (Source: TG Cole, October 2014)	1300	1500
Pumping of product water to Injection Well Facilities under RUWAP (1)	1912	l 0
Injection Well Facilities (Source: Vinod Badani, E2 Consulting Engineers, October 2014)	1312	
Back-flush of four (4) deep injection wells, lighting, HVAC, meters, instruments, SCADA	147	147
CalAm Distribution System Changes (Source: CalAm, 2014)	147	1 1 1 7 /
Increase by moving 3,500 AFY extractions from Carmel River to Seaside Basin wells	630	630
Proposed New Electricity Generation at MRWPCA Existing Cogeneration Facility	-2726	
New Purchased Electricity from Monterey Regional Waste Management District (2)	2720	-14200
NET TOTAL (with reduction in energy demand from renewable energy sources)	10,954	
(1) GWR EIR and RUWAP EIR each proposed two parallel pipelines; reduction to one pipeline and no pump st	,	
(2) The Monterey Regional Waste Management District (MRWMD) utilizes biogas produced by the decomposit	*	
material in the landfill to produce electrical energy. MRWMD will provide 1800KwH for AWPF operation at the site.		
The RTP is adjacent to the landfill and power generation facility operated by MRWMD.		
Source: MRWPCA and Kennedy Jenks, September 2017		

# **Shared Product Water Conveyance Facilities**

The proposed Project modifications include changing the product water conveyance method from a separately built pipeline to use of shared Product Water Conveyance Facilities for the PWM/GWR Project and the RUWAP. The RUWAP EIR evaluated pipelines of varying sizes (4-inch to 20-inch) and the RUWAP EIR Addendum No. 3 assumed a shared pipeline of adequate dimension to accommodate the combined projects (from 24 inches up to 30 inches) and construction areas of approximately 30-40 feet wide depending on the location.

The Blackhorse Reservoir is an approved component of the RUWAP. The proposed Project modifications include shared use of this facility by the PWM/GWR Project. The approved reservoir will have a capacity of 2.0 MG. It will be approximately 32 feet in height and 120 feet in diameter, and it will be constructed within MCWD's existing property limits on an existing tank site. See **Figures 8 and 9, Blackhorse Reservoir Tank Location and Blackhorse Reservoir Tank Site Plan** for more information on the location of the reservoir and MCWD property limits. Temporary staging areas for stockpiling of soil and/or storing of materials and equipment during construction also will be within the APE.

## GENERAL NOTES

- CONTRACTOR SHALL VERIFY ALL DIMENSIONS BEFORE STARTING WORK AND SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY DISCREPANCIES.
- CONTRACTOR SHALL POTHOLE AND VERIFY VERTICAL AND HORIZONTAL LOCATIONS OF EXISTING UTILITIES AT LEAST 7 DAYS IN ADVANCE OF CONSTRUCTION OPERATIONS TO ALLOW FOR MINOR GRADE ADJUSTMENTS WITHOUT DELAYING INSTALLATION.
- THE ENSTEADS AND LOCATION OF ANY UNDERGROUND UTILITIES OR STRUCTURES SHOWN ON THESE FAUS WERE DETAINED BY A SEARCH OF AVAILABLE REFORDS, SURREVOY OF NEINE ABOVE FROOMS OWERFACE FEATURES AT THE TIME OF SURVEY, AND LIMITED POTHOLING AS SHOWN ON THE FLANS. THIS DOES NOT GUARANTEE THE ACCURACY, COMMETENESS, LOCATION, OR THE EXISTENCE OR NOVENSTENCE OF ANY UTILITY PIPE OR STRUCTURE WITHIN THE LIMITS OF THIS PROJECT, THE CONTRACTOR IS REQUIRED TO TAKE ALL DUE PRECAUTIONARY MEANS NECESSARY TO PROTECT THOSE UTILITY THESE NOT SHOWN ON THESE PLANS.
- AN ENCROACHMENT PERMIT FROM THE APPLICABLE JURISDICTIONAL AGENCY IS REQUIRED PRIOR TO ANY WORK WITHIN PUBLIC RIGHT-OF-WAY, THE CONTRACTOR SHALL ADHERE TO ALL REQUIREMENTS OF ALL ENCROACHMENT PERMIT IS ISSUED.
- UNDER THE DIRECTION OF A SURVEYOR LICENSED IN THE STATE OF CALIFORNIA, THE CONTRACTOR SHALL RESET ALL MONUMENTATION DISTURBED OR REMOVED DURING CONSTRUCTION ACTIVITIES.
- WHERE THE RECYCLED WATERLINE CROSSES EXISTING UTILITIES, THE RECYCLED WATERLINE SHALL BE INSTALLED A MINIMUM OF 12\* VERTICALLY FROM THE EXISTING UTILITY. THE RECYCLED WATERLINE MUST CROSS BELOW EXISTING WATERLINES AND ABOVE EXISTING SANITARY SEWER LINES - SEE MCWD TYP DET W-16 FOR ADDITIONAL REQUIREMENTS.
- ANY CONTRACTOR PERFORMING WORK ON THIS PROJECT SHALL BECOME FAMILIAR WITH THE SITE AND SHALL BE SOLELY RESPONSIBLE FOR ANY DAMAGE TO EXISTING FACILITIES RESULTING DIRECTLY OR INDIRECTLY FROM HIS OPERATIONS.
- 8. DISPOSE OF ALL SURPLUS EXCAVATION MATERIALS IN CONFORMANCE WITH LOCAL CODES & REGULATIONS.
- 9. PRIOR TO ANY CONNECTION TO AN EXISTING UTILITY, THE CONTRACTOR SHALL COORDINATE WITH THE OWNER

# BASIS OF BEARINGS

COORDINATES SHOWN HEREON ARE GRID BEARINGS PER THE CALIFORNIA COORDINATE SYSTEM, NAD 83, ZONE 4 (EPOCH 2002.00), AS DETERMINED BY GPS OBSERVATIONS AND THE NATIONAL GEODETIC SURVEY'S (NGS) ONLINE POSITIONING USER SERVICE (OPUS). ALL DISTANCES ARE GRID DISTANCES. AERIAL TOPOGRAPHY FLOWN AUGUST-SEPTEMBER 2006.

BENCHMARK: ELEVATIONS SHOWN HEREON ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88), A PUBLISHED NAVD 88 ELEVATION OF 145.94 FEET FOR THE NOS BENCHMARK "L 813 RESET" (PID GUZ130) WAS HELD AS THE PRIMARY VERTICAL BENCHMARK FOR THE PROJECT.

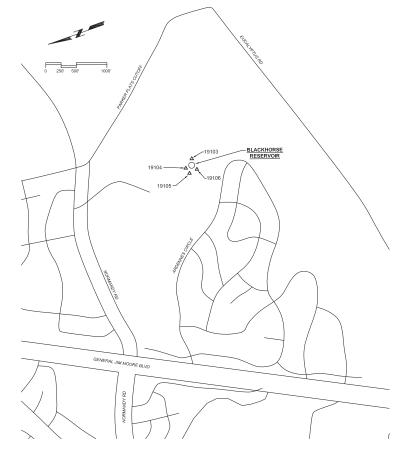
# CONTROL POINT SCHEDULE

CONTROL POINTS WERE SURVEYED IN 2006 AND SOME OR ALL OF THE CONTROL POINTS MAY NO LONGER EXIST, CONTRACTOR SHALL ESTABLISH ITS OWN SURVEY CONTROL AND CONTROL POINTS, AS NEEDED TO COMPLETE THE WORK, USING A CALIFORNIA LICENSED SURVEYOR.

PNT	NORTHING	EASTING	ELEV	DESC.
19103	2125194,6960	5740401,2660	481,9820	BEST
19104	2125329.3140	5740278.9840	483.8590	RBR/CAP
19105	2125290.9710	5740180.9040	486,3650	RBR/CAP
19106	2125158,0410	5740213.8580	484,4970	RBR/CAP

# ABBREVIATIONS

ABC	AGGREGATE BASE COURSE	FLEX	FLEXIBLE
BCW	BARE COPPER WIRE	GV	GATE VALVE
BFV	BUTTERFLY VALVE	MH	MANHOLE
С	CONDUIT	PCS	PVC COATED GALVANIZED STEEL CONDUIT
CL	CLASS	PW	POTABLE WATER
CL	CENTERLINE	(R)	RESTRAINED FITTING
C.O.	CLEANOUT	RDCR	REDUCER
CP	CONTROL PANEL	RW	RECLAIMED WATER
DIP	DUCTILE IRON PIPE	scv	SWING CHECK VALVE
DMJT	DISMANTLING JOINT	SD	STORM DRAIN
ECC	ECCENTRIC	TSP	TWITED SHREDDED PAIR



# **IDENTIFICATION** SYMBOLS

EQUIPMENT/INSTRUMENT LOCATOR

INDICATES KEYNOTE 1 (PERTAINS ONLY TO SHEET WHERE NOTE IS FOUND)

NEW STRUCTURES OR EDGE OF PAVEMENT

EQUIPMENT TAG NUMBER

## CIVIL SYMBOLS

EXISTING STRUCTURES (SCREENED) NEW PIPING (SINGLE LINE) EXISTING PIPING (TRIPLE LINES) (SCREENED) EXISTING PIPING (SINGLE LINE)(SCREENED) HIDDEN LINE OR TRAIL EDGE CENTER MONIMENT OR SURVEY LINE EXISTING CONTOURS (SCREENED) NEW CONTOURS EXISTING FENCE (SCREENED) PROPERTY LINE OR RIGHT OF WAY

ASPHALT PAVING

SWING CHECK VALVE PRIMARY FLOW ELEMENT X = M - MAGNETIC

CONTROL POINT

# MECHANICAL SYMBOLS

FLEXIBLE COUPLING ADAPTER DISMANTLING JOINT ELBOW DOWN ECCENTRIC REDUCER TF, BF -D-ELBOW, 90 DEGREE +++ GATE VALVE  $-\bowtie$ -1

Source: Carollo and MCWD, April 2017



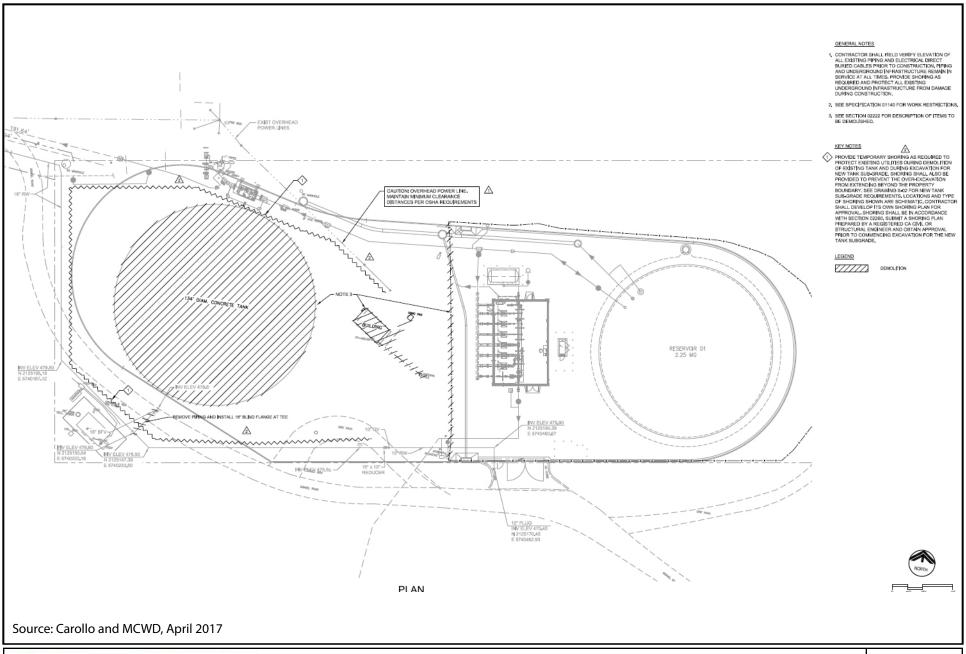


# **Blackhorse Reservoir Tank Location**

Pure Water Monterey **GWR Project** Addendum No. 3 October 2017

**Figure** 

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**Prepared by Denise Duffy and Associates** 



# Blackhorse Reservoir Tank Site Plan

Pure Water Monterey GWR Project Addendum No. 3 October 2017 Figure

9

# V. SUPPLEMENTAL OR SUBSEQUENT EIR NOT REQUIRED

Under CEQA, a lead agency shall prepare an addendum to a previously certified EIR if some changes or additions are necessary to the EIR but none of the conditions described in State CEQA Guidelines section 15162 calling for preparation of a subsequent EIR have occurred (State CEQA Guidelines, §15164(b)).

State CEQA Guidelines section 15162 provides that when an EIR has been adopted for a project, a subsequent EIR or EIR shall be prepared for that project if the lead agency determines one or more of the following have occurred:

- 3) Substantial changes are proposed in the project which will require major revisions of the previous EIR or EIR due to the involvement of new significant environmental effects or a substantial increase in the severity of previously identified significant effects;
- 4) Substantial changes occur with respect to the circumstances under which the project is undertaken which will require major revisions of the previous EIR or EIR due to the involvement of new significant environmental effects or a substantial increase in the severity of previously identified significant effects; or
- 5) New information of substantial importance, which was not known and could not have been known with the exercise of reasonable diligence at the time the previous EIR was certified as complete or the EIR shows any of the following:
  - a. The project will have one or more significant effects not discussed in the previous EIR or EIR;
  - b. Significant effects previously examined will be substantially more severe than shown in the previous EIR;
  - c. Mitigation measures or alternatives previously found not to be feasible would in fact be feasible and would substantially reduce one or more significant effects of the project, but the project proponents decline to adopt the mitigation measure or alternative; or
  - d. Mitigation measures or alternatives which are considerably different from those analyzed in the previous EIR would substantially reduce one or more significant effects on the environment, but the project proponents decline to adopt the mitigation measure or alternative.

An addendum need not be circulated for public review and can be attached to the adopted EIR (State CEQA Guidelines, §15164(c)).

This Addendum has been prepared pursuant to CEQA Guidelines Section 15164, which states: "A lead agency or responsible agency shall prepare an addendum to a previously certified EIR if some changes or additions are necessary but none of the conditions described in §15162 calling for preparation of a subsequent EIR have occurred."

The following discussion summarizes the reasons why a subsequent or supplemental EIR, pursuant to CEQA Guidelines Section 15162, is not required in connection with approvals for the proposed expansion of the capacity of the AWT Facility and shared Product Water Conveyance facilities and why an addendum is appropriate. As explained below in **Section VI**, substantial evidence supports the conclusion that the Expanded Capacity AWT Facility and shared Product Water Conveyance Facilities would not result in new significant environmental effects or a substantial increase in the severity of a significant effect previously identified in the PWM/GWR EIR or RUWAP EIR. Additionally, there is no new information that shows new significant environmental effects or an increase in the severity of

previously identified significant effects. Nor is there new evidence showing that mitigation measures or alternatives previously found not to be feasible would in fact be feasible and would substantially reduce one or more significant effects of the project, or that mitigation measures or alternatives which are considerably different from those analyzed in the previous EIR would substantially reduce one or more significant effects on the environment. For these reasons, preparation of an addendum is appropriate under these circumstances.

# VI. COMPARISON TO THE CONDITIONS LISTED IN CEQA GUIDELINES §15162

The proposed Project modifications consist of an increase in the maximum flowrate and yield of the AWT Facility and use of shared Product Water Conveyance Facilities.

# **Environmental Effects**

As detailed in Section VII, Environmental Analysis, the proposed Project modifications would not result in any new significant environmental effects that cannot be mitigated with existing, previously identified mitigation measures in the PWM/GWR EIR and the RUWAP EIR. In addition, the Expanded Capacity AWT Facility and shared Product Water Conveyance Facilities as fully described in Section IV, Proposed PWM/GWR Project Modifications would not substantially increase the severity of any significant environmental effects identified in the PWM/GWR EIR and the RUWAP EIR. The potential environmental effects associated with the modifications to the project would not result in any new environmental effects that were not previously disclosed in connection with the construction of the PWM/GWR Project and the RUWAP. The proposed Project modifications would not increase the extent of ground-disturbance and would not increase the overall length of pipeline. The proposed Project modifications would result in changes to the amount and quality of reverse osmosis (RO) concentrate, but these impacts would be consistent with the type, extent, and scope of impacts already analyzed with respect to the operation of the PWM/GWR Project. No new adverse environmental effects would occur in connection with the Expanded Capacity AWT Facility and shared Product Water Conveyance Facilities.

# **New Information**

No new information has been identified or presented to MRWPCA showing that the Expanded Capacity AWT Facility and shared Product Water Conveyance Facilities would result in: 1) significant environmental effects not identified in the PWM/GWR EIR and RUWAP EIR, or 2) an increase in the severity of significant impacts identified in the PWM/GWR EIR and RUWAP EIR. Further, no new information has been identified or presented to MRWPCA showing that mitigation measures or alternatives which were previously determined not to be feasible would in fact be feasible and would substantially reduce one or more significant effects of the project, or mitigation measures or alternatives that are considerably different from those identified in the PWM/GWR EIR would be feasible and would substantially reduce one or more significant effects of the project.

# Conclusion

Section 15164 of the CEQA Guidelines states that a lead agency or responsible agency shall prepare an addendum to a previously certified EIR if some changes or additions are necessary but none of the conditions described in Section 15162 calling for preparation of a supplemental or subsequent EIR have occurred. Based on the information in this Addendum, MRWPCA has determined that:

- No new significant environmental effects or a substantial increase in the severity of previously identified significant effects would occur as a result of operation of the proposed Expanded Capacity AWT Facility and shared Product Water Conveyance Facilities;
- No substantial changes have occurred or would occur with respect to the circumstances under which the PWM/GWR Project was originally undertaken, which would require major revisions to the previously certified PWM/GWR EIR (updated by addenda) due to the involvement of new significant environmental effects or a substantial increase in the severity of previously identified significant effects; and
- No new information of substantial importance has been received or discovered, which was not known and could not have been known with the exercise of reasonable diligence at the time the PWM/GWR EIR was certified as complete.
- None of the conditions or circumstances that would require preparation of a subsequent or supplemental EIR pursuant CEQA Guidelines Section 15162 exists.

# VII. ENVIRONMENTAL ANALYSIS

The following section consists of an analysis of the potential environmental effects associated with the proposed Project modifications. This section includes a summary of existing environmental documentation prepared for the PWM/GWR Project and RUWAP Project.<sup>12</sup> The RUWAP EIR and Addenda discussion is included for background.<sup>13</sup> RUWAP Addendum No. 3 specifically addressed construction of a single (shared) transmission pipeline and related facilities (including the Blackhorse Reservoir) to deliver purified recycled water from the AWT Facility to the Seaside Groundwater Basin for the PWM/GWR Project and to MCWD's irrigation customers for the RUWAP. Following the discussion of the findings of the prior environmental analyses, this section presents an analysis of potential environmental effects associated with the proposed project modifications. Finally, each individual topical section includes a conclusion regarding the proposed modifications' potential environmental effects.

The proposed project modifications are located within the same project area as the approved PWM/GWR Project and thus discussion of the regional and local environmental and regulatory setting, provided in detail in the PWM/GWR Final EIR, also applies to the proposed project modifications.

The proposed project modifications would incorporate and implement mitigation measures identified in the certified PWM/GWR EIR Mitigation Measure Monitoring Program (MMRP) as adopted by the MRWPCA Board upon certifying the EIR and approving the PWM/GWR Project. Specific mitigation measures relevant to a particular impact of the proposed project modifications are cited in the same manner as in the PWM/GWR EIR below. The Mitigation Measure Monitoring Program (MMRP) adopted in conjunction with the PWM/GWR approvals is included as **Appendix B**, **Table 1** of this Addendum.

<sup>&</sup>lt;sup>12</sup> The MPWMD prepared two Addenda to the PWM/GWR EIR that evaluated the potential environmental effects associated with the construction and operation of the Hilby Avenue Pump Station (Addendum No. 1) and the minor realignment of a segment of the Monterey Pipeline (Addendum No. 2). These addenda determined that these project modifications would not result in any new significant environmental effects or substantially increase the severity of a previously identified significant effect. These Addenda did not involve any changes to the AWT Facility or the use of shared facilities. As a result, these Addenda are not discussed in detail in this Addendum.

<sup>&</sup>lt;sup>13</sup> As previously noted, MCWD adopted Addendum No. 3 to the MCWD RUWAP EIR on April 18, 2016. This approval included a 2 million gallon (MG) welded steel storage tank reservoir at an existing MCWD storage tank site referred to as the Blackhorse Reservoir.

Similarly, the MCWD would implement the adopted RUWAP EIR mitigations relevant to the RUWAP Pipeline Facilities; the RUWAP MMRP is included as **Appendix B, Table 2** of this Addendum.

# **Aesthetics**

# Summary of Impacts in Previous Documents

**PWM/GWR EIR:** The PWM/GWR EIR (Vol 1 p. 4.2-1 – 4.2-52) concluded that construction and operation of the PWM/GWR Project would result in less than significant impacts to scenic views, scenic resources, and the visual quality of surrounding areas during both construction and operation. The PWM/GWR EIR concluded that construction of the AWT Facility would not result in a substantial adverse aesthetic-related effect. The PWM/GWR EIR found that there would be potentially significant aesthetic-related impacts due to additional light and glare at the Booster Pump Station and the Injection Well Facility. These impacts would be mitigated to a less than significant level through the implementation of Mitigation Measure AE-2: Minimize Construction Nighttime Lighting, and Mitigation Measure AE-4: Exterior Lighting Minimization (see **Appendix B, Table 1** for full text of mitigation measures). (see Table 4.2-4 Summary of Impacts - Aesthetics, p. 4.2-27 of the PWM/GWR EIR). The PWM/GWR EIR found that the PWM/GWR Project would have less than significant cumulative impacts to aesthetics.

RUWAP EIR and Addenda: The RUWAP EIR and Addenda evaluated a shared Product Water Conveyance Pipeline and construction of the Blackhorse Reservoir sized to accommodate both RUWAP and PWM/GWR Project storage facilities (up to 1.9 MG in size). Addendum No. 3 certified by MCWD on April 18, 2016, and related CEQA analysis, subsequently evaluated the Blackhorse Reservoir and assumed it would be up to 2.0 MG, see Aesthetics in the RUWAP EIR and Addenda (Addendum No. 3 Section 3.1, p.15). The RUWAP EIR evaluated reservoirs and tanks with over 63 acres of disturbance. The RUWAP EIR and Addenda found adverse aesthetic impacts related to construction of the Blackhorse Reservoir would be reduced to less than significant with implementation of the RUWAP EIR mitigations listed in **Appendix B, Table 2**. Mitigation measures would minimize impacts to surrounding areas through screening using fencing and/or vegetation, and use of appropriate colors that blend in with the surrounding landscape.

# **Proposed Project Modifications**

The proposed modifications to the PWM/GWR Project would not change the physical location or increase the size of the PWM/GWR or RUWAP structures or facilities. In addition, the proposed project modifications would eliminate redundant projects facilities thereby decreasing the extent of potential aesthetic-related effects. Therefore, the construction and operation of the proposed project modifications would not have a significant effect on the visual environment. Based on the overall reduction in facilities,

<sup>&</sup>lt;sup>14</sup> The Blackhorse Reservoir is currently planned for construction with approved mitigation identified in the Combined RUWAP and PWM/GWR EIR MMRP (See **Appendix B, Table 2**). The reservoir will be located at the existing MCWD storage tank site within the fence line of MCWD facilities. In 2017, DD&A prepared a CEQA Memorandum to MCWD for the RUWAP record [to address the Blackhorse Reservoir upgrade from 1.9 MG to 2.0 MG. The CEQA memorandum concluded the Blackhorse Reservoir has been addressed under multiple certified environmental documents including the RUWAP EIR and Addenda No. 1, 2 and 3, as well as the Tanks Improvement Project EA/Initial Study (IS); RUWAP EA and the State Revolving Fund (SRF) Environmental Package, dated November 2015. Sizing of the reservoir/tank has ranged up to 2.25 MG at the existing facility site. The CEQA memorandum noted the reservoir would be constructed within the APE of the previously analyzed tank site and concluded the minor increase in dimensions and capacity would not result in new significant environmental effects to aesthetic values or scenic resources nor would the minor change increase the severity of environmental impacts already identified in the previous environmental documents.

the proposed modifications would not result in any new or substantially more severe aesthetic-related effects beyond those previously identified in the above referenced environmental documentation. Additionally, all project impacts would be mitigated to less than significant with implementation of previously approved mitigation measures.

# Conclusion

In conclusion, the proposed Project modifications would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect. The findings of the existing environmental documentation relative to aesthetics would remain unchanged and no new or substantially revised mitigation measures are warranted.

# Air Quality and Greenhouse Gas

# Summary of Impacts in Previous Documents

**PWM/GWR EIR:** The PWM/GWR EIR (Vol 1 p. 4.3-1 – 4.3-40) found that emissions during operation and construction of the PWM/GWR Project would not conflict with an existing air quality plan, nor conflict with any applicable plan, policy, or regulation adopted for the purpose of reducing greenhouse gas (GHG) emissions. The PWM/GWR EIR found that construction of each of the components of the PWM/GWR Project would not result in a significant increase in PM<sub>10</sub>, however, when components of the PWM/GWR Project are implemented together (with overlapping construction schedules) the PWM/GWR Project would result in an increase in PM<sub>10</sub> that would contribute to the Monterey Bay Air Resources District (MBARD) nonattainment status for PM<sub>10</sub>. Thus, the PWM/GWR EIR identified a potentially significant air quality effect associated with the implementation of the PWM/GWR Project and related project components. This impact would be mitigated to less than significant levels by the implementation of Mitigation Measure AQ-1: Construction Fugitive Dust Control Plan (see **Appendix B, Table 1** for full text of mitigation).

The PWM/GWR EIR determined that construction and operation of the AWT Facility and the RUWAP Alignment Product Water Conveyance Pipeline would have a less than significant effect due to the emission of criteria pollutants, exposure of sensitive receptors, construction GHG emissions, and generation of odors. The PWM/GWR EIR also determined that the impacts during operations would be less than significant (see Table 4.3-5 Summary of Impacts – Air Quality and Greenhouse Gas, p. 4.3-21 of the PWM/GWR EIR). The PWM/GWR EIR found that the PWM/GWR Project would have less than significant cumulative impacts to greenhouse gas emissions and related climate change impacts. The PWM/GWR EIR found cumulative impacts related to criteria pollutants emission to be significant, but potential cumulative effects would be reduced to a less than significant level through the implementation of Mitigation Measure AQ-1: Construction Fugitive Dust Control Plan (see **Appendix B, Table 1** for full text of mitigation)<sup>15</sup>.

**RUWAP EIR and Addenda:** Addendum No. 3 to the RUWAP EIR fully evaluated the shared Product Water Conveyance Pipeline and construction of the Blackhorse Reservoir sized to accommodate both the RUWAP and PWM/GWR Project. The RUWAP EIR and Addenda found that the RUWAP air quality emissions would be less than the air emissions inventory for the Basin, and less than the MBARD thresholds of significance. The project analyzed in Addendum No. 2 included up to approximately 127,000 linear feet of pipeline, two pump stations, a 1.5 MG tank, and a 6 AF operational storage reservoir. The reduction in the overall length of pipeline and elimination of the seasonal storage reservoir

<sup>&</sup>lt;sup>15</sup> All applicable mitigation measures are being implemented by MRWPCA.

as reported in Addendum No. 3 resulted in an overall reduction in emissions and air quality impacts. Based on the overall reduction in construction activity and operational equipment use, and the associated reduction in emissions of air pollutants for the shared Product Water Conveyance Pipeline, construction and operation were found to have less than significant cumulative air quality impacts. See Air Quality, Addenda No. 3 Section 3.3, p.16). Furthermore, the RUWAP EIR and Addenda found that adverse air quality impacts related to construction of the RUWAP would be reduced to less than significant with implementation of the mitigations identified in **Appendix B, Table 2**.

# **Proposed Project Modifications**

The proposed project modifications would not change the physical location or increase the size of the PWM/GWR or RUWAP structures or facilities. In addition, the proposed project modifications would eliminate redundant project facilities thereby decreasing the extent of potential temporary construction related effects, as well as operational air quality effects. Accordingly, the proposed project modifications would result in a net reduction in temporary construction-related air quality effects. Previously approved mitigation measures intended to minimize temporary construction-related air quality impacts would continue to be applicable, thereby ensuring that impacts would be less than significant.

The proposed project modifications would result in an incremental increase in electricity use due to the operation of the higher peak production AWT Expanded Capacity Facility and pumping at the AWT Facility to deliver additional purified recycled water to MCWD customers. However, as noted previously, the modifications described above also would eliminate some facilities that no longer would be necessary due to the use of shared conveyance facilities. In particular, the previously approved RUWAP Alignment booster pump no longer would be needed. The incremental increase in electricity demand at the AWT Expanded Capacity Facility would be accommodated through the purchase of energy produced by the Monterey Regional Waste Management District (MRWMD). As detailed in **Table 3**, MRWPCA and the MRWMD have identified that there is sufficient available energy generated at the Monterey Regional Landfill to accommodate the incremental increase in energy demand associated with the proposed AWT Expanded Capacity Facility. The use of electricity generated by landfill gas recovery systems in lieu of the use of electricity produced by PG&E would reduce the net GHG emissions attributable to the PWM/GWR project, with the modifications considered in this Addendum, such that the modifications would not result in a net increase in GHG emissions. The use of electricity generated by landfill gas is not considered a new source of GHG emissions.

The proposed project modifications would not result in any new operational air quality or greenhouse gas effects. Therefore, the proposed project modifications would not change the PWM/GWR EIR's conclusion that all project impacts would be mitigated to less than significant with implementation of required mitigation measures.

# Conclusion

In conclusion, the proposed project modifications would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect for air quality and greenhouse gases. The findings of the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

# **Biological Resources**

# Summary of Impacts in Previous Documents

PWM/GWR EIR: The Biological Resources: Fisheries section in the PWM/GWR EIR (Vol 1 p. 4.4-1 – 4.4-76) found that the PWM/GWR Project would not conflict with local policies protecting fishery resources or conflict with habitat conservation plan or natural conservation community plan. However, the PWM/GWR EIR found that the PWM/GWR Project would have a potentially significant impact to fisheries resources due to habitat modification during construction of the diversion facilities. The PWM/GWR EIR, however, determined that these potentially significant impacts could be mitigated through the implementation of Mitigation Measure BT-1: Implement Construction Best Management Practices, Mitigation Measure BF-1a: Construction during Low Flow Season, Mitigation Measure BF-1b: Relocation of Aquatic Species during Construction, and Mitigation Measure BF-1c: Tidewater Goby and Steelhead Impact Avoidance and Minimization (see Appendix B, Table 1 for mitigation measures). The PWM/GWR EIR also found that there would be a potentially significant impact due to interference with fish migration during operation of the PWM/GWR Project, but this impact would be mitigated through implementation of either Mitigation Measure BF-2a: Maintain Migration Flows, or Mitigation Measure Alternate BF-2a: Modify San Jon Weir (see Appendix B, Table 1 for mitigation measures). In addition, the PWM/GWR EIR found that the PWM/GWR Project would have a less than significant impact in reducing fish habitat or fish populations due to project operation. The PWM/GWR EIR found that the PWM/GWR Project would have less than significant cumulative fisheries resources impacts.

The Biological Resources: Terrestrial section in the PWM/GWR EIR (Vol 1 p. 4.5-1 – 4.5-122) found that the PWM/GWR Project would have potentially significant impacts to terrestrial resources during project construction due to impacts to special-status species and habitat, sensitive habitats, and conflicts with local policies. These impacts would be mitigated to a less than significant level with implementation of mitigation measures identified in the PWM/GWR EIR, and further outlined below.

The PWM/GWR EIR project study area and evaluation considered: 1) federally listed or proposed species which are known to, or have the potential to, occur in the Project area; 2) a list of threatened and endangered species with the potential to be affected by the PWM/GWR Project provided by the USFWS; 3) the California Natural Diversity Database (CNDDB) occurrence reports; and, 4) and other materials and information provided in the PWM/GWR EIR, Tables 4.5-3 and 4.5-4. Three federally threatened species are known or likely to occur within the PWM/GWR Project area: Monterey spineflower, Monterey gilia, and the California red-legged frog. No state-listed species are known or likely to occur within the PWM/GWR Project area. There are no areas of designated critical habitat in the PWM/GWR Project area for terrestrial biological resources. One species for proposed listing, the tricolored blackbird, may occur within the PWM/GWR Project area. Several migratory bird species protected by the Migratory Bird Treaty Act (MBTA) also have the potential to nest and forage within the PWM/GWR Project area, including the white-tailed kite, California horned lark, and burrowing owl.

The PWM/GWR EIR found that the PWM/GWR Project would have a potentially significant impact on sensitive habitats during operation, but this potentially significant impact could be mitigated (see discussion of listed species, impacts and mitigation measures in the paragraph below). The PWM/GWR EIR found that construction of the PWM/GWR Project would have less than significant impacts to terrestrial resources due to the movement of native wildlife and native wildlife nursey sites, and conflicts with local polices, ordinances, or approved habitat conservation plan. Furthermore, the PWM/GWR EIR found that operation of the PWM/GWR Project would have less than significant impacts to terrestrial resources due to impacts to special-status species, sensitive habitats, movement of native wildlife and to

native wildlife nursey sites, and with conflicts with local policies, ordinances, or approved habitat conservation plan.

The PWM/GWR EIR found that all potentially significant impacts to biological resources could be mitigated through implementation the following mitigation measures BT-1a though BT-1q and BT-2a though BT-4 (see **Appendix B, Table 1** for full text of these mitigation measures):<sup>16</sup>

- Mitigation Measure BT-1a: Implement Construction Best Management Practices;
- Mitigation Measure BT-1b: Implement Construction-Phase Monitoring;
- Mitigation Measure BT-1c: Implement Non-Native, Invasive Species Controls;
- Mitigation Measure BT-1d: Conduct Pre-Construction Surveys for California Legless Lizard;
- Mitigation Measure BT-1e: Prepare and Implement Rare Plant Restoration Plan to Mitigate Impacts to Sandmat Manzanita, Monterey Ceanothus, Monterey Spineflower, Eastwood's Goldenbush, Coast Wallflower, and Kellogg's Horkelia;
- Mitigation Measure BT-1f: Conduct Pre-Construction Protocol-Level Botanical Surveys
  within the Product Water Conveyance: Coastal Alignment Option between Del Monte
  Boulevard and the Regional Treatment Plant site on Armstrong Ranch; and the remaining
  portion of the Project Study Area within the Injection Well Facilities site;
- Mitigation Measure BT-1g; Conduct Pre-Construction Surveys for Special-Status Bats;
- Mitigation Measure BT-1h: Implementation of Mitigation Measures BT-1a and BT-1b to Mitigate Impacts to the Monterey Ornate Shrew, Coast Horned Lizard, Coast Range Newt, Two-Striped Garter Snake, and Salinas Harvest Mouse;
- Mitigation Measure BT-1i: Conduct Pre-Construction Surveys for Monterey Dusky-Footed Woodrat;
- Mitigation Measure BT-1j: Conduct Pre-Construction Surveys for American Badger;
- Mitigation Measure BT-1k: Conduct Pre-Construction Surveys for Protected Avian Species, including, but not limited to, white-tailed kite and California horned lark;
- Mitigation Measure BT-11: Conduct Pre-Construction Surveys for Burrowing Owl;
- Mitigation Measure BT-1m: Minimize effects of nighttime construction lighting;
- Mitigation Measure BT-1n: Mitigate Impacts to Smith's blue butterfly;
- Mitigation Measure BT-1o: Avoid and Minimize Impacts to Monarch butterfly;
- Mitigation Measure BT-1p: Avoid and Minimize Impacts to Western Pond Turtle;
- Mitigation Measure BT-1q: Avoid and Minimize Impacts to California Red-Legged Frog;
- Mitigation Measure BT-2a: Avoidance and Minimization of Impacts to Riparian Habitat and Wetland Habitats;
- Mitigation Measure BT-2b: Avoidance and Minimization of Impacts to Central Dune Scrub Habitat;
- Mitigation Measure BT-2c: Avoidance and Minimization of Construction Impacts Resulting from Horizontal Directional Drilling under the Salinas River; and,
- Mitigation Measure BT-4. HMP Plant Species Salvage.

More specifically, the PWM/GWR EIR found that temporary disturbance may occur to foraging tricolored blackbirds or migratory birds during construction activities or if construction occurs during nesting

<sup>&</sup>lt;sup>16</sup> The PWM/GWR EIR found that potential effects on the California red-legged frog and other amphibious species would be reduced to less than significant levels through the implementation of mitigation measures including, for example, by scheduling activities at certain times during the year, keeping the disturbance footprint to a minimum, and monitoring (Mitigation Measure BT-1q). MRWPCA will also implement the terms and conditions of the U.S. Fish and Wildlife Biological Opinion that will further reduce effects on the California red-legged frog.

season. These impacts to migratory birds would be reduced to less than significant levels through mitigation including pre-construction surveys for protected avian species (Mitigation Measures BT-1k, BT-11), implementation of suitable buffers from nesting birds (Mitigation Measure BT-1k), avoidance and minimization of impacts to riparian habitat and wetland habitats (Mitigation Measure BT-2a). The PWM/GWR EIR found that construction may adversely affect, either directly or through habitat modification, special-status plant and wildlife species and their habitat within the PWM/GWR Project area, and that PWM/GWR Project construction and operations may also adversely affect sensitive habitats (including riparian, wetlands, and/or other sensitive natural communities) within the PWM/GWR Project area. The PWM/GWR EIR concluded impacts would be mitigated to less than significant with implementation of mitigation measures, including but not limited to: (1) implementation of construction Best Management Practices (Mitigation Measure BT-1a); (2) implementation of construction phase monitoring (Mitigation Measure BT-1b); (3) implementation of non- native, invasive species controls (Mitigation Measure BT-1c); (4) pre-construction surveys for special status and protected species (Mitigation Measures BT-1d, BT-1f, BT-1g, BT-1j, and BT-1k); and (5) the preparation and implementation of a rare plant restoration plan (Mitigation Measure BT-1e). Per PWM'GWR EIR Table 4.5-6, PWM/GWR Project construction would potentially conflict with local policies or ordinances protecting biological resources if there is an impact to the Habitat Management Plan (HMP) plant species within the PWM/GWR Project study area on the former Fort Ord (and seed salvage is not conducted), since those impacts do not require a take authorization from US Fish and Wildlife Service (USFWS) or California Department of Fish and Wildlife (CDFW). Those HMP plant species include Monterey spineflower, sandmat manzanita, Monterey ceanothus, and Eastwood's goldenbush (see PWM/GWR EIR, Table 4.5-7 HMP Species and Habitats Identified within the Project Study Area on the former Fort Ord). The PWM/GWR EIR found that any effects resulting from the conflict described above would be reduced to less than significant with mitigation. Specifically, Mitigation Measure BT-4 requires that for impacts to the HMP plant species within the PWM/GWR Project study area that do not require take authorization from USFWS or CDFW, salvage efforts for these species will be evaluated by a qualified biologist per the requirements of the HMP and Biological Opinion (see PWM/GWR EIR Mitigation Measure BT-4) in Appendix B, Table 1.The PWM/GWR EIR found that the PWM/GWR Project would have less than significant cumulative impacts to fisheries and terrestrial resources.

Furthermore, the PWM/GWR EIR found that the AWT Facility would have no substantial adverse impact to terrestrial resources from construction impacts to special-status species and habitat or construction conflicts with local policies, ordinances, or approved Habitat Conservation Plan. The PWM/EIR found that these impacts would be potentially significant impact for the RUWAP Alignment Product Water Conveyance Pipeline, but would be mitigated to a less than significant level with implementation of the mitigation measures identified above. The PWM/GWR EIR found that the AWT Facility and RUWAP Alignment Product Water Conveyance Pipeline would have a less than significant impact to terrestrial resources from construction and operational impacts to sensitive habitat, construction and operational impacts due to movement of native wildlife and native wildlife nursery sites, operational impacts to special status species and habitat and operational impacts due to conflicts with local policies, ordinances, or approved Habitat Conservation Plan.

**RUWAP EIR and Addenda:** Addendam No. 3 to the RUWAP EIR fully evaluated shared Product Water Conveyance Facilities. The RUWAP EIR, and Addenda No. 1, 2 and 3 addressed the potential biological impacts associated with the pipeline from the AWT Facility, through MCWD owned property (north of the City of Marina incorporated limits) consistent with the proposed shared Product Water Conveyance Pipeline. As noted, the overall area of disturbance would be reduced from the analysis in the RUWAP EIR and Addenda 1 and 2. See Biological Resources: Terrestrial, RUWAP EIR and Addenda (Addenda

No. 3 Section 3.4, p.17). Furthermore, the RUWAP EIR and Addenda found that biological impacts related to construction and operation of the shared Product Water Conveyance Facilities would be reduced to less than significant with implementation of the RUWAP EIR mitigation measures listed in **Appendix B, Table 2**. Also, the USFWS issued its Biological Opinion and completed the Section 7 process for the RUWAP in 2009<sup>17</sup>.

#### **Proposed Project Modifications**

The proposed modifications to the project would not change the physical location or increase the size of the PWM/GWR or RUWAP structures or facilities. In addition, the proposed project modifications would eliminate the need for some previously approved Product Water Conveyance facilities, thereby decreasing the extent of potential terrestrial biological resource-related effects<sup>18</sup>. All construction for the AWT Expanded Capacity Facility would occur within the fence line of the previously approved AWT Facility at the RTP. The overall reduction of planned facilities and elimination of 40,000 linear feet of pipeline construction would reduce the impacts to aquatic and terrestrial resources and reduce adverse effects of habitat modification, impacts to special-status plant and wildlife species and their habitat within the project area. Construction and operation of the proposed modifications to the project would not 1) have a new or substantially more severe adverse effect, either directly or through habitat modifications on candidate, sensitive, or special status species or riparian habitat or other sensitive natural community identified in local or regional plans and policies, 2) have a new or substantially more severe adverse effect on federally protected wetlands 3) have a new or substantially more severe effect due to interference with the movement of native migratory fish or wildlife species or with established wildlife corridors, 4) have a new or substantially more severe effect due to a conflict with any local policies or ordinances protecting biological resources, 5) have a new or substantially more severe effect due to a conflict with any conservation Plan, or other approved local, regional, or state habitat conservation plan. Based on the overall reduction in construction activity, and the associated elimination of facilities, construction and operation of the proposed project modifications would not result in any new or substantially more severe adverse environmental effects to biological resources beyond those identified in the above referenced environmental documentation. Therefore, the project modifications would not change the PWM/GWR EIR's conclusion that all project impacts would be mitigated to less than significant with implementation of approved mitigation measures.

#### Conclusion

In conclusion, the proposed Project modifications would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect relative to biological resources. The findings of the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

<sup>&</sup>lt;sup>17</sup> Biological Opinion dated November 24, 2009 from Diane K. Noda at the United States Department of Interior, Fish and Wildlife Service (USFWS) to Michael Kinsey regarding Formal Consultation under Section 7(a)(2) of the Endangered Species Act for MCWD RUWAP.

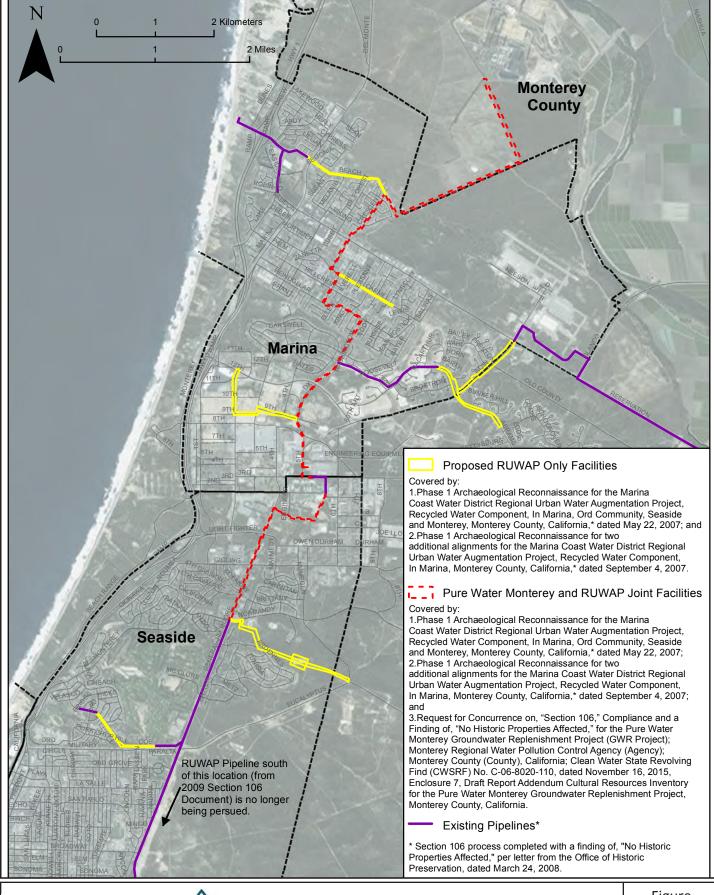
<sup>&</sup>lt;sup>18</sup> The PWM/GWR EIR found no impacts associated with the AWT Facility and RUWAP Product Water Conveyance Pipeline on fisheries resources (see Table 4.4-9 Summary of Impacts – Biological Resources: Fisheries, p. 4.4-44 of the Final EIR).

#### **Cultural and Paleontological Resources**

#### Summary of Impacts in Previous Documents

PWM/GWR EIR: The Cultural and Paleontological Resources section in the PWM/GWR EIR (Vol 1 p. 4.6-1 – 4.6-36) found that the PWM/GWR Project would not have any substantial adverse impacts due to operation of the PWM/GWR Project. The PWM/GWR EIR found that the PWM/GWR Project could result in a potentially significant impact due to construction impacts on historical resources for the Monterey Pipeline; this impact could be mitigated with the implementation of Mitigation Measure CR-1: Avoidance and Vibration Monitoring for Pipeline Installation in the Presidio of Monterey Historic District, and Downtown Monterey (see Appendix B, Table 1 for full text of mitigation measure) (see Table 4.6-6 Summary of Impacts - Cultural and Paleontological Resources, p. 4.6-24 of the Final EIR). In addition, the PWM/GWR EIR found that construction of the PWM/GWR Project could result in a potentially significant impact to archaeological resources or human remains. This potentially significant impact would, however, be mitigated with the implementation of Mitigation Measure CR-2a: Archaeological Monitoring Plan, Mitigation Measure CR-2b: Discovery of Archaeological Resources or Human Remains, and Mitigation Measure CR-2c: Native American Notification (see Appendix B, Table 1 for full text of mitigation). The PWM/GWR EIR found that construction of the PMW/GWR Project would have a less than significant impact on unknown paleontological resources. Furthermore, the PWM/GWR EIR found that the PWM/GWR Project would have no significant cumulative construction or operational cultural resources impacts.

RUWAP EIR and Addenda: The RUWAP EIR and Addenda fully evaluated shared Product Water Conveyance Facilities. The overall area of disturbance associated with the shared Project Water Conveyance Facilities represents a reduction in disturbance as compared to the analysis in the RUWAP EIR and Addenda 1 and 2. See Cultural Resources, RUWAP EIR and Addenda (Addenda No. 3 Section 3.6, p.18). The RUWAP EIR and Addendum No. 3 found that any adverse cultural resources impacts related to construction would be reduced to less than significant levels with implementation of the RUWAP mitigation measures listed in Appendix B, Table 2. Additionally, an Environmental Assessment was prepared by the Bureau of Reclamation as the NEPA lead agency and MCWD as a cooperating agency. In parallel with the NEPA process, the Bureau of Reclamation coordinated with MCWD and MRWPCA and the relevant federal agencies to comply with the National Historic Preservation Act (16 USC 470 et seq.). The Section 106 consultation process was completed on March 23, 2008 when the California State Historic Preservation Officer concurred with the U.S. Bureau of Reclamation's finding that the RUWAP would have no effect on historic properties. The EA was completed in 2009 and the U.S. Bureau of Reclamation signed the Finding of No Significant Impact (FONSI) completing the required NEPA compliance process. See Figure 10, RUWAP Cultural Resources Consultation Map identifying areas of Section 106 compliance for the RUWAP.







#### **Proposed Project Modifications**

The potential cultural and paleontological effects associated with the construction and operation of the PWM/GWR Project and the RUWAP have been extensively evaluated as part of prior environmental review and supporting technical documentation that fully evaluated all areas of ground-disturbance. The supporting technical analysis determined the project area did not contain any known cultural resources that could be affected by the AWT Facility and Product Water Conveyance Facilities (Breschini 2007; Doane and Breschini 2015, and Pacific Legacy, 2015, per the PWM/GWR EIR and RUWAP EIR and their Addenda). Prior cultural resource surveys and completed consultations with Native Americans and the State of California Office of Historic Preservation covered the entire width and depth of construction activities associated with the project modifications.

The proposed project modifications would not change the physical location or increase the size of the RUWAP or PWM/GWR structures or facilities. All construction for the AWT Expanded Capacity Facility would occur within the approved AWT Facility site. In addition, the proposed project modifications would eliminate some of the previously approved portions of the Product Water Conveyance Pipeline thereby decreasing the extent of potential effects on unknown cultural resources. Based on the overall reduction in construction activity and the associated elimination of some project facilities, construction of the proposed project modifications would not result in any new or substantially more severe adverse environmental effects to cultural and paleontological resources beyond those identified in the above referenced environmental documentation. No mitigation is required beyond the approved mitigation measures. See **Appendix B, Tables 1 and 2**.

The modifications to the project would also not result in any additional environmental effects to Tribal Cultural Resources (TRC). A separate Section 106 process was conducted for the PWM/GWR Project for all project components. The completed Section 106 process for PWM/GWR Project involved consultations with the State Historic Preservation Officer, tribes, and other identified consulting and interested parties in 2015 consistent with federal law. The APE included all project areas and elements included in **Figure 2** and **Figure 10**.

Based on the overall reduction in construction activity, and the associated elimination of some Product Water Conveyance facilities, the project modifications would not result in any new or substantially more severe adverse environmental effects to cultural resources beyond those identified in the above referenced environmental documentation. Therefore, the project modifications would not change the PWM/GWR EIR's conclusion that all project impacts would be mitigated to less than significant with implementation of required mitigation measures.

#### Conclusion

In conclusion, the proposed Project modifications would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect relative to cultural resources. The findings of the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

#### **Energy and Mineral Resources**

#### Summary of Impacts in Previous Documents

**PWM/GWR** EIR: The PWM/GWR EIR (Vol 1 p. 4.7-1 – 4.7-22) found that the PWM/GWR Project would not require or result in the construction of new electrical generation and/or transmission facilities; require or result in the expansion of existing facilities; or result in the loss of availability of locally-important

mineral resource recovery site delineated on a local general plan, specific plan or other land use plan. The PWM/GWR EIR found that operation of the PWM/GWR Project would result in less than significant impacts due to energy use or availability of mineral resources. However, the PWM/GWR EIR found that construction of the PWM/GWR Project could potentially result in significant impacts due to temporary energy use; these impacts could be mitigated through implementation of Mitigation Measure EN-1: Construction Equipment Efficiency Plan (see **Appendix B, Table 1** for mitigation measure). The PWM/GWR EIR found that the PWM/GWR Project would result in less than significant cumulative energy impacts and would not result in a substantial adverse cumulative impact to mineral resources. No additional impacts were identified in the PWM/GWR EIR for the construction and operation of the AWT Facility or RUWAP Project Water Conveyance Pipeline (see Table 4.7-2 Summary of Impacts - Energy and Mineral Resources, p. 4.7-11 of the PWM/GWR EIR).

**RUWAP EIR and Addenda:** Per Addendum No. 3 to the RUWAP EIR, the projected area of disturbance and related construction impacts would be reduced. No impacts to mineral resources were found in the RUWAP EIR and Addenda. Energy resources were not evaluated as a part of RUWAP EIR and Addenda.

#### **Proposed Project Modifications**

The proposed project modifications would not change the physical location or increase the size of the PWM/GWR or RUWAP structures or facilities. In addition, the proposed project modifications would eliminate redundant product water conveyance facilities thereby decreasing the extent of potential temporary construction related effects. Therefore, the proposed modifications would lessen the extent of potential temporary construction effects related to increased energy use. As a result, these modifications would not result in any new or substantially more severe construction-related effects. Approved mitigation identified in the above referenced environmental documentation would continue to ensure that impacts would be minimized. No additional mitigation would be necessary to address temporary construction-related effects.

The Project modifications would result in an incremental increase in energy (electricity) use associated with the AWT Facility due to the operation of the higher peak production capacity AWT Expanded Capacity Facility and pumping by the product water pump station at the AWT Facility to deliver purified recycled water to MCWD customers. As discussed above, the incremental increase in energy demand associated with AWT Expanded Capacity Facility operation would be accommodated through the purchase of energy produced by the MRWMD. As detailed in **Table 3**, **Overview of Proposed Project Electricity Demand** (updated PWM/GWR EIR Table 2.11), MRWPCA and the MRWMD have identified that there is sufficient available energy generated at the Monterey Regional Landfill to accommodate the incremental increase in energy demand associated with the proposed AWT Expanded Capacity Facility. As a result, the incremental increase in energy demand associated with AWT Facility operation would be off-set through the use of energy generated at the adjacent landfill. Therefore, the proposed modifications would not result in any new operational energy demand that would: 1) require or result in the construction of new electrical generation and/or transmission facilities; or, 2) require or result in the expansion of existing facilities. Existing energy sources are available to accommodate the incremental increase in operational energy demand associated with AWT Expanded Capacity Facility operation.

#### Conclusion

In conclusion, the proposed Project modifications would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect. The findings of the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

#### **Geology and Soils**

#### Summary of Impacts in Previous Documents

**PWM/GWR EIR:** The PWM/GWR EIR (Vol 1 p. 4.8-1 – 4.8-52) found that the PWM/GWR Project would not have a substantial adverse impact to septic system soil suitability. In addition, the PWM/GWR EIR found that the PWM/GWR Project would have less than significant impacts associated with construction-related erosion or loss of topsoil, construction-related soils collapse and soil constraints during pipeline trenching, operational exposure to fault rapture, operational exposure to seismic ground shaking and liquefaction, operation hydro-collapse of soils from well injection, and operational exposure to expansive and corrosive soils. The PWM/GWR EIR found that operation of the PWM/GWR Project would have a potentially significant impact to coastal erosion and sea level rise due to the operation of the Coastal Pipeline Alignment. This alignment was not, however, ultimately selected. Therefore, this potentially significant impact was avoided. Finally, the PWM/GWR EIR found that construction and operation the PWM/GWR Project would have a less than significant cumulative impact to geology, seismicity or soils. No additional impacts were identified in the PWM/GWR EIR for the construction and operation of the AWT Facility or RUWAP Product Water Conveyance Pipeline(see Table 4.7-2 Summary of Impacts - Geology and Soils, p. 4.8-28 of the Final EIR).

**RUWAP EIR and Addenda:** Addendum No. 3 to the RUWAP EIR fully evaluated shared Product Water Conveyance Facilities. The overall area of disturbance would be reduced from the analysis in the RUWAP EIR and Addenda 1 and 2. The RUWAP EIR and Addenda found that all geology and soils impacts related to construction of the RUWAP would be reduced to less than significant with implementation of the RUWAP EIR mitigation measures listed in **Appendix B, Table 2**. See also Geotechnical and Geological Hazards, RUWAP EIR and Addenda (Addendum No. 3 Section 3.7, p.20).

#### **Proposed Project Modifications**

The proposed project modifications would not change the physical location or increase the size of the PWM/GWR or RUWAP structures or facilities. In addition, the proposed project modifications would eliminate redundant project facilities thereby decreasing the extent of potential temporary construction related effects. As a result, the proposed modifications would lessen the extent of potential geology and soils related effects due to ground-disturbing activities (e.g., soil erosion, etc.). No new or substantially more severe geology and soils effects would occur due to the proposed modifications. Adopted mitigation identified in the above referenced environmental documentation would continue to ensure that impacts would be minimized. No additional mitigation would be necessary.

#### Conclusion

In conclusion, the proposed Project modifications would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect. The findings of the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

#### **Hazards and Hazardous Materials**

#### Summary of Impacts in Previous Documents

**PWM/GWR EIR:** The PWM/GWR EIR (Vol 1 p. 4.9-1 – 4.9-58) found that construction or operation of the PWM/GWR Project would not: 1) result in hazardous emissions within 0.25-miles of an existing or proposed school; 2) be located within an area covered by an airport land use plan or within the vicinity of

a private airstrip; 3) impair implementation of or physically interfere with an adopted emergency response plan or emergency evacuation plan; and, 4) expose people or structure to a significant risk of loss, injury, or death involving wildland fires. The PWM/GWR EIR found that the PWM/GWR Project would result in potentially significant impacts due to the accidental release of hazardous materials during construction. The PWM/GWR EIR found these potentially significant impacts would be mitigated with the implementation of Mitigation Measure HH-2a: Environmental Site Assessment, Mitigation Measure HH-2b: Health and Safety Plan, and Mitigation Measure HH-2c: Materials and Dewatering Disposal Plan (see **Appendix B, Table 1** for full text of these mitigation measures). (Also see Table 4.9-4 Summary of Impacts - Hazards and Hazardous Materials, p. 4.9-32 of the Final EIR). Finally, the PWM/GWR EIR found that the PWM/GWR Project would result in less than significant cumulative impacts related to hazards or hazardous materials.

RUWAP EIR/Addenda: The RUWAP EIR and Addenda (including changes described in the Shared Pipeline Addendum No. 3) found the shared product water conveyance facilities would not result in significant increased environmental effects from hazardous materials or increase the severity of environmental impacts already identified in the PWM/GWR EIR and the RUWAP EIR. Addendum No 3 noted all impacts would be reduced to less than significant levels through the implementation of mitigation measures described in the RUWAP EIR as modified in the Addenda. See Hazards and Hazardous Materials, RUWAP EIR and Addenda (Addendum No. 3 Section 3.8, p.20).

#### **Proposed Project Modifications**

The modifications to the project would not alter the physical location, footprint, or increase the required amount of chemicals substantially for the PWM/GWR Project. The updated types and amounts of chemicals that would be stored at the AWT Facility are shown in the Appendix A, Updated Table 4.9-6, Chemicals to be Utilized at the Advanced Water Treatment Facility. These are consistent with the chemicals and amounts evaluated for the approved PWM/GWR Project. Bulk storage of these chemicals would be located in tanks within the RTP site. The chemical storage and handling systems at the AWT Facility would be designed and constructed in accordance with specific requirements for the safe storage and handling of hazardous materials set forth in the Uniform Fire Code, Article 80. Requirements specifically applicable to the project include spill control in all storage, handling and dispensing areas, separate secondary containment for each chemical storage system, and separation of incompatible materials with a non-combustible partition. These requirements reduce the potential for a release of hazardous materials that could pose a public health or water quality risk. Additionally, the implementation of mitigation measures identified above would be required. The AWT Expanded Capacity Facility would not result in new or substantially more severe impacts related to the use and disposal of hazardous materials during construction or operation, accidental release of hazardous materials during construction or operation, use of hazardous materials during construction within 0.25miles of schools, or wildland fire hazard during construction. Modifications to the project do not alter the conclusions from the previous environmental documentation that required mitigation within the adopted PWM/GWR EIR MMRP (see Appendix B, Table 1) would reduce all hazards and hazardous materials impacts to less than significant.

#### Conclusion

In conclusion, the proposed Project modifications would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect relative to hazards and hazardous materials. All potential project-related effects would remain unchanged and no new or additional mitigation measures would be warranted.

#### **Hydrology and Water Quality**

#### Summary of Impacts in Previous Documents

PWM/GWR EIR: The Hydrology and Water Quality: Groundwater section in the PWM/GWR EIR (Vol 1 p. 4.10-1 – 4.10-94) found the PWM/GWR Project would result in beneficial impacts to both groundwater levels and overall quality in the Salinas Valley Groundwater Basin and the Seaside Basin (see Table 4.10-12 Summary of Impacts - Hydrology and Water Quality: Groundwater, p. 4.10-51 of the Final EIR). The PWM/GWR EIR found that the PWM/GWR Project would have a less than significant cumulative impacts to groundwater levels, recharge or storage in the Salinas Valley Groundwater Basin. The PWM/GWR EIR found construction of the PWM/GWR Project would not contribute to significant impacts to groundwater levels and groundwater quality. In addition, the PWM/GWR EIR found the operation of the PWM/GWR Project would have less than significant impacts to groundwater quality recharge, storage or quality in the Salinas Valley Groundwater Basin. The PWM/GWR EIR found that there would be no significant construction or operational impact to groundwater levels, recharge or storage in the Seaside Groundwater Basin nor would the PWM/GWR Project make a considerable contribution to cumulative impacts to groundwater quality in the Seaside Basin.

The Hydrology and Water Quality: Surface Water section in the PWM/GWR EIR (Vol 1 p. 4.11-1 – 4.11-122) found that the PWM/GWR Project would not have a substantial adverse impact related to a 100-year flood hazard area and would have beneficial operational impacts to Carmel River flows. The PWM/GWR EIR concluded there would be less than significant construction impacts to surface water due to surface water quality due to discharges; surface water quality due to earthmoving, drainage alterations, and use of hazardous materials. The PWM/GWR EIR found that the PWM/GWR Project would have less than significant operational impacts due to surface water quality due to well maintenance discharges; marine water quality due to ocean discharges; drainage pattern alterations; risks due to location within 100-year flood area; risks due to flooding due to levee/dam failure, or coastal inundation; seiche tsunami, or mudflow risk. The PWM/GWR EIR concluded that there would be a potentially significant impact on surface water hydrology and water quality during the construction of the source water diversions, however, this impact would be reduced to less than significant with the implementation of Mitigation Measure HS-4: Management of Surface Water Diversion Operations (see **Appendix B, Table 1** for full text of mitigation measure).

The PWM/GWR concluded there would be less than significant cumulative impacts to hydrology and water quality of inland surface waters and potentially significant cumulative impacts to marine water quality due to potential exceedance of the Ocean Plan water quality objectives for several constituents. However, the PWM/GWR EIR found this potentially significant cumulative impact would be mitigated with implementation of Mitigation Measure HS-C: Implement Measures to Avoid Exceedances over Water Quality Objectives at the Edge of the Zone of Initial Dilution (ZID) (see **Appendix B, Table 1** for full text of mitigation measure).

The PWM/GWR EIR analyzed impacts from the 4.0 mgd AWT Facility production of purified recycled water. As described, the AWT Facility would produce, among other things, reverse osmosis concentrate, which would be piped to a proposed new brine and effluent receiving, mixing, and monitoring facility. The reverse osmosis concentrate would be discharged through the existing MRWPCA outfall to Monterey Bay that runs from incorporated portions of Monterey County, ultimately reaching Monterey Bay in the City of Marina. The PWM/GWR EIR reported the current MRWPCA wastewater discharge is governed by NPDES permit R3-2014-0013 issued by the Central Coast Regional Water Quality Control Board (RWQCB). MRWPCA will need to obtain an amended permit or a new permit from the Central

Coast RWQCB to discharge the reverse osmosis concentrate. The Central Coast RWQCB considers compliance with the Water Quality Control Plan for Ocean Waters of California (Ocean Plan) which establishes water quality objectives and beneficial uses for waters of the Pacific Ocean adjacent to the California coast outside of estuaries, coastal lagoons, and enclosed bays. Additionally, the National Oceanic and Atmospheric Administration (NOAA) – Monterey Bay National Marine Sanctuary (MBNMS) will provide authorization and/or approval of the discharge permit.<sup>19</sup>

For the PWM/GWR EIR, Trussell Technologies performed water quality quantitative analysis of the PWM/GWR Project's ability to meet the Ocean Plan Water Quality objectives. Trussell Technologies conducted an analysis that estimated a worst-case water quality under five different operational scenarios for the wastewater that would be discharged through the ocean outfall and compared that discharge to the Ocean Plan objectives to determine whether there would be a significant effect on marine and ocean water quality. The results showed that the PWM/GWR Project would not result in a significant effect on ocean water quality because the wastewater discharged through MRWPCA's ocean outfall, including the PWM/GWR Project's reverse osmosis concentrate, would consistently meet the water quality objectives of the Ocean Plan. However, the PWM/GWR found that the combined operations of the PWM/GWR Project and the Monterey Peninsula Water Supply Project (MPWSP) could result in significant cumulative impacts from an exceedance of Ocean Plan water quality objectives. See PWM/GWR EIR Appendix V, Ocean Plan Compliance Assessment for the Monterey Peninsula Water Supply Project and Project Variant (herein referred to as the MPWSP/Variant Ocean Plan Assessment) (Trussell Technologies, 2015b).

The PWM/GWR EIR found the AWT Facility and RUWAP Product Water Conveyance Pipeline would not have a substantial adverse impact due to: 1) operational impacts to surface water quality due to well maintenance discharges; 2) operational surface water quality impacts due to source water diversions; 3) operational Carmel River flows; and, 4) operational risks due to flooding due to levee/dam failure, or coastal inundation; or operational seiche, tsunami, or mudflow risks. Furthermore, the PWM/GWR EIR found the AWT Facility would not have a substantial adverse operational impacts due to location within 100-year flood area, this is identified as a less than significant impact for the RUWAP Product Water Conveyance Pipeline (see Table 4.11-13 Summary of Impacts – Hydrology and Water Quality: Surface Water, p. 4.11-58 of the PWM/GWR EIR).

**RUWAP EIR and Addenda:** The RUWAP EIR and Addenda found that the RUWAP would result in less than significant impacts related to hydrology and water quality. Specifically, the Addendum No 3 identified less than significant impacts associated with ground disturbance and construction, as well as ground water impacts. Addendum No. 3 found these impacts would be reduced compared to the impacts identified in the RUWAP EIR and Addenda 1 and 2 and therefore no mitigation is required. See Hydrology and Water Quality, RUWAP EIR and Addenda (Addenda No. 3 Section 3.9, p. 20).

#### **Proposed Project Modifications**

The modifications to the project would not change the conclusions in the PWM/GWR EIR in terms of benefits and impacts to groundwater and surface water. The proposed Project modifications include an increase in *peak or maximum capacity* of the AWT Facility from 4.0 mgd to 5.0 mgd and an associated

<sup>&</sup>lt;sup>19</sup> MBNMS permit authorization is described in the Memorandum of Agreement dated April 2015 between NOAA MBNMS, USEPA, State Water Resources Control Board, Central Coast RWQCB, Association of Monterey Bay Area Governments (AMBAG), and the Coastal Commission.

increase in water supply yield of 600 AFY of purified recycled water for urban irrigation. This in turn requires application to the RWQCB and approval of an amended NPDES permit<sup>20</sup>.

For NPDES permitting and for this Addendum, Trussell Technologies performed an updated water quality quantitative analysis of the Expanded Capacity AWT Facility ability to meet the Ocean Plan Water Quality objectives. (Refer to the PWM/GWR EIR Appendices T, and U-1 and U-2). In both the 2015 and 2017 technical memoranda, the analysis estimated a worst-case water quality under different conservative operational scenarios for the waste stream that would be discharged through the ocean outfall and compared that discharge to the Ocean Plan objectives to determine whether there would be a significant effect on marine and ocean water quality. The technical analysis and updated results are provided in Appendices C and D (Appendix C, Trussell Tech September 2017 Ocean Plan Compliance Assessment for the PWM/GWR Project and Appendix D, Trussell Tech September 2017 Comparison of Dilution Results). These technical reports document that the Expanded Capacity AWT Facility would not result in a significant effect on ocean water quality because the waste stream discharged through MRWPCA's ocean outfall, including the reverse osmosis concentrate from the 5.0 mgd AWT Facility, would consistently meet the water quality objectives of the Ocean Plan. As a result of these updated technical studies, the planned capacity expansion of the AWT Facility operation and planned ocean discharges would have no operational marine water quality impacts due to ocean discharges.

Trussell Technologies also updated its technical analysis for the combined effect of the PWM/GWR project and the MPSWP. The updated technical memorandum continues to show that a significant effect could result from the MPSWP by itself, and from the combined discharges from the PWM/GWR Project and MPSWP. (See Appendix E, Trussell Technologies September 2017 Revised Ocean Plan Compliance Assessment for MPWSP and Project Variant) Therefore, the proposed project modifications would not result in any new or substantially more severe adverse environmental effects to hydrology and water quality beyond those previously identified in the existing environmental documentation. The project modifications also would not change the PWM/GWR EIR's conclusion that the cumulative significant impacts to marine resources would be reduced to less than significant with the implementation of Mitigation Measure HS-C/MR-C: Implement Measures to Avoid Exceedances over Water Quality Objectives at the Edge of the Zone of Initial Dilution.

In conclusion, the proposed Project modifications would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect relative to hydrology and water quality. The findings of the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

#### Land Use, Agricultural and Forest Resources

#### Summary of Impacts in Previous Documents

**PWM/GWR** EIR: The PWM/GWR EIR (Vol 1 p. 4.12-1 – 4.12-60) found the PWM/GWR Project would not have a substantial adverse impact related to: 1) physically dividing an established community; 2) conflicts with plans polices or regulations during construction; 3) the conversion of prime farmland, unique

<sup>&</sup>lt;sup>20</sup> MRWPCA is preparing application materials to modify its existing MRWPCA NPDES Permit per 40 Code of Regulations Part 122.62. MRWPCA, through Larry Walker Associates and Trussell Technologies, have conducted an extensive assessment in accordance with requirements specified by the RWQCB. Technical memoranda build upon the analysis conducted for the PWM/GWR EIR, and assess, among many items, the minimum probable initial dilution at the point of discharge based on likely discharge scenarios and any concomitant impacts on water quality and beneficial uses per the Ocean Plan.

farmland or farmland of statewide importance; 4) potential conflicts with existing zoning for agricultural use or, a Williamson Act contract; 5) conflicts with existing zoning for, or cause rezoning of, forest land, or timberland, or timberland zoned Timberland Production; 6) the loss of forest land or conversion of forest land to non-forest use; and, 7) conversion of forest to non-forest use due to other changes. The PWM/GWR EIR found the PWM/GWR Project would have less than significant impacts due to operational indirect farmland conversion (see Table 4.12-4, Summary of Impacts - Land Use, Agriculture, and Forest Resources, p. 4.12-34 of the PWM/GWR EIR). The PWM/GWR EIR found the PWM/GWR Project would have a potentially significant impact related to temporary farmland conversion during construction, but that this potentially significant impact would be mitigated with the implementation of Mitigation Measure LU-1: Minimize Disturbance to Farmland. (see Appendix B, Table 1 for full text of mitigation measure). However, these impacts due to temporary farmland conversion during construction were identified in the PWM/GWR EIR as less than significant for the RUWAP Product Water Pipeline and AWT Facility. In addition, the PWM/GWR EIR found the PWM/GWR Project would have a potentially significant impact related to consistency with plans policies and regulations, but that these potentially significant impacts could be mitigated to a less than significant level through implementation of mitigation measures identified in the PWM/GWR EIR (see Table 4.12-5, Mitigation Measures Required for Consistency with Policies).

RUWAP EIR and Addenda: The RUWAP EIR and Addenda found that the RUWAP would result in less than significant impacts to agricultural resources. Specifically, the pipeline alignment was revised to avoid existing areas of row crop production in Addendum No. 2. Addendum No. 3 found that shared Product Water Conveyance Pipeline would not result in new significant agricultural resources environmental effects or increase the severity of environmental impacts already identified in the RUWAP EIR or Addenda 1 and 2, see Agriculture Resources, RUWAP EIR and Addenda (Addenda No. 3 Section 3.2, p.16). Furthermore, the RUWAP EIR and Addenda found that the RUWAP would not result in a significant impact related to land use and planning, see Land Use and Planning, RUWAP EIR and Addenda (Addenda No. 3 Section 3.10, p.21).

#### **Proposed Project Modifications**

The proposed project modifications would not change the physical location or increase the size of the PWM/GWR or RUWAP structures or facilities. In addition, the proposed project modifications would eliminate redundant project facilities thereby decreasing the extent of potential project-related effects. As a result, the proposed project modifications would not result in new or substantially more severe significant impacts relating to land use, agriculture, and forest resources beyond those previously identified in the environmental documentation summarized above. All approved mitigation measures would be applicable to the proposed project modifications. No additional mitigation would be warranted.

#### Conclusion

In conclusion, the proposed Project modifications would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect relative to land use, agricultural and forest resources. The findings of the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

#### **Marine Biological Resources**

#### Summary of Impacts in Previous Documents

**PWM/GWR** EIR: The Marine Biology section in the PWM/GWR EIR (Vol 1 p. 4.13-1 – 4.13-32) found the PWM/GWR Project would not have a substantial adverse impact on any marine species identified as a candidate, sensitive, or special-status species in local or regional plans, policies, or regulations, or by the CDFW, USFWS, or NOAA Fisheries. Moreover, the PWM/GWR EIR also identified that the PWM/GWR Project would not: 1) conflict with the provisions of an adopted habitat conservation plan, natural community conservation plan, or other approved local, regional, or state habitat conservation plan governing the marine study area; and, 2) interfere substantially with the movement of any native resident or migratory fish or wildlife species or with established native resident or migratory wildlife corridors, or impede the use of native wildlife nursery sites. In addition, the PWM/GWR EIR found the PWM/GWR Project would result in less than significant operational impacts on marine biological resources.

The PWM/GWR EIR found the PWM/GWR Project would potentially make a considerable contribution to significant cumulative impacts to marine water quality due to the potential exceedance of the California Ocean Plan water quality objectives for several constituents; however, with implementation of Mitigation Measure MR-C, the impact would be reduced to less than significant and would not make a considerable contribution to a significant cumulative impact. Mitigation Measure MR-C would implement measures to avoid exceedances over water quality objectives at the edge of the Zone of Initial Dilution (ZID) (see **Appendix B, Table 1** for full text of mitigation measure; also see Table 4.13-2 Summary of Impacts - Marine Biology, p. 4.13-19 of the Final EIR).

**RUWAP EIR/Addenda:** The RUWAP EIR and Addenda found the RUWAP would not result in any significant adverse impacts to marine resources and identified a beneficial impact from the Project (reduction in the amount of effluent discharged from the MRWPCA treatment plant was considered to have an environmentally beneficial impact on marine resources). See Marine Resources, RUWAP EIR and Addenda (Addenda No. 3 Section 3.5, p.18).

#### **Proposed Project Modifications**

This analysis of impacts of the disposal of reverse osmosis concentrate on the marine biological resources in the Monterey Bay/Pacific Ocean focuses on the water quality changes that may occur in the vicinity of the MRWPCA ocean outfall. As described in the PWM/GWR EIR (Section 4.11, Hydrology and Water Quality: Surface Water), Flow Science modeled dilution factors for various combinations of source water flows and ocean climatic conditions, incorporating conservative assumptions regarding the MRWPCA ocean outfall, ocean conditions, and other factors that affect the dilution of wastewater in the area near the outfall's diffuser ports (i.e., the openings in the outfall through which discharges flow out). Trussell Technologies updated this analysis (See **Appendices C, D and E**). In addition to conservative assumptions about dilution characteristics of the discharge, numerous conservative assumptions were integrated into the approach for estimating the concentrations of contaminants in the reverses osmosis concentrate to be discharged into the MRWPCA ocean outfall.

Trussell Technologies in association with Larry Walker Associates performed water quality quantitative analyses for the AWT Expanded Capacity Facility's ability to meet the Ocean Plan Water Quality objectives addressing the waste stream effluent including reverse osmosis concentrate under anticipated worst-case scenario and conditions for the expanded capacity 5.0 AWT Facility and evaluated impacts on marine water quality. These water quality data were then combined for various discharge scenarios, and a concentration at the edge of the ZID was calculated for each constituent and scenario. Based on the

data, assumptions, modeling, and analytical methodology presented in the 2017 updated analysis conducted by Trussell Technologies and as reported in the 2017 technical memoranda, the Project, as modified, would comply with the Ocean Plan objectives, including toxicity of the discharges. The Proposed Project would have a less than significant impact related to toxicity of ocean discharges on marine resources. Trussell Technologies also updated the technical analysis for the combined effect of the PWM/GWR project and the MPSWP. The updated technical memorandum continues to show that a significant effect could result from the MPSWP by itself, and from the combined discharges from the PWM/GWR Project and MPSWP. (See Appendix E, Trussell Tech September 2017 Revised Ocean Plan Compliance Assessment for MPWSP and Project Variant)

The modeling for the AWT Expanded Capacity Facility (Trussell Technologies, September 2017), used the updated and actual configuration of the outfall with 130 ports open (i.e., 129 existing ports plus the open end gate replaced with one diffuser port). Under this Ocean Plan Compliance Assessment, the reverse osmosis (RO) concentrate flow was therefore increased from 0.94 MGD to 1.17 mgd, in relation to increasing purified water production capacity from 4 mgd to 5.0 mgd. Additionally, the original COP compliance analyses for the PWM/GWR Project (February 2015 and April 2015) modeled the end of the existing ocean outfall as an open pipe, which is the current configuration of the outfall. The September 2017 modeling work assumed that a 6-inch Tideflex valve was installed on the end of the outfall; this modification will occur prior to any discharge of desalination brine. The PWM/GWR Project will convey the combined effluent waste stream to the MRPWCA outfall for discharge 11,260 feet offshore and 100 feet below the ocean surface. The discharge point is well below the photic zone, and subject to dilution and mixing through buoyancy and tidal velocities. The conditions near the outfall are not conducive to algae growth or eutrophication, because the sunlight, geochemical, and hydrologic conditions that create algae blooms are not present (Larry Walker Associates, 2017). Thus, consistent with the findings of the PWM/GWR EIR, the project modification would result in less than significant operational impacts on marine biological resources.

The PWM/GWR EIR found the AWT Facility may have potential significant cumulative impacts on marine biological resources due to the potential exceedance of the Ocean Plan water quality objectives for several constituents. The proposed project modifications would not change the conclusion that these potentially significant impacts would be mitigated with implementation of Mitigation Measure MR-C: Implement Measures to Avoid Exceedances over Water Quality Objectives at the Edge of the Zone of Initial Dilution (ZID).

#### Conclusion

In conclusion, the modifications to the project would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect. The findings of the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

#### **Noise and Vibration**

#### Summary of Impacts in Previous Documents

**PWM/GWR** EIR: The PWM/GWR EIR (Vol 1 p. 4.14-1 – 4.14-72) found the majority of the facility components of the PWM/GWR Project would not have a substantial adverse impact related to excessive groundborne noise during construction, vibration during operation, or exposure to aircraft noise. The PWM/GWR EIR concluded that there would be a significant and unavoidable impact due to noise generated during construction of the Tembladero Slough diversion and Monterey Pipeline. Although the

impact may not be reduced to less than significant levels, implementation of Mitigation Measure NV-1a: Drilling Contractor Noise Measures, Mitigation Measure NV-1b: Monterey Pipeline Noise Control Plan for Nighttime Pipeline Construction, Mitigation Measure NV-1c: Neighborhood Notice, Mitigation Measure NV-1d: RUWAP Pipeline Construction Noise, Mitigation Measure NV-2a: Construction Equipment, & Mitigation Measure NV-2b: Construction Hours, would reduce the severity of the impact.

RUWAP EIR and Addenda: The RUWAP was approved to include additional facility components, including pump stations and motors, than the RUWAP Shared Pipeline Project considered under Addendum No. 3, as outlined above. Addendum No. 3 reduced the RUWAP proposed facilities and in particular, the distance of linear pipeline that must be installed or constructed in comparison with the RUWAP EIR. The RUWAP EIR and Addenda also included noise impacts from operation of the booster pump station near the intersection of 3rd Street and 5th Avenue in the City of Marina. See Noise, RUWAP EIR and Addenda (Addendum No. 3 Section 3.11, p.21). Required RUWAP noise mitigation restricts construction activity timeframes, requires compliance with noise ordinances of relevant local jurisdictions, and requires location of all stationary noise-generating equipment as far as possible from nearby noise-sensitive receptors. See Appendix B, Table 2.

#### **Proposed Project Modifications**

The proposed project modifications would not change the physical location or increase the size of the PWM/GWR or RUWAP structures or facilities. In addition, the proposed project modifications would eliminate redundant product water conveyance facilities. As a result, the proposed project modifications would reduce potential temporary construction related noise associated with the eliminated facilities. In addition, the proposed project modifications would also eliminate operational noise associated with the eliminated booster pump facilities. Although the proposed project modifications would increase the capacity of the AWT Facility, these modifications would not result in any increased noise impacts beyond those previously identified in the existing environmental documentation summarized above (see Table 4.14-11 Summary of Impacts – Noise and Vibration, p. 4.14-28 of the Final EIR). All existing mitigation measures would be implemented as part of the proposed project modifications. No additional mitigation would be warranted.

#### Conclusion

In conclusion, the modifications to the project would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect. The findings of the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

#### **Population and Housing**

#### Summary of Impacts in Previous Documents

**PWM/GWR EIR:** The PWM/GWR EIR (Vol 1 p. 4.15-1 – 4.15-12) found the PWM/GWR Project would not have a substantial adverse impact related to displacing housing units or displacing substantial numbers of people. In addition, the PWM/GWR EIR found the PWM/GWR Project would have less than significant impacts related to growth inducement. The PWM/GWR EIR found the PWM/GWR Project would have less than significant cumulative impacts related to population and housing (see Table 4.15-3 Summary of Impacts – Population and Housing, p. 4.15-6 of the PWM/GWR EIR).

RUWAP EIR and Addenda: The RUWAP EIR and Addenda found that the maximum quantity of recycled water that may be delivered to the MCWD customers (in the former Ord Community) would

not increase and therefore there would no adverse growth inducing impact or impact to population or housing. This is due to the restrictions of use of recycled water for only irrigation. As described in the RUWAP EIR and Addenda, in urban areas, recycled water would be used for irrigation of recreational areas, including public and private landscaped areas, school ball fields, parks, and golf courses. Additionally, the RUWAP EIR and Addenda all noted that new recreation areas or landscaped areas may be developed with the proposed recycled water; however, the new development potential would not increase due to the maximum limitations of recycled water to the MCWD customers in the Former Ord Community. The Addendum No. 3 and Addendum No. 2 both assumed a total of 1,720 AFY of recycled water (1,430 AFY for the former Ord Community and 140 AFY to Central Marina). See Population and Housing / Growth, RUWAP EIR and Addenda (Addenda No. 3 Section 3.12, p.21) and Growth Inducement, RUWAP EIR and Addenda (Addenda No. 3 Section 3.16, p.23).

#### **Proposed Project Modifications**

The proposed project modifications would not change the physical location or increase the size of the PWM/GWR or RUWAP structures or facilities. In addition, the proposed project modifications would eliminate redundant product water conveyance facilities. As a result, the proposed project modifications would not cause a new or substantially more severe impacts regarding displacement of a substantial number of existing housing units or people. In addition, the increase in peak capacity of the AWT Facility would not induce substantial population growth. The Expanded Capacity AWT Facility would provide an additional 600 AFY of purified recycled water for urban irrigation to MCWD customers<sup>21</sup>. The use of this water is restricted to irrigation and outdoor landscaping and would not be available to accommodate new growth and development such that a substantial growth inducing effect would occur. Therefore, the proposed project modifications would not result in any new or substantially more severe environmental effects beyond those previously identified in existing environmental documentation. In addition, no new mitigation measures would be warranted.

#### Conclusion

In conclusion, the modifications to the project would not: 1) result in any new significant environmental effects; or 2) substantially increase in severity of a previously identified significant effect pertaining to population and housing. The findings of the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

#### Public Services, Utilities, and Recreation

#### Summary of Impacts in Previous Documents

**PWM/GWR EIR:** The PWM/GWR EIR (Vol 1 p. 4.16-1 – 4.16-24) found the PWM/GWR Project would not have a substantial adverse impact related to increased use of existing parks causing deterioration of facilities, including schools, parks and recreational facilities, or require the construction or expansion of recreational facilities which might have an adverse physical effect on the environment. The PWM/GWR EIR found the PWM/GWR Project would have less than significant impacts due to demand for public services associated with the provision of new or physically altered governmental facilities (e.g., police, fire, etc.). The PWM/GWR EIR found that there would be a potentially significant impact during construction due to conflict with solid waste policies and regulations. The PWM/GWR EIR found that this impact would be reduced to less than significant level with the implementation of Mitigation Measure

<sup>&</sup>lt;sup>21</sup>MCWD and MRWPCA entered into a MOU in 2009 providing that MRWPCA and MCWD would supply up to 1,427 AFY to the RUWAP as discussed in the PWM/GWR EIR and RUWAP EIR.

PS-3: Construction Waste Reduction and Recycling Plan (see **Appendix B, Table 1** for mitigation measure). The PWM/GWR EIR found the PWM/GWR Project would have less than significant cumulative impacts related to schools, parks, recreational facilities, or other public services and utilities (fire and police protection, solid waste) (see Table 4.16-5 Summary of Impacts - Public Services, Utilities, and Recreation, p. 4.16-13 of the PWM/GWR EIR).

**RUWAP EIR and Addenda:** The RUWAP EIR and Addenda sections under Public Services and Recreation (Addendum No. 3 Section 3.13, p.22) identified a potentially significant due to limited access to public services and open space during construction; however this would be mitigated through implementation of mitigation. The RUWAP EIR and Addenda identified a less than significant impact from construction and operational landfill capacity and public services during operation.

#### **Proposed Project Modifications**

The proposed project modifications would not change the physical location or increase the size of the PWM/GWR or RUWAP structures or facilities. In addition, the proposed project modifications would eliminate redundant product water conveyance facilities. As a result, the proposed project modifications would not increase the demands for public services, utilities, or recreational facilities. Although the proposed project modifications would modify the capacity of the AWT Facility, the increase in capacity would not result in an increased demand for public services, utilities, or recreational facilities. The modifications to the proposed project would not result in new or substantially more severe environmental effects beyond those previously identified in the existing environmental documentation. The PWM/GWR EIR identified a potentially significant impact due to construction solid waste policies and regulations; however, the proposed modifications would not increase the amount of solid waste generated. Mitigation previously approved for the PWM/GWR Project would continue to apply, and the project modifications would not change the PWM/GWR's conclusion that the mitigation measure would reduce potential impacts to less than significant. No additional mitigation measures would be warranted.

#### Conclusion

In conclusion, the modifications to the project would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect. The findings of the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

#### **Traffic and Transportation**

#### Summary of Impacts in Previous Documents

**PWM/GWR EIR:** The PWM/GWR EIR (Vol 1 p. 4.17-1 – 4.17-52) found the PWM/GWR Project would not have a substantial adverse impact related to: 1) conflicts with Congestions Management Programs; 2) air traffic patterns; 3) increased hazards due to design; and, 4) conflicts with adopted policies regarding transit, bicycle, or pedestrian facilities. The PWM/GWR EIR found the PWM/GWR Project would have less than significant impacts due to construction and operational traffic. The PWM/GWR EIR found the PWM/GWR Project would have a potentially significant impact due to construction traffic delays, safety, and access limitations. The PWM/GWR EIR found that these potentially significant impacts would be mitigated with implementation of Mitigation Measure TR-2: Traffic Control and Safety Assurance Plan (see **Appendix B, Table 1** for mitigation measure). Furthermore, the PWM/GWR EIR found that the PWM/GWR Project would have a potentially significant impact due to construction related road deterioration. The PWM/GWR EIR found that these potentially significant impacts would be mitigated

with implementation of Mitigation Measure TR-3: Roadway Rehabilitation Program (see **Appendix B, Table 1** for mitigation measure). Also, the PWM/GWR EIR found the PWM/GWR Project would have a potentially significant impact due to construction parking interference, but that these potentially significant impacts would be mitigated with implementation of Mitigation Measure TR-4: Construction Parking Requirements (Applies to Product Water Conveyance pipelines (see **Appendix B, Table 1** for mitigation measure). The PWM/GWR EIR found the PWM/GWR Project would have less than significant cumulative operational impacts related to traffic and transportation or traffic and transportation impacts from cumulative development.

**RUWAP EIR and Addenda:** The RUWAP EIR and Addenda addressed Traffic and Circulation including cumulative conditions (Addendum No. 3 Section 3.14, p.22). Addendum No. 3 reduced the area of disturbance/area of potential effect for the RUWAP and found that traffic and circulation impacts that were previously identified as significant would be reduced in severity by the proposed changes. As reported in the RUWAP EIR and specifically in Addendum No. 3, adverse impacts resulting from construction of the shared Product Water Conveyance Pipeline as well as in conjunction with cumulative construction projects such as the PWM/GWR Project, would be reduced to less than significant levels through the implementation of mitigation measures identified in the RUWAP EIR, refer to **Appendix B, Table 2**.

#### **Proposed Project Modifications**

The proposed project modifications would not change the physical location or increase the size of the PWM/GWR or RUWAP structures or facilities. In addition, the proposed project modifications would eliminate redundant project water conveyance facilities. As a result, the proposed modifications would lessen the extent of construction-related traffic impacts, including construction-related traffic delays, safety, and access limitations. Moreover, the proposed modifications would not increase the extent of potential traffic-related effects associated with roadway deterioration and parking related impacts during construction. Temporary construction-related impacts would remain unchanged and the project modifications would not change the PWM/GWR EIR's conclusion that approved mitigation would ensure that impacts are minimized. The proposed project modifications would not result in any additional environmental effects beyond those previously identified in the existing environmental documentation summarized above. As a result, the proposed modifications would not result in any new or substantially more severe effects relating to construction traffic. No additional mitigation would be necessary.

The proposed modifications described above would not result in an increase in operational traffic. The proposed project modifications reduce product water conveyance facilities and would not result in any increase in the number of employees required to construct or operate the AWT Expanded Capacity Facility. The PWM/GWR EIR determined that project operation would result in a minor amount of additional operational traffic, which would be negligible in comparison to the existing traffic at the site and would not result in a noticeable change in traffic operations. The proposed modifications would not result in any additional operational traffic impacts beyond those previously identified in existing environmental documentation. Additionally, the project modifications would not change the PWM/GWR EIR's conclusion that all project traffic impacts would be mitigated to less than significant with implementation of required mitigation measures.

#### Conclusion

In conclusion, the modifications to the project would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect. The findings of

the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

#### Water Supply and Wastewater Systems

#### Summary of Impacts in Previous Documents

**PWM/GWR EIR:** The PWM/GWR EIR (Vol 1 p. 4.18-1 – 4.18-46) found the PWM/GWR Project would not have a substantial adverse impact related to construction of new water or wastewater treatment facilities or the expansion of existing facilities. The PWM/GWR EIR found the PWM/GWR Project would have less than significant impacts due construction an operational impacts on water supplies or entitlements, and construction and operational impacts on wastewater treatment capacity. The PWM/GWR EIR found the PWM/GWR Project would have less than significant cumulative impacts related to water supply, wastewater treatment capacity, or ocean outfall capacity(see Table 4.18-6 Summary of Impacts - Water Supply and Wastewater Systems, p. 4.18-28 of the PWM/GWR EIR).

**RUWAP EIR/Addenda:** Utilities and Service Systems are addressed in the RUWAP EIR and Addenda (Addenda No. 3 Section 3.15, p.23). The RUWAP EIR and Addenda found that the RUWAP would have less than significant impacts on wastewater and water quality. Refer to the Combined RUWAP and PWM/GWR EIR MMRP in **Appendix B, Table 2**.

#### **Proposed Project Modifications**

The proposed project modifications would not change the physical location or increase the size of the PWM/GWR or RUWAP structures or facilities. The AWT Expanded Capacity Facility would provide up to 600 AFY of purified recycled water to urban users in the MCWD service area and former Fort Ord. The proposed project modifications would eliminate redundant product water conveyance facilities based upon shared use of distribution and storage facilities between MRWPCA and MCWD. **Appendix F** presents an updated table and memorandum regarding **600 AFY RUWAP Recycled Water Urban Irrigation Use and Implications for CSIP Yields**. The analysis illustrates the availability of water to MCWD and to CSIP under various scenarios. The proposed project modifications would not result in any new or substantially more severe environmental effects beyond those previously disclosed in existing environmental documentation.

#### Conclusion

In conclusion, the modifications to the project would not: 1) result in any new significant environmental effects; or 2) substantially increase the severity of a previously identified significant effect. The findings of the existing environmental documentation would remain unchanged and no new or substantially revised mitigation measures are warranted.

#### **Cumulative Impacts**

#### Summary of Impacts in Previous Documents

The PWM/GWR EIR concluded that the impacts of the approved project, when combined with the impacts of past, present, and reasonably foreseeable projects, would result in the conclusions reached on **Table S-2 Determination of Significance and Discussion of Contribution of the Proposed Project to Cumulative Impacts (if applicable)**. The following discussion of significant cumulative impacts which can be mitigated to a less than significant level was presented in the PWM/GWR EIR:

**PWM/GWR EIR, Cumulative Air Quality:** The PWM/GWR EIR found that the PWM/GWR Project would contribute to the significant cumulative effect of regional emissions of PM<sub>10</sub>; however, with implementation of Mitigation Measure AQ-1 (Construction Fugitive Dust Control Plan), this cumulative impact would be reduced to less than significant. With mitigation, the PWM/GWR Project would therefore not make a considerable contribution to this significant cumulative impact.

**PWM/GWR EIR, Cumulative Marine Water Quality & Biological Resources:** The PWM/GWR EIR found that the PWM/GWR Project would potentially make a considerable contribution to significant cumulative impacts to marine water quality and marine biological resources due to the potential exceedance of the California Ocean Plan water quality objectives for several constituents if, in the future, the proposed CalAm desalination plant is constructed and placed into operation<sup>22</sup>.

However, with implementation of Mitigation Measure HS-C/MR-C (Implement Measures to Avoid Exceedances over Water Quality Objectives at the Edge of the Zone of Initial Dilution), the impact would be reduced to less than significant and the PWM/GWR Project would not make a considerable contribution to a significant cumulative impact.

RUWAP EIR and Addenda: Cumulative Impacts for the RUWAP EIR and Addenda were addressed in Addendum No. 3 (Section 3.17, p.23). Per the 2004 RUWAP EIR, potential significant cumulative impacts due to a recycled water system were limited to the areas of air quality, biological resources, and traffic<sup>23</sup>. See the discussions in Section 5.3 of the RUWAP EIR and Addenda under air quality, biological resources, and traffic. Addendum No. 3 notes that there were significant changes to the status and timeframes of the construction projects since the RUWAP EIR was certified (October 2004), the RUWAP EIR and Addenda found that the RUWAP would contribute to the significant cumulative effect of air quality, biological resources and traffic, however, with implementation of mitigation as described in Appendix B, Table 2. These cumulative impacts would be reduced to less than significant<sup>24</sup> with mitigation, therefore the RUWAP and particularly the shared Product Water Conveyance Pipeline as described in RUWAP Addendum No. 3 would not make a considerable contribution to these significant cumulative impacts.

<sup>&</sup>lt;sup>22</sup> For a list of Ocean Plan constituents and predicated concentrations, see PWM/GWR EIR Table 4.11-20 and Table 4.11-21; see also Appendices addressing Ocean Plan Compliance in the 2015 PWM/GWR EIR and Appendices C, D and E addressing Ocean Plan Compliance Assessment for the Pure Water Monterey Groundwater Replenishment Project and the Monterey Peninsula Water Supply Project (MPWSP) in this Addendum.

<sup>&</sup>lt;sup>23</sup> The Draft Environmental Impact (EIR) report was distributed to the public and circulated through the SCH for review from June 22, 2004 through August 5, 2004. The District held a public meeting certified the EIR, adopted an MMRP, and approved the Project on May 25, 2005. Addendum No. 3 to the EIR was distributed to the public and circulated through the SCH for review from March 23, 2016 through April 6, 2016. The District held a public meeting and adopted Addendum No. 3 to the EIR on April 18, 2016. Addenda No. 1 and 2 were not circulated through the State Clearinghouse. The District approved a Statement of Overriding Considerations on May 25, 2005 which stated that the Project will result in significant unavoidable adverse impacts on the environment related to aesthetics, air quality, biological resources, cultural resources, geotechnical and geologic hazards, hazards and hazardous materials, noise, public services recreation, and traffic and circulation. The District approved Addendum No. 1 to the EIR and adopted a second Mitigation Monitoring and Reporting Program on November 17, 2006 which reduced the significant unavoidable impacts covered in the Statement of Overriding Considerations to less than significant.

<sup>&</sup>lt;sup>24</sup> In accordance with the February 14, 2007 MMRP, the District shall implement the following Mitigation Measures: 4.3-R1 and 4.3-R2 for air quality, 4.6-RA and 4.6-RB for cultural resources, 4.4-R1 through 4.4-R19, and 4.4-R22 for biological resources, and 4.7-R1 through 4.7-R3 for geotechnical and geologic hazards.

#### **Proposed Project Modifications**

The proposed modifications would not alter the underlying impact conclusions or growth assumptions of the PWM/GWR EIR **Table S-2 Determination of Significance and Discussion of Contribution of the Proposed Project to Cumulative Impacts (if applicable)**. Therefore, there would be no change in the cumulative or growth inducing effects of the project.

Cumulative Air Quality: The modifications to the project would not increase the project's contribution to the significant cumulative effect of regional emissions of PM<sub>10</sub>. Therefore, the project modifications would not change the PWM/GWR's conclusion that, with implementation of Mitigation Measure AQ-1 (Construction Fugitive Dust Control Plan), this cumulative impact would be reduced to less than significant. As noted previously above, the proposed modifications considered in this Addendum would reduce redundant product water conveyance facilities, which would eliminate potential adverse temporary construction-related air quality effects associated with the construction of those facilities.

Cumulative Hydrology: Marine Water Quality: Operation of the AWT Expanded Capacity Facility would increase the amount of reverse osmosis concentrate discharged by the PWM/GWR project. However, with implementation of Mitigation Measure HS-C, Mitigation Measure HS-C (Implement Measures to Avoid Exceedances over Water Quality Objectives at the Edge of the Zone of Initial Dilution), the cumulative impact to marine water quality would be reduced to less than significant. See also discussion below.

Cumulative Marine Resources: Marine Quality: Operation of the AWT Expanded Capacity Facility would increase the amount of reverse osmosis concentrate discharged by the PWM/GWR project. The PWM/GWR EIR found that the project would potentially make a considerable contribution to significant cumulative impacts to marine water quality due to the potential exceedance of the California Ocean Plan water quality objectives for several constituents if, in the future, the proposed MPWSP desalination plant is constructed and placed into operation. Updated analysis prepared by Trussell Technologies is provided in Appendix E: Trussell Tech September 2017 Revised Ocean Plan Compliance Assessment for MPWSP and Project Variant, and includes modeling by Dr. Phillip Roberts presented in Appendix A of Appendix E. Mitigation Measure HR-C/MR-C (Implement Measures to Avoid Exceedances over Water Quality Objectives at the Edge of the Zone of Initial Dilution) requires that prior to operation of the MPWSP desalination plant, and before MRWPCA will accept the desalination brine discharge at its outfall, the discharger(s) will be required to test the MPWSP source water in accordance with protocols approved by the RWQCB. If the water quality assessment indicates that the water at the edge of the ZID will exceed the Ocean Plan water quality objectives, specific design features and/or operational measures would be required to be employed, individually or in combination, to reduce the concentration of constituents to below the Ocean Plan water quality objectives at the edge of the ZID. Further, MPWSP operational discharges would be subject to the permit requirements prescribed by the RWQCB as part of the NPDES permit amendment process for the desalination plant. Such requirements would be designed to ensure that operation of the MPWSP Desalination Plant would not violate waste discharge requirements defined in the amended NPDES permit, which incorporate the Ocean Plan objectives (personal communication Justin Taplin, Principal and Senior Environmental Scientist, Sutro Science LLC). With implementation of Mitigation Measure HS-C/MR-C, the cumulative impact to marine water quality would be reduced to less than significant.

None of the significance conclusions or findings in the Final PWM/GWR EIR would be altered, no new significant impact would occur, and none of the previously identified significant impacts would be substantially worsened.

**Table S-2 Summary of Cumulative Impacts and Mitigation Measures** 

#	Topical Section/ Impact Issue	Cumulative	Determination of Significance and Discussion of Contribution of the Proposed Project to Cumulative Impacts (if applicable)	Mitigation Measures						
4.2	Aesthetics		LS: There would be no significant cumulative construction or operational aesthetic impacts.							
4.3	Air Quality and Greenhouse	d Greenhouse gas emissions and the related global climate change impacts.								
	Gas	Overall Greenhouse Gas Emissions	LS: The Proposed Project would not make a considerable contribution to significant cumulative impacts of greenhouse gas emissions and the related global climate change impacts							
		Air Quality: Overall PM10	<b>LSM:</b> The Proposed Project would potentially make a considerable contribution to significant cumulative of regional emissions of PM <sub>10</sub> ; however, with implementation of Mitigation Measure AQ-1, the impact would be reduced to less than significant and the proposed Project would not make a considerable contribution to a significant cumulative impact.	AQ-1 (see Table S-1)						
4.4	Biological Resour	ces: Fisheries	LS: There would be no significant construction or operational cumulative impacts to biological resources: fisheries.							
4.5	Biological Resour	ces: Terrestrial	LS: The Proposed Project would not make a considerable contribution to significant cumulative impacts to biological resources: terrestrial.							
4.6	Cultural and Pale Resources	eontological	LS: There would be no significant construction or operational cumulative impacts to cultural and paleontological resources.							
4.7	4.7 Energy and Energy Mineral Resources Minerals		LS: The Proposed Project would not make a cumulatively considerable contribution to a significant cumulative energy impact.							
			LS: There would be no significant construction or operational cumulative impacts to mineral resources.							
4.8	4.8 Geology, Soils, and Seismicity		LS: There would be no significant construction or operational cumulative geology, seismicity or soils impacts.							
4.9	Hazards and Hazardous Materials		LS: There would be no significant construction or operational cumulative impacts related to hazards or hazardous materials.							
4.10	Hydrology/Water Quality: Groundwater		LS: The Proposed Project would not contribute to significant cumulative impacts to groundwater levels, recharge, storage or quality in the Salinas Valley Groundwater Basin. There would be no significant construction or operational impact to groundwater levels, recharge or storage in the Seaside Groundwater Basin. The Proposed Project would not make a considerable contribution to cumulative impacts to groundwater quality in the Seaside Basin.							
4.11	Hydrology/Water Quality: Surface Water	r Inland Surface Waters	LS: There would be no significant construction or operational cumulative impacts to hydrology and water quality of inland surface waters.							
	Marine Surface Waters		<b>LSM:</b> The Proposed Project would potentially make a considerable contribution to significant cumulative impacts to marine water quality due to the potential exceedance of the California Ocean Plan water quality objectives for several constituents; however, with implementation of Mitigation Measure HS-C, the impact would be reduced to less than significant and the proposed Project would not make a considerable contribution to a significant cumulative impact.	HS-C						
4.12	Land Use, Agricu Resources	ılture, and Forest	LS: There would be no significant construction or operational cumulative land use impacts, and the Proposed Project would not make a considerable contribution to significant cumulative impacts related to conversion of agricultural lands within unincorporated Monterey County.							
4.13	Marine Biologica	l Resources	<b>LSM:</b> The Proposed Project would potentially result in a considerable contribution to significant cumulative impacts on marine biological resources due to the potential exceedance of the Ocean Plan water quality objectives for several constituents; however, with implementation of Mitigation Measure MR-C, the impact would be reduced to less than significant and the Proposed Project would not make a considerable contribution to a significant cumulative impact.	MR-C (Implement HS-C)						

**Table S-2 Summary of Cumulative Impacts and Mitigation Measures** 

#	Topical Section/ C Impact Issue	umulative	Determination of Significance and Discussion of Contribution of the Proposed Project to Cumulative Impacts (if applicable)	Mitigation Measures					
4.14	Noise and Vibratio	n	LS: There would be no significant construction or operational cumulative noise and vibration impacts.						
4.15	Population and Ho	using	LS: The Proposed Project would not make a considerable contribution to significant cumulative impacts related to population and housing.						
4.16	Public Services, Red Utilities	creation, and	LS: The Proposed Project would not contribute to cumulative impacts related to schools, parks, and recreational facilities.  The Proposed Project would not make a considerable contribution to significant cumulative impacts to other public services and utilities (fire and police protection, solid waste).						
4.17	4.17 Traffic and Transportation		LS: There would be no significant cumulative construction-related traffic and transportation impacts. The Proposed Project would not make a considerable contribution to significant cumulative traffic and transportation impacts due to cumulative development.						
4.18	Water Supply Water and Wastewater Supply		LS: The Proposed Project would not make a considerable contribution to significant cumulative impacts to water supply.						
	Systems Wastewater LS: There would be no significant cumulative impacts on wastewater treatment capacity or ocean outfall disposal capacity								
Source: 1	Source: PWM/GWR EIR, Table S-2								

#### VIII. REPORT PREPARATION AND REFERENCES

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- Trussell Technologies, 2015c. Amendment to Ocean Plan Compliance Assessment for the Monterey Peninsula Water Supply Project and Project Variant, April 17, 2015. [PWM GWR EIR Appendix U-2]
- Trussell Technologies, September 2017 Ocean Plan Compliance Assessment for the Pure Water Monterey Groundwater Replenishment Project
- Trussell Technologies, September 2017 Draft Communication: Comparison of Dilution Results
- Trussell Technologies, September 2017 Revised Ocean Plan Compliance Assessment for Monterey Peninsula Water Supply Project and Project Variant

#### IX. EIR Preparers, Agencies And Persons Consulted

#### **Agencies**

Lead Agency: Monterey Regional Water Pollution Control Agency (Monterey One Water)

- Robert Holden, P.E., Principal Engineer/Project Manager
- Alison Imamura, Associate Engineer

Partner Agency: Monterey Peninsula Water Management District

#### Marina Coast Water District

Mike Wegley, District Engineer

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- Ashley Quackenbush, Assistant Planner
- Shaelyn Hession, Assistant Scientist
- Mary Echevarria, Contracts and Operations Manager
- Robyn Simpson, Administrative Professional

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• Denise H. Conners, Associate

#### Trussell Technologies

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- John Kenny, P.E., Senior Engineer
- Brie Webber, P.E., Associate Engineer

#### Perkins Coie

- Barbara J. Schussman, Managing Partner
- Laura Zagar, Partner

#### **Additional Technical Consultants**

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- Jonathon P. Marshall, P.E., LEED AP, Senior Engineer
- Anne Prudhel, P.E., Infrastructure Project Manager, Associate Vice President

#### E2 Consulting Engineers

- Loren Weinberger, P.E., Engineer
- Vinod Badani, Vice President

#### Illingworth & Rodkin

• James Reyff, Air Quality/GHG

#### Kennedy/Jenks Consultants

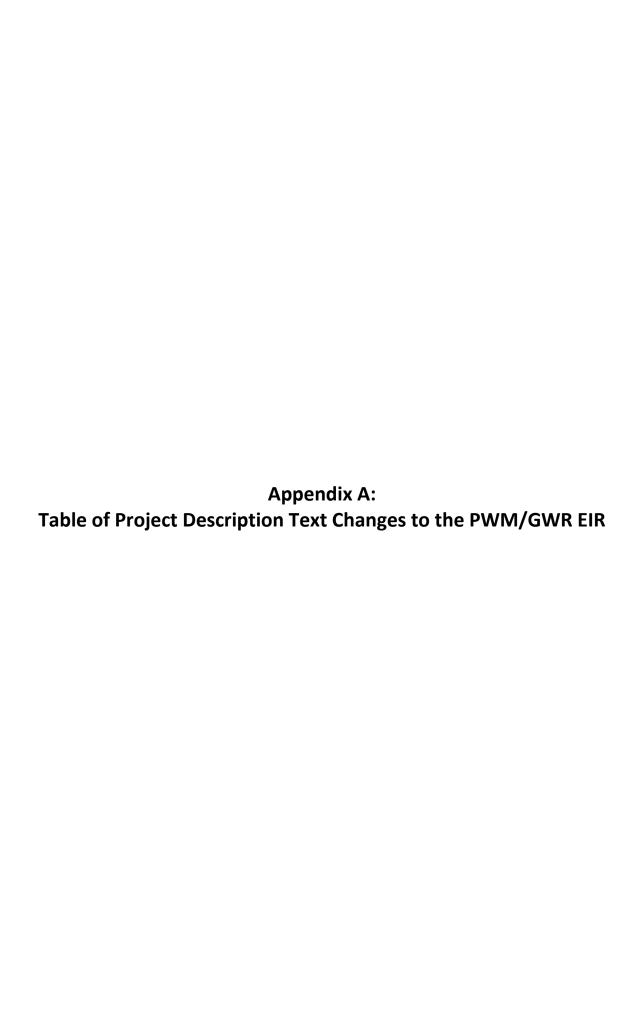
• Todd Reynolds, P.E., Vice President, Water Practice Leader

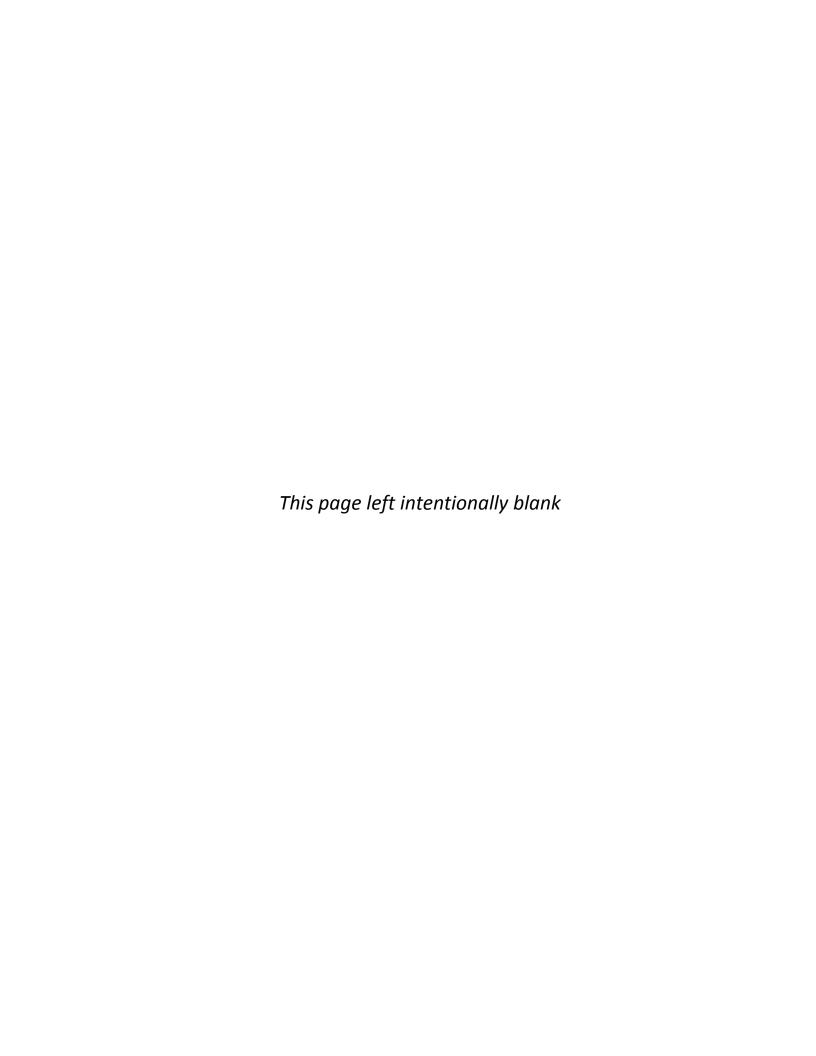
#### Schaaf & Wheeler, Consulting Civil Engineers

• Andrew Sterbenz, P.E. Project Engineer

In addition to the above, the following were consulted in the preparation of this Addendum:

- Taplin, Justin, Senior Environmental Scientist, Sutro Science LLC
- Zigas, Eric, Project Managers for the MPWSP EIS/EIR, Environmental Science Associates, Inc.
- Roberts, Philip J. W., Consulting Engineer Ph.D., P.E.
- Applied Marine Sciences, Inc.





	-	ed Capacity Advanced Water Treatment Facility Project Modifications adum to the PWM/GWR EIR Changes from 2015 PWM/GWR Final EIR
Page	PWM/GWR EIR Section	Changes from 2015 PWM/GWR Final EIR shown in Strike-out and Underline
2-3	Project Description 2.1.1.2 GWR Facilities (Footnote 3) Project	Amend to add Footnote:  1 Throughout the EIR and this Addendum, the term Advanced Water Treatment (or AWT) Facility is used for consistency. During design and bidding of this project component, the name of the same facility is also referred to as the Advanced Water Purification (or AWP) Facility. The two terms are interchangeable.  Amend Section 2.1.1.2 GWR Facilities by adding the following paragraph to the end of the
2-4	Description 2.1.1.2 GWR Facilities	section:  MRWPCA is now proposing to increase the GWR project AWT Facility maximum capacity  (product water flowrate) from 4 million gallons per day (mgd) to 5 mgd to provide up to 600  AFY of purified recycled water for urban irrigation for the Marina Coast Water District  customers.
2-6	Project Description Section 2.3 Project Background	Amend Section 2.3 Project Background, as follows:  This section provides information on the impetus for the Proposed Project, including a description of the agencies that have primary responsibility for its development and implementation (MRWPCA and Water Management District), an overview of the Seaside Groundwater Basin, an overview of the water resources of the Salinas Valley, a discussion of the relationship of the GWR Features to the proposed CalAm desalination plant, and a discussion of the relationship of the Crop Irrigation component to the Salinas Valley Reclamation Plant and CSIP. In addition, this section provides information on the Marina Coast Water District aspects of the GWR Project.
2-15	Project Description 2.3.3.3 Marina Coast Water District	Amend the third paragraph of Section 2.3.3.3 Marina Coast Water District, as follows:  Water demands on the former Fort Ord are projected to increase with development envisioned in the Fort Ord Base Reuse Plan. To address the need for additional water supply, Marina Coast Water District is developing the Regional Urban Water Augmentation Project (RUWAP). The RUWAP would provide an additional 2,400 AFY of potable and/or recycled water. Marina Coast Water District certified the EIR for the RUWAP in 2005, and approved addenda to the EIR in 2007 and 2008 to address changes to the proposed pipeline alignment, construction assumptions, and water quantities. The trunk main of the RUWAP system is coincident with the Proposed Project's RUWAP Pipeline alignment option. The RUWAP recycled water distribution system has been designed and partially constructed, but is not yet in operation. Addendum No. 3 to this EIR addresses the proposed shared use of MCWD's RUWAP pipeline and providing 600 AFY of irrigation water to the MCWD customers.
2-32	Project Description Section 2.6.1 Proposed Project Facilities Overview	<ul> <li>Amend the bulleted list in Section 2.6.1 Proposed Project Facilities Overview, as follows:         <ul> <li>Source water diversion and storage – facilities to enable diversion of new source waters to the existing municipal wastewater collection system and conveyance of those waters as municipal wastewater to the Regional Treatment Plant to increase availability of wastewater for recycling. Modifications would also be made to the existing Salinas Industrial Wastewater Treatment Facility to allow the use of the existing treatment ponds for storage of excess winter source water flows and later delivery to the Regional Treatment Plant for recycling.</li> <li>Treatment facilities at Regional Treatment Plant – use of existing primary and secondary treatment facilities at the Regional Treatment Plant, as well as new pretreatment, advanced water treatment (AWT), product water stabilization, product water pump station, and concentrate disposal facilities, and modifications to the Salinas Valley Reclamation tertiary treatment plant.</li> </ul> </li> </ul>

- Product water conveyance new pipelines, booster pump station, appurtenant facilities along one of two optional pipeline alignments to move the product water from the Regional Treatment Plant to the Seaside Groundwater Basin injection well facilities. In its October 8, 2015 resolution (Resolution 2015-24) approving the PWM/GWR project, MRWPCA selected the RUWAP Alignment for the Project Water Conveyance. The project water conveyance facilities are now proposed to be shared facilities built by Marina Coast Water District. Specifically, MCWD and MRWPCA are working towards an agreement to share conveyance facilities for purified recycled water for urban irrigation and for groundwater replenishment. The booster pump station previously included in the PWM/GWR project will not be necessary.
- Injection well facilities new deep and vadose zone wells to inject Proposed Project product water into the Seaside Groundwater Basin, along with associated back-flush facilities, pipelines, electricity/ power distribution facilities, and electrical/motor control buildings.
- Distribution of groundwater from Seaside Groundwater Basin new CalAm distribution system improvements needed to convey extracted groundwater and deliver it to CalAm customers. These same CalAm distribution improvements also would be needed if CalAm were to implement the Monterey Peninsula Water Supply Project, which is undergoing separate CEQA review.

# 2-33 Project to 2- Description 35 Section 2.6.2 Proposed Project

Overview

#### Amend Section 2.6.2 Proposed Project Overview as follows:

The Proposed Project would operate with annual and seasonal variations based on the amount of available runoff, the water year type, the varying irrigation demand for recycled water, and the amount of water stored in the Seaside Groundwater Basin as a drought reserve each year.

The primary project objective is to replenish the Seaside Groundwater Basin to produce high quality water to replace CalAm water supply as required by State Orders. The ability of the project to meet the primary project objective of providing CalAm extractions of 3,500 AFY would not depend on water year type (wet, normal, or dry).

The Proposed Project would also increase the amount of recycled water available for crop irrigation within the existing CSIP service area by approximately 4,500 to 4,750 AFY during normal and wet years, and by up to 5,900 AFY during drought conditions. For MRWPCA to secure the necessary rights and agreements to use the source waters needed for the Proposed Project, preliminary negotiations with stakeholders indicate that MRWPCA also would need to increase the amount of recycled water provided to the CSIP area. This amount is within the total permitted capacity of the Salinas Valley Reclamation Plant of 29.6 mgd. Irrigation demands vary seasonally, peaking in the spring and summer months, and also by water year type, increasing in dry and hotter years. Irrigation demand can also change in response to changes in cropping patterns and irrigation practices. The Salinas Valley Reclamation Plant produces tertiary-treated, disinfected water supply (recycled water) from treated municipal wastewater for the CSIP. Peak irrigation demands in the CSIP system exceed the amount of available treated municipal wastewater, so additional water is supplied from the Salinas River and the Salinas Groundwater Basin. The Proposed Project would increase the availability of recycled water during the peak demand periods by providing new sources of water supply to the Salinas Valley Reclamation Plant. The Project also would increase the availability of recycled water for crop irrigation during low demand periods by modifying the Salinas Valley Reclamation Plant to allow production and delivery at lower daily rates, thus further reducing pumping from supplementary groundwater wells.

In addition, to better accommodate variable annual crop irrigation demands for recycled

water, an additional 200 AFY would be produced and injected into the Seaside Groundwater Basin during most years to develop a drought reserve of up to 1,000 acre-feet of stored water. This would allow MRWPCA to reduce deliveries of product water to the Seaside Groundwater Basin during drought years, while still enabling CalAm to pump 3,500 AFY from the Seaside Groundwater Basin by using the reserved water. By reducing deliveries of product water to the Seaside Groundwater Basin during drought years, MRWPCA would be able to increase deliveries of recycled water to growers by a commensurate amount.

Finally, to provide irrigation water to MCWD's customers, an additional 600 afy would be produced and delivered to MCWD. In order to satisfy variations in the MCWD irrigation demand, the AWT Facility may operate in the range of production in order to meet irrigation demands; the variability is needed in order meet MCWD demand from urban irrigation customers. The Proposed Project's AWT Facility would be designed and constructed to allow production rates from 1.32 mgd (900830 gpm) to 45.0 mgd (2,7003,500 gpm). During a wet or normal year, the AWT Facility would operate at an average rate of 3.54.0 mgd during the summer months (April to September). If the drought reserve is full (1,000 acre-feet additional have been "deposited" in the Seaside Groundwater Basin), the winter production rate would remain 3.54.0 mgd. If the drought reserve is not full, the winter production rate would be increased to 4.0 4.2 mgd to allow the production of an additional 200 AFY. During certain dry years, the AWT Facility production rate would be decreased in the summer months, to rates as low as 1.38 mgd, depending upon the amount of water "deposited" in the drought reserve and the demands of the CSIP irrigators. The average monthly flows have been provided. Instantaneous production and flow could reach 5.0 mgd at any time during any month within those stated range of flow rates. The Proposed Project would produce enough advanced treated water in each year so that the amount of injected water plus the amount of "withdrawn" drought reserve or operational reserve equals the 3,500 AFY extracted by CalAm. Water supplies not used for the AWT Facility would be used by the Salinas Valley Reclamation Plant to produce additional recycled water for the CSIP.

**Table 2-9, Proposed Project Monthly Flows for Various Flow Scenarios** summarizes typical flow operations for the AWT Facility based on seasonal flow and demand conditions. Although presented as fixed water year types, actual system operation would require daily or weekly management of the production rates to address the variability in irrigation demands and supply availability. Source water diversions would be similarly managed to maximize water availability during the peak irrigation season, as discussed in **Section 2.7.1**.

Table 2-9, Proposed Project Monthly Flows for Various Flow Scenarios

### AWT Facility Influent/Feed Purified Recycled Water Delivery

	Product Water Delivery Schedules for			Acre-Feet per Month (AF/month)											Total	Add to	Reserve as
	Seaside Basin Injectio	n	Oct	Nov	Dec	Jan	Feb	Mar	Apr	May	June	July	Aug	Sep	AFY	Reserve	of April 1
1	Drought Reserve <1,000 AF (Oct)	Wet/Normal Year	331	321	331	331	299	331	288	297	288	297	297	288	3,700	200	-
2	Drought Reserve 1,000 AF (Oct)	Wet/Normal Year	297	288	297	297	268	297	288	297	288	297	297	288	3,500	-	-
3	Drought Reserve <1,000 AF (Oct)	Drought Year	331	321	331	331	299	331	255	263	255	263	263	255	3,500	200	200
4	Drought Reserve <1,000 AF (Oct)	Drought Year	331	321	331	331	299	331	222	229	222	229	229	222	3,300	200	400
5	Drought Reserve <1,000 AF (Oct)	Drought Year	331	321	331	331	299	331	189	196	189	196	196	189	3,100	200	600
6	Drought Reserve <1,000 AF (Oct)	Drought Year	331	321	331	331	299	331	156	162	156	162	162	156	2,900	200	800
7	Drought Reserve <1,000 AF (Oct)	Drought Year	331	321	331	331	299	331	124	128	124	128	128	124	2,700	200	1,000
8	Drought Reserve 1,000 AF (Oct)	Drought Year	297	288	297	297	268	297	124	128	124	128	128	124	2,500	-	1,000
	Administration Delication Control			Gallons per Minute (gpm) Maximum Injection Rate										tion Rate			
	Maximum Monthly Injection Rates		Oct	Nov	Dec	Jan	Feb	Mar	Apr	May	June	July	Aug	Sep		(gpm)	İ
	Santa Margarita Aquifer (90%)		2,175	2,179	2,175	2,175	2,175	2,175	1,955	1,951	1,955	1,951	1,951	1,955		2,179	
	Paso Robles Aquifer (10%)			242	242	242	242	242	217	217	217	217	217	217		242	
	Total		2,417	2,422	2,417	2,417	2,417	2,417	2,173	2,168	2,173	2,168	2,168	2,173		2,422	

Extra to build drough reserved to build drough	Jan	Feb	Mar	Apr	May	June	July	Aug	Sep	Oct	Nov	Dec	Total (AFY)
Advanced Water Treatment Facility Reverse O	smosis Fe	ed (acre-	feet) (Se	ee Note	1)	109	133	137	133	1.	7	37	133
After drought reserve complete	367	331	367	355	367	355	367	367	355	367	355	367	4,32
Extra to build drought reserve	42	38	42	-			1405.1	- Incol		42	41	42	247
2. Wet and Normal Years	409	369	409	355	367	355	367	367	355	409	396	409	4,568
3. Drought Years when Full Drought Reserve	409	369	409	133	137	133	137	137	133	409	396	409	3,21:

Note 1: These estimated flows exclude the membrane filtration backwash quantities that would be recirculated back to the Regional Treatment Plant headworks and thus would not be considered to be new flows.

Operation of the Proposed Project facilities would require some additional staff at the MRWPCA Regional Treatment Plant and administrative office. The AWT Facility would require up to five personnel to operate the facility 24-hours a day, 7-days a week. The Salinas Valley Reclamation Plant would operate with the same number of staff as currently assigned, but operations would extend into the wet season. The source water diversion and product water conveyance and injection facilities would not require on-site staff, but would require periodic site visits and maintenance activities. These are discussed in detail in the sections below regarding each component.

Table 2-11, Overview of Proposed Project Electricity Demand (all in megawatt-hours per year) summarizes the power demands of the Proposed Project. The energy the requirements of the Proposed Project under the 2015 PWM/GWR EIR would require an estimated 10,952 megawatt-hours per year (mW-hr/yr). Power use for the Crop Irrigation component would peak during drought years when additional recycled water is being produced. Electrical power at the existing MRWPCA facilities comes from solar panels and from generators running on a mix of methane (from the Regional Treatment Plant) and natural gas (from PG&E), with backup electrical service from PG&E. Additional power would be generated using increased methane from processing of new source water, and increased purchase of biogas from the adjacent landfill and natural gas from PG&E. Electrical power for the source water diversion facilities, product water booster pump station, and injection well facilities would be purchased from PG&E. Salinas Pump Station and future Salinas Ponds power would be from City of Salinas solar panels; refer to Updated Table 2-11 shown below.

Table 2-10, Overview of Typical Facility Operations – Proposed Project provides an overview of typical facility operations, truck trips and employees under the Proposed Project. Table 2-11, Overview of Proposed Project Electricity Demand (all in megawatt-hours per **year)** summarizes the power demands of the Proposed Project. The Project modifications under the Expanded AWT Facility Capacity would result in an incremental increase in energy (electricity) use associated with the AWT Facility due to the operation of the higher peak production capacity and pumping by the product water pump station at the AWT Facility to deliver purified recycled water to MCWD customers. The incremental increase in energy demand associated with Expanded AWT Capacity Facility operation would be accommodated through the purchase of energy produced by the MRWMD. As detailed in Updated Table 2.11, there is sufficient landfill-gas generated electricity available at the Monterey Regional Landfill to accommodate the incremental increase in electricity demand associated with the proposed Expanded AWT Capacity Facility.

2-37 Project Description 2.6.1 Proposed **Project Facilities** Overview

<u>Updated</u> Table 2-11 Overview of Proposed Project Electricity Demand (all in megawatt-hours per year) to update energy demand and identify new renewable energy sources. Refer to Updated Tables in this section.

	Table 2 11	
2.50	Table 2-11	Annual Castina 2 0 4 Occasions of Treatment Facilities at the Basic and Treatment Blank
2-59	Project	Amend Section 2.8.1 Overview of Treatment Facilities at the Regional Treatment Plant, as
	Description	follows:
	Section 2.8.1	Under the Proposed Project, a new AWT Facility would be constructed to receive Regional
	Overview of	Treatment Plant secondary effluent for advanced treatment and, ultimately, injection into the
	Treatment	Seaside Groundwater Basin and provision to MCWD for urban landscape irrigation through its
	Facilities at the	RUWAP. In addition, modifications to the existing Salinas Valley Reclamation Plant are
	Regional	proposed in order to enable increased use of tertiary treated wastewater for crop irrigation
	Treatment Plant	during winter months. The proposed new and modified treatment facilities at the Regional
		Treatment Plant, including the Advanced Water Treatment Facility (or AWT Facility) and the
		Salinas Valley Reclamation Plant Modifications, would be constructed on approximately 3.5
		acres of land within the MRWPCA Regional Treatment Plant (Regional Treatment Plant) site
		west of the existing treatment facilities (see Figure 2-10, Projected Regional Treatment Plant
		<b>Flows).</b> The following is a list of the proposed structures and facilities proposed to be
		constructed at the Regional Treatment Plant (see <b>Figure 2-27, Advanced Water Treatment</b>
		Facility Site Plan):
		inlet source water secondary effluent diversion structure, an influent  - inlet source water secondary effluent diversion structure, an influent  - inlet source water secondary effluent diversion structure, an influent
		approximately 60-foot long, 30-inch diameter pipeline to bring the secondary
		effluent to the source water pump station, the source water pump station, and an
		approximately 360 300-foot long, 24-inch diameter pipeline to bring secondary
		effluent to the <u>rest of the</u> AWT Facility;
		advanced treatment process facilities, including     a chloromination
		<ul><li>chloramination,</li><li>ozonation,</li></ul>
		6.1
		<ul> <li>booster pumping of the ozone effluent,</li> <li>biologically active filtration (if-approved for the Project, but may not be</li> </ul>
		required),
		o automatic straining,
		o membrane filtration treatment,
		<ul> <li>booster pumping of the membrane filtration filtrate,</li> </ul>
		o cartridge filtration,
		o <u>reverse osmosis pre-treatment</u> chemical addition,
		o booster pumping of the pre-treated reverse osmosis feed,
		o reverse osmosis membrane treatment,
		o advanced oxidation using ultraviolet light and hydrogen peroxide
		<del>(advanced oxidation)</del> ,
		o side stream decarbonation, and
		o product-water stabilization with calcium, alkalinity and pH adjustment
		liquid lime;
		<ul> <li>final product storage and distribution pumping water pump station;</li> </ul>
		<ul> <li>brine mixing facilities; and</li> </ul>
		<ul> <li>waste water equalization and pump station; and</li> </ul>
		o modifications to the Salinas Valley Reclamation Plant (see Section 2.8.2 for
		a detailed description this Proposed Project component).
		The proposed advanced treatment facilities would include several structures as tall as 31 34

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<sup>&</sup>lt;sup>1</sup> As described in previous sections, the Proposed Project proposes to divert additional water sources and convey those waters with municipal effluent to the Regional Treatment Plant, including urban and agricultural runoff, agricultural wash water flows, and excess/unused Regional Treatment Plant secondary-treated wastewater.

		feet and totaling approximately 6050,000 square feet. The proposed brine mixing facility would be up to 16 feet tall and totaling approximately 10,000 square feet. New pipes and pumps would be underground. Additional information on each component of the AWT Facility is presented in the following sections. Figure 2-28, Proposed Advanced Water Treatment Flow Diagram, provides a simplified AWT Facility process flow diagram illustrating the proposed treatment facilities.
2-60	Project	Amend Section 2.8.1.1 AWT Facility Design Flows and System Waste Streams, as follows:
to 2- 61	Description Section 2.8.1.1 AWT Facility Design Flows and System Waste Streams	The proposed new AWT Facility would have a design capacity of 4 5.0 mgd of product water. As described in <b>Section 2.7.1</b> , a range of monthly source water flows has been estimated, depending upon the seasonal availability of source waters. The facility would be operated to produce up to 3,700 AFY of purified recycled water for injection and 600 AFY of purified recycled water to MCWD for urban landscape irrigation, which equates to an annual production rate of 3.38 mgd. The 45.0 mgd facility size is required to allow for peak seasonal operation and system down time. Similarly, the system components must be sized to allow for losses during treatment such as backwashing and brine concentrate disposal. Additional information on the proposed AWT Facility component design is presented in <b>Tables 2-18 and 2-19</b> . See Updated Tables following this section.
		In producing highly purified water, the proposed new AWT Facility would also produce two three waste streams: biological filtration backwash (if included in the system), membrane filtration backwash, and reverse osmosis concentrate. The biological filtration backwash (if included) and-the membrane filtration backwash would be diverted back to the Regional Treatment Plant headworks. The, while the reverse osmosis concentrate would be piped to a proposed new brine and effluent receiving, mixing, and monitoring facility. The AWT discharged out through the existing ocean outfall. The AWT Facility is expected to be able to produce water at up to 90% of design capacity, on average, due to some anticipated down time for membrane "clean in place" practices and repairs. The down time is assumed to be evenly distributed each month, though planned events would be scheduled for times when the least source water is available. The AWT Facility would need to be large enough to produce the required product water during the operational times (90% of each month). The resulting flow quantities for the AWT Facility are shown in Table 2-19, Proposed Project AWT Facility Process Design Flow Assumptions below. See Updated Tables following this section  Based on these assumptions (including the 90% in-service, 81% reverse osmosis recovery, 90% microfiltration recovery), an AWT Facility design flow rate of 45.0 mgd would be required to provide up to 3,700 AFY of high quality water for groundwater injection and 600 AFY to MCWD for urban landscape irrigation.
2-62	Project	Amend Section 2.8.1.3 Raw Water Pretreatment, as follows:
	Description Section 2.8.1.3 Raw Water Pretreatment	Before membrane filtration, the secondary effluent would be pretreated using pre-screening and up to three separate subsystems:  • Chloramination  • Ozonation  • Biologically active filtration (if approved for the Project, but may not be required)  Chloramination. Chloramines would be used to reduce biofouling of the membrane systems.
		The chloramination system would include sodium hypochlorite storage, and chemical feed pumps, and an inline injection and mixing system. Sodium hypochlorite would be injected upstream of ozonation or upstream of membrane filtration. Sodium hypochlorite reacts with ammonia present in the source water to form chloramine, which is an effective biocide that reduces biological fouling on the membrane filtration and reverse osmosis process membranes.

Ozonation. Ozone treatment is proposed to provide a chemical/pathogen destruction barrier and reduce the membrane fouling. The ozone system would be comprised of several components: liquid oxygen storage and vaporizers or an onsite oxygen generator; a nitrogen boost system; an ozone generator and power supply unit; a cooling water system; a sidestream injection system; ozone contactor; and ozone destruct units. There are two potential approaches for supplying high High-purity oxygen for ozone generation: (1) will be produced via liquid oxygen delivered to onsite cryogenic storage tanks and evaporated through vaporizers, or (2) produce oxygen at the treatment facility using a pressure-swing adsorption oxygen generation system. The liquid oxygen system is included in the 10% design, but an onsite generation system would occupy approximately the same amount of space. Ozone generators would convert oxygen gas into a mixture of oxygen and ozone gas. The mixture of oxygen and ozone gas would be injected into a side stream of feed water flow that would then be recombined with the main supply line after ozone injection. The ozonated water would flow into one or more parallel contactors a pipeline contactor to provide contact time for disinfection/oxidation, ozone residual decay, and off-gassing. Off-gas would be treated through a catalytic-based ozone destruct system to prevent the release of ozone to the atmosphere. Once dissolved in the process water, ozone reacts with various contaminants in the water, resulting in several treatment benefits, including (1) reduction of organic compounds that cause membrane fouling, (2) reduction of many constituents of emerging concern (CECs),<sup>2</sup> and (3) inactivation of pathogenic microorganisms. A quenching system to eliminate any ozone residual that remains in the water is included at the end of this process step. Quenching would be performed through the addition of sodium bisulfite, hydrogen peroxide or calcium thiosulfate, which would be stored on-site.

Biologically Active Filtration (if approved for the Project, but it may not be required): This process may be would have been used downstream of ozone treatment to reduce the concentration of ammonia and residual organic matter present in the ozone effluent and to reduce the solids loading on the membrane filtration process. The biologically active filtration system would consist have consisted of gravity-feed filter basins with approximately 12 feet of granular media, and an underdrain/media support system. Ancillary systems would include have included an alkalinity addition system for pH control, backwash water basin (also used for membrane filtration backwash), backwash pumps, an air compressor and supply system for an air scour system, an air compressor and supply system for process air, and a wash water basin to facilitate filter backwashing. Depending upon the discharge permitting conditions, this This process step may was approved for the Project, but was determined to not be required to meet regulatory requirements for the PWM/GWR Project; therefore, it may would not be constructed. 13

#### Added footnote 13:

<sup>13</sup> Although this treatment process is not needed for the PWM/GWR Project, it could be constructed to mitigate Ocean Plan compliance impacts of future MPWSP desalination project, if that project is approved by the CPUC,.

# 2-63 Project Description Section 2.8.1.4 Microfiltration/U Itrafiltration Membrane Treatment

## Amend Section 2.8.1.4 Microfiltration/Ultrafiltration Membrane Treatment System, as follows:

The membrane filtration system would remove suspended and colloidal solids, including bacteria and protozoa through hollow fiber membrane modules. Additional components of the membrane filtration system include valve manifolds to direct the flow of feed, filtrate, cleaning system, backwash supply, backwash waste, and compressed air to the corresponding module piping. Feed pumps would draw water from the feed clearwell tank and supply a pressurized

<sup>&</sup>lt;sup>2</sup> See **Chapter 3. Water Quality Permitting and Regulatory Overview** for more information about the current understanding and regulation of these substances.

acid, caustic, and sodium hypochlorite, which would be stored on-site. Backwash residuals would be adjusted to a neutral pH in the waste water equalization basin to the Regional Treatment Plant headworks, along with residuals associated with system. The projected recovery of treated water from the membrane filter system 90%; this recovery accounts for waster residuals associated with backwashing, clea pretreatment straining.  2-63  Project  Description Section 2.8.1.5  Reverse Osmosis Membrane Treatment System  Anend Section 2.8.1.5 Reverse Osmosis Membrane Treatment System, as follows: Membrane Treatment System  System  System  System would include a second stage to increase the product water recovery. The proposed reverse osmosis system would include individual process trains, hour process membranes in pressure vessels along with connecting piping and valve me feed, permeate, concentrate, cleaning and flush supplies. The ancillary equipment overall reverse osmosis system would include a membrane cleaning system and prilush system. Reverse osmosis system would be delivered from the upstream memb filtration system through an intermediate equalization tank. Low pressure boosted tank. Transfer pumps would move the water into the pretreatment system. Pertreament was the water from pretreatment into the reverse osmosis treatment trains. Concentre ververse osmosis system would be delivered from the upstream memb fil			
Description Section 2.8.1.5 Reverse Osmosis Membrane Treatment System Treatment System System System Treatment System System Treatment System		System	feed to pretreatment strainers and the membrane units. Cleaning chemicals would include acid, caustic, and sodium hypochlorite, which would be stored on-site. Backwash and screening residuals would be adjusted to a neutral pH in the waste water equalization basin and returned to the Regional Treatment Plant headworks, along with residuals associated with the cleaning system. The projected recovery of treated water from the membrane filter system is roughly 90%; this recovery accounts for waste residuals associated with backwashing, cleaning, and pretreatment straining.
Description Section 2.8.1.5 Reverse Osmosis Membrane Treatment System Treatment System System System Treatment System System Treatment System	2-63 F	Project	Amend Section 2.8.1.5 Reverse Osmosis Membrane Treatment System, as follows:
2-64 Project Description Section 2.8.1.6 Advanced Oxidation Process System  Advanced Oxidation Process System Oxidation Process System Oxidation Process System  Advanced Oxidation Process System Oxidation Process System  Advanced Oxidation Process System Oxidation Process System  Advanced Oxidation Process System Oxidation Process System Oxidation Oxidation Process System  Advanced Oxidation Oxidat	[ S	Description Section 2.8.1.5 Reverse Osmosis Membrane Treatment	A reverse osmosis process that employs semi-permeable membranes is proposed to remove dissolved salts, inorganic and organic constituents, and pathogens from the membrane filtration treated water. The proposed reverse osmosis system would consist of a single pass, which separates the membrane filtration filtrate feed water into a purified product stream (permeate) and a concentrated brine stream (concentrate). The proposed reverse osmosis system would include a second stage to increase the product water recovery.  The proposed reverse osmosis system would include individual process trains, housing the process membranes in pressure vessels along with connecting piping and valve manifolds for feed, permeate, concentrate, cleaning and flush supplies. The ancillary equipment for the overall reverse osmosis system would include a membrane cleaning system and permeate flush system. Reverse osmosis membrane cleaning chemicals would likely include proprietary anticipant anti-scalant chemicals, acid, and caustic detergent, stored on-site.  Feed to the reverse osmosis system would be delivered from the upstream membrane filtration system through an intermediate equalization tank. Low-pressure booster a MF filtrate tank. Transfer pumps would move the water into the pretreatment system. Pretreatment would include cartridge filters, followed by the addition of an antiscalant and acid to lower the pH, which would be injected into a low-pressure line. High-pressure feed pumps would move the water from pretreatment into the reverse osmosis treatment trains. Concentrate from the reverse osmosis system would be discharged to into a new brine mixing structure wet well, where it would be combined with other effluent streams to enable adequate final disposal effluent sampling, and then disposed through the existing MRWPCA ocean outfall. Product water would flow to the advanced oxidation system. Separate cleaning and flush system
Description Section 2.8.1.6 Advanced Oxidation Process System  Oxidation Process System  Description Section 2.8.1.6 Advanced Oxidation Support section and an additional chemical destruction barrier for the reverse osmosis of a chemical feed to add peroxide and reactors housing arrays of ultraviolet lamps along with ballasts to poultraviolet system. Ultraviolet light reacts with hydrogen peroxide to form hydroxy which, along with the ultraviolet light, oxidizes, destroys oxidize, destroy, or inactive inactivate chemicals of concern and pathogens. The system sizing would be driven requirement in the California Code of Regulations, Title 22, §60320.200 et seq., "In Potable Reuse: Groundwater Replenishment – Subsurface Application" criteria for oxidation. Support facilities for the reactors would include chemical storage and members of the pumps, and ballasts. The advanced oxidation product water would be directed to the treatment system for stabilization.  Project  Description  Advanced  Advanced  Advanced oxidation system would consist of a chemical feed to add peroxide and reactors housing arrays of ultraviolet lamps along with ballasts to poult of the provide and reactors housing arrays of ultraviolet lamps along with ballasts to poult of the provide and reactors would consist of a chemical feed to add peroxide to add perox	2.64	Project	
	[ S	Description Section 2.8.1.6 Advanced Oxidation Process System	The proposed advanced oxidation system would provide a final polishing step for pathogen disinfection and an additional chemical destruction barrier for the reverse osmosis permeate. The proposed advanced oxidation system would consist of a chemical feed to add hydrogen peroxide and reactors housing arrays of ultraviolet lamps along with ballasts to power the ultraviolet system. Ultraviolet light reacts with hydrogen peroxide to form hydroxyl radicals, which, along with the ultraviolet light, oxidizes, destroys-oxidize, destroy, or inactivates inactivate chemicals of concern and pathogens. The system sizing would be driven by the requirement in the California Code of Regulations, Title 22, §60320.200 et seq., "Indirect Potable Reuse: Groundwater Replenishment – Subsurface Application" criteria for advanced oxidation. Support facilities for the reactors would include chemical storage and metering pumps, and ballasts. The advanced oxidation product water would be directed to the post-treatment system for stabilization.
Section 2.8.1.7 treatment system. Due to the high removal of minerals that is achieved through representation of the product water would be needed.	[ S	Description Section 2.8.1.7 Post-Treatment	Amend Section 2.8.1.7 Post-Treatment System, as follows:  Product water from the advanced oxidation process would be sent to the proposed post- treatment system. Due to the high removal of minerals that is achieved through reverse osmosis treatment, post-treatment stabilization of the product water would be needed to prevent corrosion of pipe materials in the product water conveyance system. Stabilization

would also be used to reduce the potential for product water to leach minerals and other chemicals from the soils within the Seaside Groundwater Basin upon injection. Reverse osmosis permeate is a soft, low alkalinity water, and the final product water quality would be adjusted to specific goals for hardness, alkalinity, and pH. This adjustment would include decarbonation by air stripping to remove carbon dioxide (CO<sub>2</sub>), the addition of calcium and alkalinity, and pH adjustment with CO2 addition. There are two proposed options for calcium and alkalinity adjustment: (1) the addition of purchased hydrate lime slurry (calcium hydroxide slurry), or (2) addition of sodium hydroxide (NaOH) and calcium chloride (CaCl2). Sodium hypochlorite may be added to the product water for secondary disinfection.) and the addition a hydrated lime slurry (calcium hydroxide slurry). 2-64 Project Amend Section 2.8.1.9 Brine Mixing Facility, as follows: & 2-Description As discussed above, the new AWT Facility would produce reverse osmosis concentrate water 65 Section 2.8.1.9 that would be disposed or discharged via the MRWPCA's existing ocean outfall. In addition to **Brine Mixing** the AWT reverse osmosis reject water, other water that is currently discharged to the outfall Facility includes secondary effluent from the Regional Treatment Plant, and brine waste collected from individual water softeners and private desalination facilities and delivered by truck to the Regional Treatment Plant. Proper disposal of these waste streams to the outfall, and

As discussed above, the new AWT Facility would produce reverse osmosis concentrate water that would be disposed or discharged via the MRWPCA's existing ocean outfall. In addition to the AWT reverse osmosis reject water, other water that is currently discharged to the outfall includes secondary effluent from the Regional Treatment Plant, and brine waste collected from individual water softeners and private desalination facilities and delivered by truck to the Regional Treatment Plant. Proper disposal of these waste streams to the outfall, and eventually the ocean, requires flow metering and water quality sampling and monitoring. The brine mixing facility at the AWT Facility is not necessary for the PWM/GWR Project. While a new brine mixing facility is not currently under construction, it could still be constructed at the Regional Treatment Plant in the future for the purpose of mixing and monitoring seawater desalination plant brine (such as from the proposed Monterey Peninsula Water Supply Project desalination plant), RO concentrate, and unused secondary effluent. In addition, the site of the previously proposed brine mixing facility has been changed because it is not needed for disposing reverse osmosis concentrate, it is only needed for disposing brine from a potential future desalination plant; therefore, it has been located in closer proximity to a proposed brine return line and the outfall. The proposed new brine mixing facility would be located west of the AWT Facility and would accomplish the required mixing, metering and sampling, using the following key processes and facilities:

- Two (2) A diversion structure, comprised of a cast-in-place, two-chamber, concrete vaults structure on the existing 60-inch diameter land outfall, one to divert secondary treated effluent to pipeline,
- Piping between the <u>diversion structure on the land outfall and the brine</u> mixing facility and one approximately 170-ft downstream basins,
- Four (4), below grade, brine mixing basins, operating in parallel, each with a single mechanical mixer, and
- A flow meter to measure the total mixed flow returned from the mixing basins to the diversion structure and outfall.

### Ancillary facilities would include the following:

- A flow bypass system on the outfall to return the blended carry flows to in the land outfall. Both structures would be equipped with two around the diversion structure, including valves, slide gates gate, pipe and fittings and a bypass manhole. The bypass system enables construction of the diversion structure and maintenance and repairs of the structure in dry conditions.
- A trucked brine station to receive and measure trucked brine waste prior to control the amount of secondary effluent diverted through the mixing facility and

passed through mixing with other flows and eventual discharge to the outfall.

- A cast-in-place concrete mixing structure, configured to receive secondary effluent and brine waste from separate inflow pipes and equipped with a 60-inch (nominal) static mixer in a fiberglass mixing pipe and an air release valve on the upstream end of the static mixer
- A 54-inch pipeline (high density polyethylene) from the diversion vault to the mixing structure and then to the return vault
- 48-inch flow meters on the pipelines entering and leaving the mixing structure, installed below grade in concrete boxes
- A sampling port in the return vault for access to measure total dissolved solids, pH, dissolved oxygen temperature, and other constituents of the blended effluent as required by permit conditions—Sampling pumps and pipeline to collect samples of pre- and post-mixed flows for analysis.
- Flow bypass system for the CAW brine waste flow and trucked brine in the event the Diversion structure is out of service for maintenance or repair.
- Class "C" water connection for washing down equipment and facilities.

Only one new above-grade structure, the Lab and Control Building would be built and would receive architectural treatment similar to the other buildings at the Regional Treatment Plant. The maximum depth of excavation would be 30 to 32 feet. A new cast concrete driveway would extend from the existing road on the north side to the Lab and Control Building delivery door on the north side. A new four-foot wide concrete walkway would extend along the south side. Storm water drainage would be directed through site grading to a new retention basin at the west end of the site for percolation.

2-65 Project
Description
Section 2.8.1.10
Power Supply

### Amend Section 2.8.10 Power Supply, as follows:

The AWT Facility power would be supplied through a two new PG&E utility connection connections, one from the Monterey Regional Waste Management District (MRWMD) adjacent landfill and one from PG&E to the Regional Treatment Plant. The system components would include a utility service, transformers, and switchgear. The major electrical loads would be from the new influent pumping, oxygen generator (if liquid oxygen is not used), ozone generator, biological filtration backwash pumps (if included in the final system), ozone generator, membrane filtration and reverse osmosis feedwater pumping, ultraviolet light reactors, and product water pumping. In the case of a power failure, the AWT Facility would shut down and the secondary treated influent water would bypass the AWT Facility and be discharged to Monterey Bay, if not used first by the Salinas Valley Reclamation Plant. The Regional Treatment Plant has three power supplies: cogeneration, utility connection, and a standby diesel generator. If all three power supplies fail, there are provisions to connect mobile generators to the critical facilities. See Updated Table 2.12, Overview of Proposed **Project Electricity Demand** for a summary of the power demands of the Expanded Capacity Treatment Facilities at the Regional Treatment Plant. (Source: V. Badani, E2 Consulting Engineers; A. Wesner, SPI Engineering; B. Holden MRWPCA; T.G. Cole, October 2014; Kennedy Jenks, September 2017)

Page Project 2-6.5 Description Section 2.8.1.12. Under AWT Facility Operation 3 follows:  Water residuals would include backwash from the biological filtration system (if included_bloogically active filtration was approved for the Project, but it may not be required), backwash and cleaning wastes from the membrane filtration treatment system and concentrate and cleaning wastes from the reverse osmosis system. Cleaning wastes from each system would be neutralized in the waste water equalization basin and returned to the head of the Regional Treatment Plant, adong with backwash woste residuals from the membrane treatments system, Reverse osmosis concentrate would be discharged through a new-brian extent system, Reverse osmosis concentrate would be discharged through a new-brian extent system, Reverse osmosis concentrate would be discharged through a new-brian extent system, Reverse osmosis concentrate would be discharged through a new-brian extent system. Reverse osmosis concentrate would be discharged through a new-brian extent system, Reverse osmosis concentrate would be discharged through a new-brian extent system. Reverse osmosis concentrate would be discharged through a new-brian extent system. Reverse osmosis concentrate would be discharged through a new-brian extent system. Reverse osmosis concentrate would be discharged through a new-brian extended to proposed project construction.  Page Project Construction Schedule is provided in Figure 2-40, Proposed Project Construction Schedule is provided in Figure 2-40, Proposed Project Construction Schedule is provided in Figure 2-40, Proposed Project Construction Schedule is provided in July 2-40, Proposed Project Construction Schedule is provided in Figure 2-40, Proposed Project Construction Schedule is provided in Figure 2-40, Proposed Project Construction Schedule is provided in Figure 2-40, Proposed Project Construction Schedule is provided in Figure 2-40, Proposed Project Construction Schedule is provided in Figure 2-40, Proposed Project Construction Schedul	Dago	Droject	Amond Section 2.9.1.12 Under AWT Facility Operation, as follows:
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		Addendum No. 1 to the ASR EIR/EA and the PWM/GWR EIR for the Hilby Avenue Pump Station
		and Monterey Pipeline on June 20, 2016.
Page	Project	Amend Table 2-21. Proposed Project Construction Assumptions to update AWT Facility (
2-87	Description	See Updated Tables following this section
	Table 2-21	AWT Facility
		Inlet source water diversion structure and influent pump station to bring secondary effluent
		AWT Facility, prescreening, ozonation, upflow biologically active filtration (optional), chemical
		addition, membrane filtration treatment, booster pumping of the membrane filtration filtrate
		(potentially with intermediate storage), cartridge filtration (optional), chemical addition,
		reverse osmosis membrane treatment, advanced oxidation using ultraviolet light and hydrogen
		peroxide (advanced oxidation), decarbonation (optional), product-water stabilization with
		calcium, alkalinity and pH adjustment liquid lime, product water pump station (AWT Pump
		Station), brine mixing facilities.

### **Updated Tables**

- Table 2-11. Overview of Proposed Project Electricity Demand
- Table 2-18. AWT Facilities Design Summary
- Table 2-19. Proposed Project AWT Facility Process Design Flow Assumptions
- Table 2-20 under Product Water Conveyance Facilities
- Table 2-21. Proposed Project Construction Assumptions for AWT Facility Components
- Table 4.9.6 Chemicals to be Utilized at the Advanced Water Treatment Facility

# **Updated Table 2-11. Overview of Proposed Project Electricity Demand**

Updated Revised Table 2-11	4.0 MGD	5.0 MGD
Overview of Proposed Project Electricity Demand (all in megawatt-hours per year)	EIR 2015	Addendum
Source Water Diversion and Storage Sites (Source: Vinod Badani, E2 Consulting, October 2014, except as noted	)	
Existing MRWPCA Wastewater Collection System Pump Stations	1100	1100
(increased pumping for source water collection) (Source: Bob Holden, MRWPCA, October 2014)		
Proposed Salinas Pump Station Diversions	10	10
(lighting, SCADA, misc. electricity)[Note: this facility now operates almost exclusively using solar energy.]		
Proposed Salinas Industrial Wastew ater Treatment Plant Storage and Recovery Component	224	100
(pumping, lighting, SCADA, misc. electricity)		
Existing Salinas Treatment Facility and Stormwater Operations	-1875	-1875
(reduction of pumping, Ron Cole, February 2014 modified by MRWPCA staff October 2014)		
Proposed Reclamation Ditch Diversion	250	250
(pumping, lighting, SCADA, misc. electricity)		
Proposed Tembladero Slough Diversion	461	461
(pumping, lighting, SCADA, misc. electricity)		
Proposed Blanco Drain Diversion	731	731
(pumping, lighting, SCADA, misc. electricity)		
Proposed Lake El Estero Diversion	10	10
(lighting, SCADA, misc. electricity)		
Treatment Facilities at Regional Treatment Plant (Source: Bob Holden, October 2014)		
Existing Primary and Secondary Processes	3673	3673
(existing on-site cogeneration facility would provide a reduction in this value, see below)		
(9,900 AFY more w astew ater flows through treatment processes)		
Existing Salinas Valley Reclamation Plant	1300	1300
(existing plant operations use solar array electricity, which has reduced electricity demand by up to 1,400 mWhr/yr)		
(4,260 AFY more crop irrigation w ater produced)		
4.0 AWT Facility (2015 GWR ⊟R)	7007	0
(new treatment facilities, not including product water pumping; assumes 3,700 AFY of water production to build drought reserve; demand will be less when Drought Reserve is at full capacity and when Drought Reserve is being used by CSIP)		
5.0 Expanded Capacity AWT Facility assumes 4,300 AFY of water production (Source: Kennedy Jenks September 2017)	0	<u>12930</u>
CSIP Supplemental Wells (Source: Bob Holden, MRWPCA, October 2014)		
Reduction of use of CSIP Supplemental Wells by 4,260 AFY	-1900	-1900
Product Water Conveyance (Source: TG Cole, October 2014)		
Pumping of product water to Injection Well Facilities under RUWAP (1)	1912	0
Injection Well Facilities (Source: Vinod Badani, E2 Consulting Engineers, October 2014)		
Back-flush of four (4) deep injection wells, lighting, HVAC, meters, instruments, SCADA	147	147
CalAm Distribution System Changes (Source: CalAm, 2014)		•
Increase by moving 3,500 AFY extractions from Carmel River to Seaside Basin wells	630	630
Proposed New Electricity Generation at MRWPCA Existing Cogeneration Facility	-2726	-2726
New Purchased Electricity from Monterey Regional Waste Management District (2)		-14200
NET TOTAL (with reduction in energy demand from renewable energy sources)	10,954	
(1) GWR EIR and RUWAP EIR each proposed two parallel pipelines; reduction to one pipeline and no pump sta	ations along con	veyance line
(2) The Monterey Regional Waste Management District (MRWMD) utilizes biogas produced by the decompositi	on of waste	
material in the landfill to produce electrical energy. MRWMD will provide 1800KwH for AWPF operation at the	e site.	
The RTP is adjacent to the landfill and power generation facility operated by MRWMD.		
Source: MRWPCA and Kennedy Jenks, September 2017		

### **Updated Table 2-18. AWT Facilities Design Summary**

### **Updated Table 2-18 AWT Facilities Design Summary**

Component	Design Capacity (See Note a)
Pipeline from secondary treatment system outfall pipe to AWT Facility	N/A
AWT Facility Influent Wetwell	<del>0.2 mg</del>
Influent Pumping (see Note b) <u>Secondary Effluent Diversion Structure, Source Water Pump Station, and Chloramination</u>	2.7 to 5 <u>6</u> .9 mgd
Ozone System <del>(see Note b)</del>	<del>5</del> <u>6</u> .9 mgd
Biologically Active Filtration (may not beif required) (see Note c)	<del>5.5 mgd</del>
Membrane Filtration System	<del>4</del> <u>6</u> .9 mgd
Reverse Osmosis System	<u>6</u> .2.2 to 4.9 mgd
Advanced Oxidation System, Product Water Stabilization and Pumping Product Water Pump Station	<u>45</u> .0 mgd

#### Notes:

- a. Capacities represent process feedwater flows; units are million gallons (mg) and million gallons per day (mgd).
- b. For the case where biological filtration is not included, the range for the influent pumping would be 2.7 to 5.5 mgd, and the ozone system would be sized for 5.5 mgd.
- c. The biologically active filtration would be sized to treat up to 80 percent of the process flow; the 5.5 mgd represents the total product flow when combined with the by pass. The Biologically Active Filtration is not included in the PWM/GWR Project; it may be constructed later at the AWTF if it is required as mitigation for California Ocean Plan compliance (for disposal of reverse osmosis concentrate to the MRWPCA ocean outfall).

# Updated Table 2-19. Proposed Project AWT Facility Process Design Flow Assumptions

### Updated Table 2-19 Proposed Project AWT Facility Process Design Flow Assumptions

	Annual Flows <sup>1</sup>	Average Flow Conditions <sup>1</sup>	Maximum Flow Conditions <sup>2</sup>
AWT Facility Process	AFY	mgd	mgd
Source Water Pump Station and Ozone System Feed	5, <del>496</del> <u>898</u>	<del>4.9</del> 5.3	<del>5</del> <u>6</u> .9
Biologically Active Filtration Feed	4,481	4.0	4.8
Biologically Active Filtration Backwash returned to Regional Treatment Plant Headworks	<del>421</del>	0.4	<del>0.5</del>
Biologically Active Filtration Bypass <sup>3</sup>	<del>1,015</del>	0.9	1.1
Membrane Filtration Feed	5, <del>075</del> <u>898</u>	<del>4.</del> 5 <u>.3</u>	<del>5.5</del> <u>6.9</u>
Membrane Filtration Backwash retuned to Regional Treatment Plant Headworks	<del>508</del> 590	0.5	0. <del>6</del> <u>7</u>
Reverse Osmosis Feed	<del>4,567</del> <u>5,309</u>	4. <del>1</del> 7	4.9 <u>6.2</u>
Reverse Osmosis Concentrate	<del>867</del> 1,009	0. <u>89</u>	<del>0.9</del> 1.2
Reverse Osmosis Product Water (AWT Facility Design Size)	<del>3,700</del> 4,300	3. <del>3</del> <u>8</u>	<u>45</u> .0
Advanced Oxidation Process, <u>Product Water Stabilization, and</u> <u>Product Water Pump Station</u>	<del>3,700</del> 4,300	3. <del>3</del> <u>8</u>	<del>4</del> <u>5</u> .0

#### Notes

- <sup>1</sup>. Average annual flows reflect <del>3,700</del> <u>4,300</u> AFY, typical annual production while building the drought reserve.
- $^{\rm 2}.$  Maximum flow condition reflects design peak production rate.
- <sup>3</sup>. 80% of the flow would pass through the Biologically Active Filtration, and 20% may bypass directly to the membrane filtration
  Although Biologically Active Filtration will not be included in the PWM/GWR Project, it may be constructed later at the AWT Facility if it is required as mitigation for California Ocean Plan compliance for disposal of reverse osmosis concentrate to the MRWPCA ocean outfall.

# Updated Table 2-20 under Product Water Conveyance and Shared Facilities Construction Area of Disturbance and Permanent Footprint

Combined Duadout Water Commerces		Construction Permanent Component Footpri Boundary (feet)			int (feet)		
Combined Product Water Conveyance Facilities and Blackhorse Reservoir Project Components	Length	Width	Length	Width	Maximum Height (above ground surface)	Maximum Depth (below ground surface)	
Product Water Pipelines							
RUWAP Pipeline from AWT to Injection Wells	46,900	<u> 10 – 15</u>	<u>46,900</u>	<u>&lt;6</u>	<u>0</u>	10 (trenched	
RUWAP Pipeline from Gen. Jim Moore to Blackhorse Reservoir	<u>3,840</u>	<u>"</u>	<u>3,840</u>	<u>"</u>	<u>o</u>	sections); 25 (trenchless sections	
TOTAL Conveyance Pipeline	<u>50,074</u>		0	0	0	and pits)	
Approved Blackhorse Reservoir	Dian	neter		•			
Tank/Reservoir	<u>120</u>				<u>32</u>		
Eliminated Components Shared Components eliminates ti	he followin	g redundant j	facilities and ar	eas of impact	<u>:</u>		
RUWAP AWT to Booster Pump Station	<del>28,000</del>	<del>10 <b>–</b> 15</del>	<del>28,000</del>	<del>&lt;6</del>	θ		
RUWAP Booster Pump Station to Injection Wells	<del>18,900</del>	<del>10 - 15</del>	<del>18,900</del>	<del>&lt;6</del>	θ		
RUWAP Booster Pump Station (one of two optional sites)	<del>100</del>	<del>60</del>	<del>80</del>	<del>60</del>	<u>25</u>	<del>10</del>	
Note: 2.0 MG Blackhorse Reservoir (tank) would have a footprint of approximately 11,000 square feet (120 ft. diameter).  Additional RUWAP Booster Pump Station in Marina also eliminated,							

(Source: PWM/GWR EIR, October 2015)

# Updated Table 2-21. Proposed Project Construction Assumptions for AWT Facility Components

Project Component	Excess Spoils/Debris to Off-Haul (cubic yards)	Construction Equipment	Construction Shifts and Work Hours (
Treatment Facilities at the Regional	Treatment Plant		
AWT Facility Inlet source water diversion structure and influent pump station to bring secondary effluent AWT Facility, prescreening, ozonation, upflow biologically active filtration (optional), chemical addition, membrane filtration treatment, booster pumping of the membrane filtration filtrate (potentially with intermediate storage), cartridge filtration (optional), chemical addition, reverse osmosis membrane treatment, advanced oxidation using ultraviolet light and hydrogen peroxide (advanced oxidation), decarbonation (optional), product-water stabilization with ealcium, alkalinity and pH adjustment liquid lime, product water pump station (AWT Pump Station), brine mixing facilities.	510	Excavators, backhoes, air compressors, loaders, boom trucks, cranes, pavers and rollers, concrete transport trucks, concrete pump trucks, flatbed trucks, generators, pickup trucks, trucks for materials delivery	Up to four (4) shifts with construction occurring 24- hours per day, 7 days per week

# Updated Table 4.9.6 Hazardous Materials Chemicals to be Utilized at the Advanced Water Treatment Facility

### **Updated Table 4.9-6**

Chemicals to be Utilized at the Advanced Water Treatment Facility

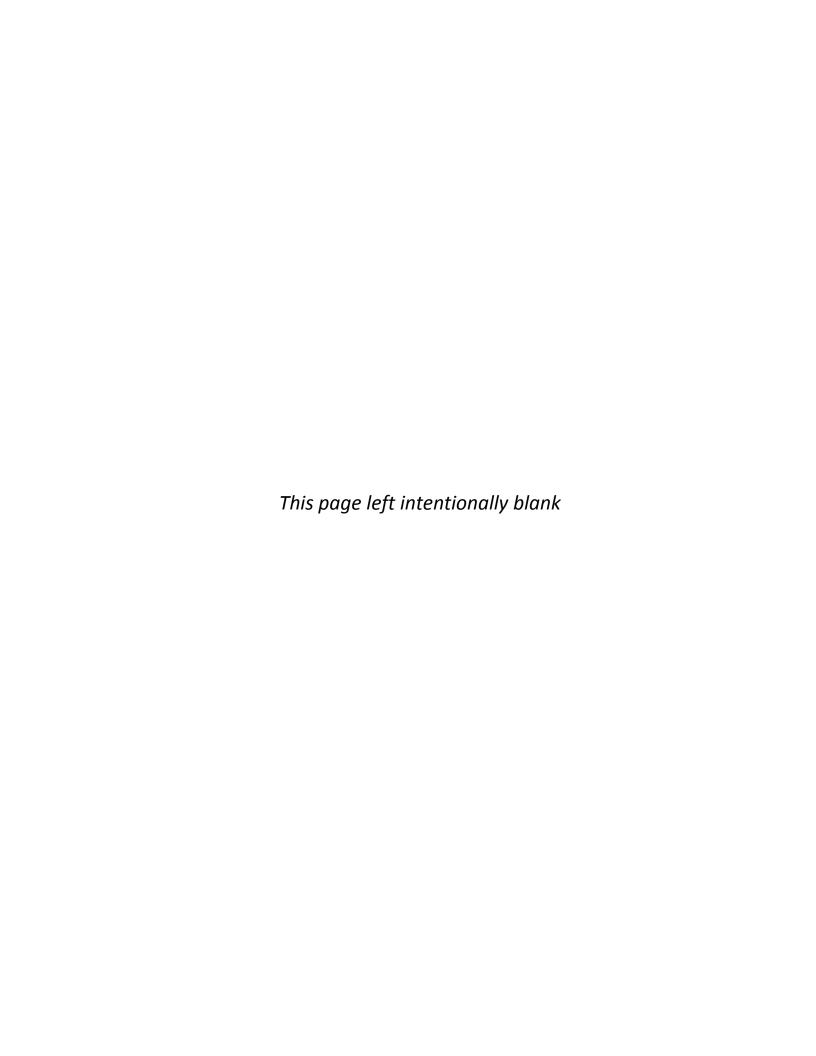
01!1	Chamical Application Application								
Chemical	Application	Annual Usage (pounds)							
Sodium Hypochlorite	Ozone Feed	270,000 (avg), 560,000 (max)							
Liquid Oxygen (LOX)	Ozone Feed	2,200,000 (avg), 5,600,000 (max)							
Sodium Bisulfite	Ozone Effluent	5,200 (avg), 10,000 (max)							
Sodium Hypochlorite	MF Cleaning	50,000 (avg), 61,000 (max)							
Sodium Hydroxide	MF Cleaning	72,000 (avg), 84,000 (max)							
Sulfuric Acid	MF Cleaning	20 (avg), 23 (max)							
Sulfuric Acid	Reverse Osmosis Feed	2,600,000 (avg), 5,100,0000 (max)							
Threshold inhibitor	Reverse Osmosis Feed	43,000 (avg), 51,000 (max)							
Hydrogen Peroxide	UV/AOP Feed	41,000 (avg), 82,000 (max)							
Ammonium Sulfate	Product Water	22,00 (avg), 51,000 (max)							
Sodium Hypochlorite	Product Water	23,000 (avg), 55,000 (max)							
Slurry of Hydrated Lime	Product Water	530,000 (avg), 960,000 (max)							
Sodium Bisulfite	Reverse Osmosis Concentrate Dechlorination	33,000 (avg), 38,000 (max)							
Tri-Sodium Phosphate	Reverse Osmosis Cleaning	5,000 (avg), 5,900 (max)							
Sodium Dodecyl Benzene Sulfonate	Reverse Osmosis Cleaning	5,000 (avg), 5,900 (max)							
Sodium Hydroxide	Reverse Osmosis Cleaning	12 (avg), 14 (max)							
Sulfuric Acid	Waste Equalization Basin	92,000 (avg), 110,000 (max)							
Sodium Hydroxide	Waste Equalization Basin	17 (avg), 20 (max)							
Sodium Bisulfite	Waste Equalization Basin	99,000 (avg), 120,000 (max)							
Ferric Chloride	Waste Equalization Basin	34,000 (avg), 80,000 (max)							

Note: Average annual usage based on average dose for building reserve scenario flow scenario (4,300 AFY production); maximum annual usage based on maximum dose and capacity (5 mgd with 10% downtime).

Biologically Active Filtration would require additional chemicals not included in this table.

# Appendix B: Adopted PWM/GWR and RUWAP Mitigation Monitoring and Reporting Program

1 – PWM/GWR MMRP 2 -RUWAP MMRP



#### MITIGATION MONITORING AND REPORTING PROGRAM

# for the Pure Water Monterey Groundwater Replenishment Project: Staff-Recommended Alternative (October 1, 2015)

#### INTRODUCTION

Section 21081.6 of the California Public Resources Code and Section 15091(d) and Section 15097 of the California Environmental Quality Act (CEQA) Guidelines require public agencies "to adopt a reporting or monitoring program for changes to the project which it has adopted or made a condition of project approval in order to mitigate or avoid significant effects on the environment." This Mitigation Monitoring and Reporting Program (MMRP) has been prepared for the Pure Water Monterey Groundwater Replenishment (GWR) Project, as modified by the Alternative Monterey Pipeline, and reflecting selection of the Regional Urban Water Augmentation Project (RUWAP) alignment for the Product Water Conveyance pipeline and booster pump station. This MMRP is based on the mitigation measures included in the Final Environmental Impact Report (EIR).

This MMRP is applicable to the Staff-Recommended Alternative of the GWR Project. The Staff-Recommended Alternative includes the RUWAP Alignment Option for the Product Water Conveyance pipeline and booster pump station and the Alternative Monterey Pipeline for the CalAm Distribution System Improvements. Therefore, this MMRP includes mitigation measures, monitoring and reporting requirements identified in the Final EIR for these two project components, and it does not include mitigation measures identified for the originally proposed Monterey or Transfer Pipelines of the CalAm Distribution System Improvements, nor the Coastal Alignment Option for the Product Water Conveyance pipeline and booster pump station, since those components are not recommended for approval. Mitigation measures, monitoring and reporting requirements for all other GWR Project components, as modified by the Alternative Monterey Pipeline, are included herein.

For a complete list of acronyms used in this document, please refer to the acronym list in the Draft EIR on pages xii through xvi.

1

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Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
Impact AE-2: Construction Impacts due to Temporary Light and Glare	Mitigation Measure AE-2: Minimize Construction Nighttime Lighting. As part of its contract specifications, MRWPCA shall require its construction contractors to implement site-specific nighttime construction lighting measures for nighttime construction at the proposed Injection Well Facilities site and for the CalAm Distribution System: Alternative Monterey Pipeline. The measures shall, at a minimum, require that lighting be shielded, directed downward onto work areas to minimize light spillover, and specify that construction lighting use the minimum wattage necessary to provide safety at the construction sites. MRWPCA shall ensure these measures are implemented at all times during nighttime construction at the Injection Well Facilities site and for the CalAm Distribution System: Alternative Monterey Pipeline and for the duration of all required nighttime construction activity at these locations.	Injection Well Facilities Site and CalAm Distribution System: Alternative Monterey Pipeline	In contract specifications and during project construction	MRWPCA, CalAm, construction contractors	During project construction	MRWPCA and CalAm
Impact AE-3: Degradation of Visual Quality of Sites and Surrounding Areas	Mitigation Measure AE-3: Provide Aesthetic Screening for New Above-Ground Structures. Proposed above-ground features at the Booster Pump Station and Injection Well Facilities (at a minimum, at the well clusters and back-flush basin), shall be designed to minimize visual impacts by incorporating screening with vegetation, or other aesthetic design treatments, subject to review and approval of the City of Seaside which has also requested that the buildings be designed with Monterey/Mission style architecture to match the design of the structures that have been built on the Santa Margarita ASR site and the Seaside Middle School ASR Site. All pipelines placed within the City of Seaside on General Jim Moore Boulevard shall be placed underground. MRWPCA shall coordinate with the City of Seaside on the location of injection wells and booster pumps in order to reduce conflicts with future commercial/residential development opportunities. Screening and aesthetic design treatments at the RUWAP Booster Pump Station component shall be subject to review and approval by the City of Marina. Use of standard, commercial-grade, chain link fencing and barbed wire should be discouraged.	RUWAP Booster Pump Station and Injection Well Facilities	Prior to City of Seaside and City of Marina issuance of grading, easements/ ROW permits	MRWPCA project engineers and contractors	During project construction	MRWPCA; Cities of Seaside and Marina (public works directors)
Impact AE-4: Impacts due to Permanent Light and Glare during Operations	<ul> <li>Mitigation Measure AE-4: Exterior Lighting Minimization. To prevent exterior lighting from affecting nighttime views, the design and operation of lighting at the RUWAP Product Water Conveyance Booster Pump Station and Injection Well Facilities, shall adhere to the following requirements:</li> <li>Use of low-intensity street lighting and low-intensity exterior lighting shall be required. No floodlights shall be allowed at night within the City of Marina.</li> <li>Lighting fixtures shall be cast downward and shielded to prevent light from spilling onto adjacent offsite uses.</li> <li>Lighting fixtures shall be designed and placed to minimize glare that could affect users of adjacent properties, buildings, and roadways.</li> <li>Fixtures and standards shall conform to state and local safety and illumination requirements.</li> </ul>	RUWAP Booster Pump Station and Injection Well Facilities	Prior to City of Seaside and Marina issuance of grading and easements/ ROW permits	MRWPCA project engineers and contractors	During project operation	MRWPCA; Cities of Seaside and Marina (public works directors)
Impact AQ-1: Construction Criteria Pollutant Emissions	<ul> <li>Mitigation Measure AQ-1: Construction Fugitive Dust Control Plan. The following standard Dust Control Measures shall be implemented during construction to help prevent potential nuisances to nearby receptors due to fugitive dust and to reduce contributions to exceedances of the state ambient air quality standards for PM<sub>10</sub>, in accordance with MBUAPCD's CEQA Guidelines.</li> <li>Water all active construction areas as required with non-potable sources to the extent feasible; frequency should be based on the type of operation, soil, and wind exposure and minimized to prevent wasteful use of water.</li> <li>Prohibit grading activities during periods of high wind (over 15 mph).</li> <li>Cover all trucks hauling soil, sand, and other loose materials and require trucks to maintain at least 2 feet of freeboard.</li> <li>Sweep daily (with water sweepers) all paved access roads, parking areas, and staging areas at construction sites.</li> <li>Sweep streets daily (with water sweepers) if visible soil material is carried onto adjacent public streets.</li> <li>Enclose, cover, or water daily exposed stockpiles (dirt, sand, etc.).</li> <li>Replant vegetation in disturbed areas as quickly as possible.</li> <li>Wheel washers shall be installed and used by truck operators at the exits of the construction sites to the AWT Facility site, the</li> </ul>	All components	During project construction	MRWPCA, CalAm project engineers and contractors	During project construction	MRWPCA, CalAm, and MBUAPCD

<sup>&</sup>lt;sup>1</sup> CalAm Distribution System: Alternative Monterey Pipelines and the associated mitigation measures would be the responsibility of CalAm to implement and the local jurisdictions and/or the California Public Utilities Commission to monitor.

Pure Water Monterey GWR Project – Staff Recommended Alternative

October 2015 Denise Duffy & Associates, Inc.

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	<ul> <li>Injection Well Facilities, and the Booster Pump Station.</li> <li>Post a publicly visible sign that specifies the telephone number and person to contact regarding dust complaints. This person shall respond to complaints and take corrective action within 48 hours. The phone number of the MBUAPCD shall also be visible to ensure compliance with MBUAPCD rules.</li> </ul>					
	Mitigation Measure BF-1a: Construction during Low Flow Season. Implement Mitigation Measure BT-1a. Conduct construction of diversion facilities, including the directional drilling under the Salinas River, during periods of low flow outside of the SCCC steelhead migration periods, i.e. between June and November, which would be outside of the adult migration period from December through April and outside of the smolt migration period from March through May.	Reclamation Ditch, Tembladero Slough, and Blanco Drain Diversions	Prior to commencing construction	MRWPCA engineers and contractors	During construction	MRWPCA
	Mitigation Measure BF-1b: Relocation of Aquatic Species during Construction. Conduct pre-construction surveys to determine whether tidewater gobies or other fish species are present, and if so, implement appropriate measures in consultation with applicable regulatory agencies, which may include a program for capture and relocation of tidewater gobies to suitable habitat outside of work area during construction. Pre-construction surveys shall be consistent with requirements and approved protocols of applicable resource agencies and performed by a qualified fisheries biologist.	Reclamation Ditch and Tembladero Slough Diversions	Prior to project construction	Qualified biologists	Prior to construction	MRWPCA
Impact BF-1: Habitat Modification Due to Construction of Diversion Facilities	Mitigation Measure BF-1c Tidewater Goby and Steelhead Impact Avoidance and Minimization. To ensure compliance with the federal Endangered Species Act (FESA) and the California Endangered Species Act (CESA), consultation with NFMS/NOAA, USFWS, and CDFW shall be conducted as required, and any necessary take permits or authorizations would be obtained. If suitable habitat for tidewater goby (Tembladero Slough) and steelhead cannot be avoided, any in-stream portions of each project component (where the Project improvements require in-stream work) shall be dewatered/ diverted. A dewatering/diversion plan shall be prepared and submitted to NMFS, USFWS, and CDFW:  • Required Pre-Construction surveys identified in Mitigation Measure BF-1b shall be consistent with requirements and approved protocol of applicable resource agencies and performed by a qualified fisheries biologist.  • All dewatering/diversion activities shall be monitored by a qualified fisheries biologist. The fisheries biologist shall be responsible for capture and relocation of fish species out of the work area during dewatering/diversion installation.  • The project proponents shall designate a qualified representative to monitor on-site compliance of all avoidance and minimization measures. The fisheries biologist shall have the authority to halt any action which may result in the take of listed species.  • Only USFWS/NMFS/CDFW-approved biologists shall participate in the capture and handling of listed species subject to the conditions in the Incidental Take Permits as noted above.  • No equipment shall be permitted to enter wetted portions of any affected drainage channel. All equipment operating within streams shall be in good conditions and free of leaks.  • Spill containment shall be installed under all equipment staged within stream areas and extra spill containment and clean up materials shall be located in close proximity for easy access.  • Work within and adjacent to streams shall not occur between November 1 and June 1 unless otherwise appro	Reclamation Ditch and Tembladero Slough Diversions	Prior to project construction	MRWPCA Qualified biologists	During construction	MRWPCA, NMFS/NOAA, USFWS, CDFW

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	• Water turbidity shall be monitored by a qualified biologist or water quality specialist during all instream work. Water turbidity shall be tested daily at both an upstream location for baseline measurement and downstream to determine if project activities are altering water turbidity. Turbidity measures shall be taken within 50 feet of construction activities to rule out other outside influences. Additional turbidity testing shall occur if visual monitoring indicates an increased in turbidity downstream of the work area. If turbidity levels immediately downstream of the project rise to more than 20 NTUs (Nephelometric Turbidity Units) above the upstream (baseline) turbidity levels, all construction shall be halted and all erosion and sediment control devices shall be thoroughly inspected for proper function, or shall be replaced with new devices to prevent additional sediment discharge into streams.					
	The above mitigation is subject to review and approval for CESA and FESA requirements by approving agencies as identified above and may be modified to further reduce, avoid or minimize impacts to species.					
Impact BF-2: Interference with Fish	Mitigation Measure BF-2a: Maintain Migration Flows. Implement BF-1a, BF-1b, and BF-1c. Operate diversions to maintain steelhead migration flows in the Reclamation Ditch based on two criteria – one for upstream adult passage in Jan-Feb-Mar and one for downstream juvenile passage in Apr-May. For juvenile passage, the downstream passage shall have a flow trigger in both Gabilan Creek and at the Reclamation Ditch, so that if there is flow in Gabilan Creek that would allow outmigration, then the bypass flow requirements, as measured at the San Jon Gage of the Reclamation Ditch, shall be applied (see Hagar Environmental Science, Estimation of Minimum Flows for Migration of Steelhead in the Reclamation Ditch, February 27, 2015, in Appendix G-2, of the Draft EIR and Schaaf & Wheeler, Fish Passage Analysis: Reclamation Ditch at San Jon Rd. and Gabilan Creek at Laurel Rd. July 15, 2015 in Appendix CC of this Final EIR). If there is no flow in Gabilan Creek, then only the low flow (minimum bypass flow requirement as proposed in the project description) shall be applied, and these flows for the dry season at Reclamation Ditch as measured at the San Jon USGS gage shall be met. Note: If there is no flow gage in Gabilan Creek, then downstream passage flow trigger shall be managed based on San Jon Road gage and flows.	Reclamation Ditch Diversion	During project operations	MRWPCA	During project operations	MRWPCA, NMFS/NOAA, USFWS, CDFW
Migration	Alternately, as the San Jon weir located at the USGS gage is considered a barrier to steelhead migration and the bypass flow requirements have been developed to allow adult and smolt steelhead migration to have adequate flow to travel past this obstacle, if the weir were to be modified to allow steelhead passage, the mitigation above would not have to be met. Therefore, alternate Mitigation Measure BF-2a has been developed, as follows:  Mitigation Measure Alternate BF-2a: Modify San Jon Weir. Construct modifications to the existing San Jon weir to provide for steelhead passage. Modifications could include downstream pool, modifications to the structural configuration of the weir to allow passage or other construction, and improvements to remove the impediment to steelhead passage defined above.	Reclamation Ditch Diversion	Prior to project operations	Project engineers, construction	Prior to project operations	MRWPCA, NMFS/NOAA, USFWS, CDFW
	The above mitigation is subject to compliance with CESA and FESA and appropriate approving agencies may modify the above mitigation to further reduce, avoid, or minimize impacts to species.			contractors	operations	
Impact BT-1: Construction Impacts to Special-Status Species and Habitat	<ul> <li>Mitigation Measure BT-1a: Implement Construction Best Management Practices. The following best management practices shall be implemented during all identified phases of construction (i.e., pre-, during, and post-) to reduce impacts to special-status plant and wildlife species:</li> <li>1. A qualified biologist must conduct an Employee Education Program for the construction crew prior to any construction activities. A qualified biologist must meet with the construction crew at the onset of construction at the site to educate the construction crew on the following: 1) the appropriate access route(s) in and out of the construction area and review project boundaries; 2) how a biological monitor will examine the area and agree upon a method which would ensure the safety of the monitor during such activities, 3) the special-status species that may be present; 4) the specific mitigation measures that will be incorporated into the construction effort; 5) the general provisions and protections afforded by the USFWS and CDFW; and 6) the proper procedures if a special-status species is encountered within the site.</li> <li>2. Trees and vegetation not planned for removal or trimming shall be protected prior to and during construction to the maximum extent possible through the use of exclusionary fencing, such as hay bales for herbaceous and shrubby vegetation, and protective wood barriers for trees. Only certified weed-free straw shall be used, to avoid the introduction of non-native, invasive species. A biological monitor shall supervise the installation of protective fencing and monitor at least once per week until construction is complete to ensure that the protective fencing remains intact.</li> <li>3. Protective fencing shall be placed prior to and during construction to keep construction equipment and personnel from impacting</li> </ul>	All components	Prior to, during and after project construction	MRWPCA, CalAm, construction contractors and qualified biologist	Prior to and during project construction	MRWPCA, CalAm, qualified biologist and construction biological monitor; City of Seaside for Injection Well Facilities

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	<ul> <li>vegetation outside of work limits. A biological monitor shall supervise the installation of protective fencing and monitor at least once per week until construction is complete to ensure that the protective fencing remains intact.</li> <li>4. Following construction, disturbed areas shall be restored to pre-construction contours to the maximum extent possible and revegetated using locally-occurring native species and native erosion control seed mix, per the recommendations of a qualified biologist.</li> <li>5. Grading, excavating, and other activities that involve substantial soil disturbance shall be planned and carried out in consultation with a qualified hydrologist, engineer, or erosion control specialist, and shall utilize standard erosion control techniques to minimize erosion and sedimentation to native vegetation (pre-, during, and post-construction).</li> <li>6. No firearms shall be allowed on the construction sites at any time.</li> <li>7. All food-related and other trash shall be disposed of in closed containers and removed from the project area at least once a week during the construction period, or more often if trash is attracting avian or mammalian predators. Construction personnel shall not feed or otherwise attract wildlife to the area.</li> <li>8. To protect against spills and fluids leaking from equipment, the project proponent shall require that the construction contractor maintains an on-site spill plan and on-site spill containment measures that can be easily accessed.</li> <li>9. Refueling or maintaining vehicles and equipment should only occur within a specified staging area that is at least 100 feet from a waterbody (including riparian and wetland habitat) and that has sufficient management measures that will prevent fluids or other construction materials including water from being transported into waters of the state. Measures shall include confined concrete washout areas, straw wattles placed around stockpiled materials and plastic sheets to cover materials from becoming airbo</li></ul>					
Impact BT-1: Construction Impacts to Special-Status Species and Habitat	Mitigation Measure BT-1b: Implement Construction-Phase Monitoring. The project proponents shall retain a qualified biologist to monitor all ground disturbing construction activities (i.e., vegetation removal, grading, excavation, or similar activities) to protect any special-status species encountered. Any handling and relocation protocols of special-status wildlife species shall be determined in coordination with CDFW prior to any ground disturbing activities, and conducted by a qualified biologist with appropriate scientific collection permit. After ground disturbing project activities are complete, the qualified biologist shall train an individual from the construction crew to act as the onsite construction biological monitor. The construction biological monitor shall be the contact for any special-status wildlife species encounters, shall conduct daily inspections of equipment and materials stored on site and any holes or trenches prior to the commencement of work, and shall ensure that all installed fencing stays in place throughout the construction period. The qualified biologist shall then conduct regular scheduled and unscheduled visits to ensure the construction biological monitor is satisfactorily implementing all appropriate mitigation protocols. Both the qualified biologist and the construction biological monitor shall have the authority to stop and/or redirect project activities to ensure protection of resources and compliance with all environmental permits and conditions of the project. The qualified biologist and the construction monitor shall complete a daily log summarizing activities and environmental compliance throughout the duration of the project. The log shall also include any special-status wildlife species observed and relocated.	Salinas Pump Station, Salinas Treatment Facility, Blanco Drain Diversion, Product Water Conveyance: RUWAP Alignment (Pipeline and Booster Pump Station) and Injection Well Facilities	Prior to and during project construction	MRWPCA, qualified biologists	Prior to and during project construction	MRWPCA qualified biologist and construction biological monitor; CDFW
(continued)	<ol> <li>Mitigation Measure BT-1c: Implement Non-Native, Invasive Species Controls. The following measures shall be implemented to reduce the introduction and spread of non-native, invasive species:</li> <li>Any landscaping or replanting required for the project shall not use species listed as noxious by the California Department of Food and Agriculture (CDFA).</li> <li>Bare and disturbed soil shall be landscaped with CDFA recommended seed mix or plantings from locally adopted species to preclude the invasion on noxious weeds in the Project Study Area.</li> <li>Construction equipment shall be cleaned of mud or other debris that may contain invasive plants and/or seeds and inspected to reduce the potential of spreading noxious weeds, before mobilizing to arrive at the construction site and before leaving the construction site.</li> <li>All non-native, invasive plant species shall be removed from disturbed areas prior to replanting.</li> </ol>	All except Alternative Monterey Pipeline	During project construction	Construction contactors	During project construction	MRWPCA qualified biologist and construction biological monitor

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
Impact BT-1: Construction Impacts to Special-Status Species and Habitat (continued)	Mitigation Measure BT-1d: Conduct Pre-Construction Surveys for California Legless Lizard. The project proponents shall retain a qualified biologist to prepare and implement a legless Lizard management plan in coordination with CDFW, which shall include, but is not limited to, the protocols for pre-construction surveys, construction monitoring, and salvage and relocation. The management plan shall include, but is not limited to, the following:  • Pre-Construction Surveys, Pre-construction surveys for legless lizards shall be conducted in all suitable habitat proposed for construction, ground disturbance, or staging. The qualified biologist shall hold or obtain a CDFW scientific collection permit for this species. The pre-construction surveys shall use a method called "high-grading," The high grading method shall include surveying the habitat where legless lizards are most likely to be seen and captured (early morning, high soil moisture, overcast, etc.). The intensity of a continued search may then be adjusted, based on the results of the first survey in the best habitat. A "three pass method" shall be used to locate and remove as many legless lizards as possible. A first pass shall locate as many legless lizards as possible, a second pass should locate fewer lizards than the second pass. All search passes shall be conducted in the carly morning when legless lizards are casiest to capture. Vegetation may be removed by hand to facilitate hand raking and search efforts for legless lizards in the soil under brush. If lizards are found during the first pass, an overnight period of no soil fustrabnace must occur before the second pass, and the same requirement shall be implemented after the second pass. If no lizards are found during the second pass, a third pass is not required. Installation of a barrier, in accordance with the three pass method, shall be required if legless lizards are found at the limits of construction (project boundaries) and sufficient soft snan and vegetative over are present outspect addition	Product Water Conveyance: RUWAP Alignment (Pipeline and Booster Pump Station) and Injection Well Facilities	Prior to and during project construction	MRWPCA, qualified biologist	Prior to and during project construction	MRWPCA, qualified biologist

7

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	Mitigation Measure BT-1e: Prepare and Implement Rare Plant Restoration Plan to Mitigate Impacts to Sandmat Manzanita, Monterey Ceanothus, Monterey Spineflower, Eastwood's Goldenbush, Coast Wallflower, and Kellogg's Horkelia. Impacts to rare plant species individuals shall be avoided through project design and modification, to the extent feasible while taking into consideration other site and engineering constraints. If avoidance is not possible, the species shall be replaced at a 1:1 ratio for area of impact through preservation, restoration, or combination of both. A Rare Plant Restoration Plan, approved by the lead agency prior to commencing construction on the component site upon which the rare plant species would be impacted, shall be prepared and implemented by a qualified biologist. The plan shall include, but is not limited to, the following:  a. A detailed description of on-site and/or off-site mitigation areas, salvage of seed and/or soil bank, plant salvage, seeding and planting specifications, including, if appropriate, increased planting ratio to ensure the applicable success ratio. Specifically, seed shall be collected from the on-site individuals that would be impacted and grown in a local greenhouse, and then transplanted within the mitigation area. Plants shall be transplanted while they are young seedlings in order to develop a good root system. Alternatively, the mitigation area may be broadcast seeded in fall; however, if this method is used, some seed shall be retained in the event that the seeding fails to produce viable plants and contingency measures need to be employed.  b. A description of a 3-year monitoring program, including specific methods of vegetation monitoring, data collection and analysis, restoration goals and objectives, success criteria, adaptive management if the criteria are not met, reporting protocols, and a funding mechanism.  The mitigation area shall be preserved in perpetuity through a conservation easement or other legally enforceable land preservation agreement. E	RUWAP Pipeline Alignment, and , Injection Well Facilities,; does not apply to HMP species within the former Fort Ord.	Prior to project construction	Project engineers, project biologist, MRWPCA	For 3 years upon completion of construction	MRWPCA qualified biologist
Impact BT-1: Construction Impacts to Special-Status Species and Habitat (continued)	Mitigation Measure BT-1f: Conduct Pre-Construction Protocol-Level Botanical Surveys within the remaining portion of the Project Study Area within the Injection Well Facilities site. The project proponents shall retain a qualified biologist to conduct protocol-level surveys for special-status plant species within the Injection Well Facilities site not yet surveyed. Protocol-level surveys shall be conducted by a qualified biologist at the appropriate time of year for species with the potential to occur within the site. A report describing the results of the surveys shall be provided to the project proponents prior to any ground disturbing activities. The report shall include, but is not limited to: 1) a description of the species observed, if any; 2) map of the location, if observed; and 3) recommended avoidance and minimization measures, if applicable. The avoidance and minimization measures shall include, but are not limited to, the following:  • Impacts to State listed plant species cannot be avoided through project design and modification, to the extent feasible while taking into consideration other site and engineering constraints.  • If impacts to State listed plant species cannot be avoided, the project proponents shall comply with the CESA and consult with the CDFW to determine whether authorization for the incidental take of the species is required prior to commencing construction. If it is determined that authorization for incidental take is required from the CDFW, the project proponents shall comply with the CESA to obtain an incidental take permit prior to commencing construction on the site upon which state listed plant species could be taken. Permit requirements typically involve preparation and implementation of a mitigation plan and mitigating impacted habitat at a 3:1 ratio through preservation and/or restoration, as described below. The project proponents shall be replaced at a 1:1 ratio through preservation and/or restoration, as described below. The project proponents shall retain a qualified	Non-HMP species at the Injection Well Facilities site	Prior to project construction	MRWPCA, qualified biologist	During construction and 3 years following completion of construction	MRWPCA qualified biologist

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	collected from the on-site individuals that will be impacted and grown in a local greenhouse, and then transplanted within the mitigation area. Plants shall be transplanted while they are young seedlings in order to develop a good root system. Alternatively, the mitigation area may be broadcast seeded in fall; however, if this method is used, some seed shall be retained in the event that the seeding fails to produce viable plants and contingency measures need to be employed.  O A description of a 3-year monitoring program, including specific methods of vegetation monitoring, data collection and analysis, restoration goals and objectives, success criteria, adaptive management if the criteria are not met, reporting protocols, and a funding mechanism.					
	The mitigation area shall be preserved in perpetuity through a conservation easement or other legally enforceable land preservation agreement. Exclusionary fencing shall be installed around the mitigation area to prevent disturbance until success criteria have been met.					
Impact BT-1: Construction Impacts to Special-Status Species and Habitat (continued)	<ul> <li>Mitigation Measure BT-1g: Conduct Pre-Construction Surveys for Special-Status Bats. To avoid and reduce impacts to special-status bat species, the project proponents shall retain a qualified bat specialist or wildlife biologist to conduct site surveys during the reproductive season (May 1 through September 15) to characterize bat utilization of the component site and potential species present (techniques utilized to be determined by the biologist) prior to tree or building removal. Based on the results of these initial surveys, one or more of the following shall occur:</li> <li>If it is determined that bats are not present at the component site, no additional mitigation is required.</li> <li>If it is determined that bats are utilizing the component site and may be impacted by the Project, pre-construction surveys shall be conducted no more than 30 days prior to any tree or building removal (or any other suitable roosting habitat) within 100 feet of construction limits. If, according to the bat specialist, no bats or bat signs are observed in the course of the pre-construction surveys, tree and building removal may proceed. If bats and/or bat signs are observed during the pre-construction surveys, the biologist shall determine if disturbance would jeopardize a maternity roost or another type of roost (i.e., foraging, day, or night).</li> <li>If a single bat and/or only adult bats are roosting, removal of trees, buildings, or other suitable habitat may proceed after the bats have been safely excluded from the roost. Exclusion techniques shall be determined by the biologist and would depend on the roost type.</li> <li>If an active maternity roost is detected, avoidance is preferred. Work in the vicinity of the roost (buffer to be determined by biologist) shall be postponed until the biologist monitoring the roost determines that the young have fledged and are no longer dependent on the roost. The monitor shall ensure that all bats have left the area of disturbance prior to initiation of pruning and/or rem</li></ul>	Salinas Pump Station, Salinas Treatment Facility, Blanco Drain Diversion, Product Water Conveyance: RUWAP Alignment and Injection Well Facilities	Prior to project construction	MRWPCA, qualified biologist (bat/wildlife specialist)	Prior to project construction	MRWPCA and qualified biologist
	Mitigation Measure BT-1h: Implementation of Mitigation Measures BT-1a and BT-1b to Mitigate Impacts to the Monterey Ornate Shrew, Coast Horned Lizard, Coast Range Newt, Two-Striped Garter Snake, and Salinas Harvest Mouse. If these species are encountered, implementation of Mitigation Measures BT-1a and BT-1b, which avoid and minimize impacts through implementing construction best management practices and monitoring, would reduce potential impacts to these species to a less-than-significant level.	Blanco Drain Diversion, Product Water Conveyance: RUWAP Alignment and Injection Well Facilities	Prior to and during project construction	MRWPCA contractors and qualified biologists	Prior to and during project construction	MRWPCA qualified biologist
	Mitigation Measure BT-1i: Conduct Pre-Construction Surveys for Monterey Dusky- Footed Woodrat. To avoid and reduce impacts to the Monterey dusky-footed woodrat, the project proponents shall retain a qualified biologist to conduct pre-construction surveys in suitable habitat proposed for construction, ground disturbance, or staging within three days prior to construction for woodrat nests within the project area and in a buffer zone 100 feet out from the limit of disturbance. All woodrat nests shall be flagged for avoidance of direct construction impacts and protection during construction, where feasible. Nests that cannot be avoided shall be manually deconstructed prior to land clearing activities to allow animals to escape harm. If a litter of young is found or suspected, nest material shall be replaced, and the nest left alone for 2-3 weeks before a re-check to verify that young are capable of independent survival before proceeding with nest dismantling.	Blanco Drain Diversion, Product Water Conveyance: RUWAP Pipeline Alignment, and Injection Well Facilities	Prior to project construction	MRWPCA contractors and qualified biologists	Prior to project construction	MRWPCA qualified biologist

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	<ul> <li>Mitigation Measure BT-1j: Conduct Pre-Construction Surveys for American Badger. To avoid and reduce impacts to the American badger, the project proponents shall retain a qualified biologist to conduct focused pre-construction surveys for badger dens in all suitable habitat proposed for construction, ground disturbance, or staging no more than two weeks prior to construction. If no potential badger dens are present, no further mitigation is required. If potential dens are observed, the following measures are required to avoid potential significant impacts to the American badger:</li> <li>If the qualified biologist determines that potential dens are inactive, the biologist shall excavate these dens by hand with a shovel to prevent badgers from reusing them during construction.</li> <li>If the qualified biologist determines that potential dens may be active, the den shall be monitored for a period sufficient (as determined by a qualified biologist) to determine if the den is a maternity den occupied by a female and her young, or if the den is occupied by a solitary badger.</li> <li>Maternity dens occupied by a female and her young shall be avoided during construction and a minimum buffer of 200 feet in which no construction activities shall occur shall be maintained around the den. After the qualified biologist determines that badgers have stopped using active dens within the project boundary, the dens shall be hand-excavated with a shovel to prevent re-use during construction.</li> <li>Solitary male or female badgers shall be passively relocated by blocking the entrances of the dens with soil, sticks, and debris for three to five days to discourage the use of these dens prior to project construction disturbance. The den entrances shall be blocked to an incrementally greater degree over the three to five day period. After the qualified biologist determines that badgers have stopped using active dens within the project boundary, the dens shall be hand-excavated with a shovel to prevent re-use during constr</li></ul>	Product Water Conveyance: RUWAP Pipeline Alignment	Prior to project construction	MRWPCA construction contractors and qualified biologists	Prior to project construction	MRWPCA qualified biologist
Impact BT-1: Construction Impacts to Special-Status Species and Habitat (continued)	Mitigation Measure BT-1k: Conduct Pre-Construction Surveys for Protected Avian Species, including, but not limited to, white-tailed kite and California horned lark. Prior to the start of construction activities at each project component site, a qualified biologist shall conduct pre-construction surveys for suitable nesting habitat within the component Project Study Area and within a suitable buffer area from the component Project Study Area. The qualified biologist shall determine the suitable buffer area based on the avian species with the potential to nest at the site.  In areas where nesting habitat is present within the component project area or within the determined suitable buffer area, construction activities that may directly (e.g., vegetation removal) or indirectly (e.g., noise/ground disturbance) affect protected nesting avian species shall be timed to avoid the breeding and nesting season. Specifically, vegetation and/or tree removal can be scheduled after September 16 and before January 31. Alternatively, a qualified biologist shall be retained by the project proponents to conduct pre-construction surveys for nesting raptors and other protected avian species where nesting habitat was identified and within the suitable buffer area if construction commences between February 1 and September 15. Pre-construction surveys shall be conducted no more than 14 days prior to the start of construction activities during the early part of the breeding season (February through April) and no more than 30 days prior to the initiation of these activities during the late part of the breeding season (May through August). Because some bird species nest early in spring and others nest later in summer, surveys for nesting birds may be required to continue during construction to address new arrivals, and because some species breed multiple times in a season. The necessity and timing of these continued surveys shall be determined by the qualified biologist based on review of the final construction plans.  If active raptor o	All components	Prior to project construction and if found establish and comply with no-disturbance buffer	MRWPCA, CalAm, construction contractors, and qualified biologists	Prior to project construction	MRWPCA, CalAm, qualified biologist(s), USFWS
	Mitigation Measure BT-1l: Conduct Pre-Construction Surveys for Burrowing Owl. In order to avoid impacts to active burrowing owl nests, a qualified biologist shall conduct pre-construction surveys in suitable habitat within the construction footprint and within a suitable buffer, as determined by a qualified biologist, of the footprint no more than 30 days prior to the start of construction at a component site. If ground disturbing activities are delayed or suspended for more than 30 days after the pre-construction survey, the site shall be resurveyed. The survey shall conform to the DFG 1995 Staff Report protocol. If no burrowing owls are found, no further mitigation is required. If it is	Product Water Conveyance: RUWAP Pipeline Alignment	Prior to project construction	Construction contractor, MRWPCA, qualified biologist	Prior to project construction	MRWPCA qualified biologist

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	determined that burrowing owls occupy the site during the non-breeding season (September 1 through January 31), then a passive relocation effort (e.g., blocking burrows with one-way doors and leaving them in place for a minimum of three days) shall be undertaken to ensure that the owls are not harmed or injured during construction. Once it has been determined that the owls have vacated the site, the burrows shall be collapsed, and ground disturbance can proceed. If burrowing owls are detected within the construction footprint or immediately adjacent lands (i.e. within 250 feet of the footprint) during the breeding season (February 1 to August 31), a construction-free buffer of 250 feet shall be established around all active owl nests. The buffer area shall be enclosed with temporary fencing, and construction equipment and workers shall not enter the enclosed setback areas. Buffers shall remain in place for the duration of the breeding season or until it has been confirmed by a qualified biologist that all chicks have fledged and are independent of their parents. After the breeding season, passive relocation of any remaining owls shall take place as described above.					
	Mitigation Measure BT-1m: Minimize Effects of Nighttime Construction Lighting. Nighttime construction lighting shall be focused and downward directed to preclude night illumination of the adjacent open space area.	Injection Well Facilities and CalAm Distribution System: Alternative Monterey Pipeline	During project construction	MRWPCA and CalAm construction contractors	During project construction	MRWPCA, CalAm, City of Seaside, City of Monterey
	Mitigation Measure BT-1p: Avoid and Minimize Impacts to Western Pond Turtle. A qualified biologist shall survey suitable habitat no more than 48 hours before the onset of work activities at the component site for the presence of western pond turtle. If pond turtles are found and these individuals are likely to be killed or injured by work activities, the biologist shall be allowed sufficient time to move them from the site before work activities begin. The biologist shall relocate the pond turtles the shortest distance possible to a location that contains suitable habitat and would not be affected by activities associated with the project.	Blanco Drain Diversion	Prior to project construction	MRWPCA construction contractor and qualified biologist	Prior to project construction	MRWPCA qualified biologist
	Mitigation Measure BT-1q: Avoid and Minimize Impacts to California Red-Legged Frog. The following measures for avoidance and minimization of adverse impacts to California Red-Legged Frog (CRLF) during construction of the Project components are those typically employed for construction activities that may result in short-term impacts to individuals and their habitat. The focus of these measures is on scheduling activities at certain times of year, keeping the disturbance footprint to a minimum, and monitoring.					
Impact BT-1: Construction Impacts to	• The MRWPCA shall annually submit the name(s) and credentials of biologists who would conduct activities specified in the following measures. No project construction activities at the component site would begin until the MRWPCA receives confirmation from the USFWS that the biologist(s) is qualified to conduct the work.					
Special-Status Species and Habitat (continued)	• A USFWS-approved biologist shall survey the work site 48 hours prior to the onset of construction activities. If CRLF, tadpoles, or eggs are found, the approved biologist shall determine the closest appropriate relocation site. The approved biologist shall be allowed sufficient time to move the CRLF, tadpoles or eggs from the work site before work activities begin. Only USFWS-approved biologists shall participate in activities associated with the capture, handling, and moving of CRLF.			MRWPCA	Prior to and	N TOWN TO A
	• Before any construction activities begin on the project component site, a USFWS-approved biologist shall conduct a training session for all construction personnel. At a minimum, the training shall include a description of the CRLF and its habitat, the importance of the CRLF and its habitat, general measures that are being implemented to conserve the CRLF as they relate to the project, and the boundaries within which the project construction activities may be accomplished. Brochures, books and briefings may be used in the training session, provided that a qualified person is on hand to answer any questions.	Salinas Treatment Facility and Blanco Drain Diversion	Prior to and during project construction	construction contractor and qualified biologist	during project construction	MRWPCA, qualified biologist, USFWS
	• A USFWS-approved biologist shall be present at the work site until such time as all removal of CRLF, instruction of workers, and disturbance of habitat have been completed. After this time, the biologist shall designate a person to monitor onsite compliance with all minimization measures and any future staff training. The USFWS-approved biologist shall ensure that this individual receives training outlined in Mitigation Measure Bt-1a and in the identification of CRLF. The monitor and the USFWS-approved biologist shall have the authority to stop work if CRLF are in harm's way.					
	• The number of access routes, number and size of staging areas, and the total area of the activity shall be limited to the minimum necessary to achieve the project goal. Routes and boundaries shall be clearly demarcated, and these areas shall be outside of riparian and wetland areas to the extent practicable.					

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	<ul> <li>Work activities shall be completed between April 1 and November 1, to the extent practicable. Should the project proponent demonstrate a need to conduct activities outside this period, the project proponent may conduct such activities after obtaining USFWS approval (applies to Blanco Drain site only).</li> <li>If a work site is to be temporarily dewatered by pumping, intakes shall be completely screened with wire mesh not larger than five millimeters (mm) to prevent CRLF from entering the pump system. Water shall be released or pumped downstream at an appropriate rate to maintain downstream flows during construction. Upon completion of construction activities, any barriers to flow shall be removed in a manner that would allow flow to resume with the least disturbance to the substrate.</li> <li>The Declining Amphibian Populations Task Force's Fieldwork Code of Practice shall be followed to minimize the possible spread of chytrid fungus or other amphibian pathogens and parasites.</li> </ul>					
Impact BT-2: Construction Impacts to Sensitive Habitats	Mitigation Measure BT-2a: Avoidance and Minimization of Impacts to Riparian Habitat and Wetland Habitats. Implement Mitigation Measure BT-1a. When designing the facilities at these component sites, the MRWPCA shall site and design project features to avoid impacts to the riparian and wetland habitats shown in Attachment 8 of Appendix H and Appendix I, including direct habitat removal and indirect hydrology and water quality impacts, to the greatest extent feasible while taking into account site and engineering constraints. To protect this sensitive habitat during construction, the following measures shall be implemented:  Place construction fencing around riparian and wetland habitat (i.e., areas adjacent to or nearby the Project construction) to be preserved to ensure construction activities and personnel do not impact this area.  All proposed lighting shall be designed to avoid light and glare into the riparian and wetland habitat. Light sources shall not illuminate these areas or cause glare.  In the event that full avoidance is not possible and a portion or all of the riparian and wetland habitat would be impacted, the following minimization measures shall be implemented:  Permanently impacted riparian and wetland habitat shall be mitigated at no less than a 2:1 replacement-to-loss ratio through restoration and/or preservation. The final mitigation amounts for both temporary and permanent impacts to riparian and wetland habitat shall be determined during the design phase but cannot be less than 2:1 for permanent impacts and 1:1 for temporary impacts, and must be approved by the relevant permitting agencies (USACOE, RWQCB, CDFW, and the entity issuing any Coastal Development Permit). The preserved mitigation land shall be managed to improve welland and riparian conditions compared to existing conditions. It is expected that the mitigation can occur within the Locke Paddon Lake watershed, along the Tembladero Slough, and within the Salinas River corridor near the Blanco Drain near where impacts may occur. A H	Reclamation Ditch, Tembladero Slough Diversion, Blanco Drain Diversion	Prior to and during project construction	MRWPCA construction contractor and qualified biologist	Prior to and during project construction	MRWPCA qualified biologist

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	Mitigation Measure BT-2c: The project proponents in coordination with the contractor shall prepare and implement a Frac-Out Plan to avoid or reduce accidental impacts resulting from horizontal directional drilling (HDD) beneath the Salinas River. The Frac-Out Plan shall address spill prevention, containment, and clean-up methodology in the event of a frac out. The proposed HDD component of the Blanco Drain diversion shall be designed and conducted to minimize the risk of spills and frac-out events. The Frac-Out Plan shall be prepared and submitted to United States Fish and Wildlife Services, California Department of Fish and Wildlife, National Marine Fisheries Services, and the Regional Water Quality Control Board prior to commencement of HDD activities for the Blanco Drain Diversion construction. The following are typical contents of a Frac-Out Plan:	Blanco Drain Diversion				
Impact BT-2:	<ul> <li>Project description, including details of the HDD design and operations</li> <li>Site description and existing conditions</li> </ul>		Prior to project construction	MRWPCA, construction contractors		
Construction Impacts to Sensitive Habitats (continued)	<ul> <li>Potential modes of HDD failure and HDD failure prevention and mitigation</li> </ul>				Prior to and during project construction	MRWPCA, USFWS, CDFW, NOAA/NMFS, RWQCB
	• Frac-out prevention measures (including for example, geotechnical investigations, planning for appropriate depths based on those investigations, presence of a qualified engineer during drilling to monitor the drilling process, live adjustments to the pace of drill advancement to ensure sufficient time for cutting and fluid circulation and to prevent or minimize plugging, maintaining the minimum drilling pressure necessary to maintain fluid circulation, etc.)					
	Monitoring requirements (for example, monitoring pump pressure circulation rate, ground surface and surface water inspection, advancing the drill only during daytime hours, on-site biological resource monitoring by a qualified biologist)					
	<ul> <li>Response to accidental frac-out (including stopping drilling, permitting agency notification, surveying the area, containing the frac-out material, contacting the project biological monitor to identify and relocate species potentially in the area, turbidity monitoring, procedures for clean-up and mitigation of hazardous waste spill materials, preparation of documentation of the event, etc.)</li> <li>Coordination plan and contact list of key project proponents, biological monitor, and agency staff in the event of an accidental frac-out event.</li> </ul>					
Impact BT-4: Construction Conflicts with Local Policies, Ordinances, or Approved Habitat Conservation Plan	Mitigation Measure BT-4. HMP Plant Species Salvage. For impacts to the HMP plant species within the Project Study Area that do not require take authorization from USFWS or CDFW, salvage efforts for these species shall be evaluated by a qualified biologist per the requirements of the HMP and BO. A salvage plan shall be prepared and implemented by a qualified biologist, which shall would include, but is not limited to: a description and evaluation of salvage opportunities and constraints; a description of the appropriate methods and protocols of salvage and relocation efforts; identification of relocation and restoration areas; and identification of qualified biologists approved to perform the salvage efforts, including the identification of any required collection permits from USFWS and/or CDFW. Where proposed, seed collection shall occur from plants within the Project Study Area and topsoil shall be salvaged within occupied areas to be disturbed. Seeds shall be collected during the appropriate time of year for each species by qualified biologists. At the time of seed collection, a map shall also be prepared that identifies the specific locations of the plants for any future topsoil preservation efforts. The collected seeds shall be used to revegetate temporarily disturbed construction areas and reseeding and restoration efforts on- or off-site, as determined appropriate in the salvage plan.	Product Water Conveyance: RUWAP Pipeline Alignment, and Injection Well Facilities site within the former Fort Ord only	Prior to, during, and after construction	MRWPCA Biologist	During, and after construction	MRWPCA qualified biologist
Impact CR-1: Construction Impacts on Historic Resources	Mitigation Measure CR-1: Avoidance and Vibration Monitoring for Pipeline Installation in the Presidio of Monterey Historic District, and Downtown Monterey. Avoidance and Vibration Monitoring for Pipeline Installation in the Presidio of Monterey Historic District, and Downtown Monterey. (Applies to portion of the CalAm Distribution System: Alternative Monterey Pipeline) CalAm shall construct the section of the Alternative Monterey Pipeline located on Stillwell Avenue within the Presidio of Monterey Historic District, adjacent to the Spanish Royal Presidio, and within the Monterey Old Town National Historic Landmark District (including adjacent to Stokes Adobe, the Gabriel de la Torre Adobe, the Fremont Adobe, Colton Hall, and Friendly Plaza in downtown Monterey) <sup>2</sup> as close as possible to the centerlines of these streets to: (1) avoid direct impacts to the historic Presidio Entrance Monument, and (2) reduce impacts from construction vibration to below the 0.12 inches per second (in/sec) peak particle velocity vibration PPV) threshold. If CalAm determines that the pipeline	Portion of the CalAm Distribution System- Alternative Monterey Pipeline within historic districts and adjacent to historic buildings	During project construction	CalAm, project engineers, construction contractors	During project construction	CalAm and City of Monterey

<sup>&</sup>lt;sup>2</sup> A modification to this mitigation measure has been made to clarify its applicability to the Staff-Recommendation Alternative of the GWR Project. Specifically, the text highlighted in gray has been added and the following text deleted: "and within W. Franklin Street in downtown Monterey." This change to the mitigation measure does not constitute significant new information; it merely clarifies the mitigation for the selected alternative.

Pure Water Monterey GWR Project – Staff Recommended Alternative

Mitigation Monitoring and Reporting Program

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	cannot be located near the centerline of these street segments due to traffic concerns or existing utilities, the historic properties identified on Table 4.6-2 of the GWR Project Draft EIR (MRWPCA/DD&A, April 2015) shall be monitored for vibration during pipeline construction, especially during the use of jackhammers and vibratory rollers. If construction vibration levels exceed 0.12 in/sec PPV, construction shall be halted and other construction methods shall be employed to reduce the vibration levels below the standard threshold. Alternative construction methods may include using concrete saws instead of jackhammers or hoe-rams to open excavation trenches, the use of non-vibratory rollers, and hand excavation. If impact sheet pile installation is needed (i.e., for horizontal directional drilling or jack-and-bore) within 80 feet of any historical resource or within 80 feet of a historic district, CalAm shall monitor vibration levels to ensure that the 0.12-in/sec PPV damage threshold is not exceeded. If vibration levels exceed the applicable threshold, the contractor shall use alternative construction methods such as vibratory pile drivers.					
Impact CR-2: Construction Impacts on Archaeological Resources or Human Remains	Mitigation Measure CR-2a: Archaeological Monitoring Plan. Each of the project proponents shall contract a qualified archaeologist meeting the Secretary of the Interior's Qualification Standard (Lead Archaeologist) to prepare and implement an Archaeological Monitoring Plan, and oversee and direct all archaeological monitoring activities during construction. Archaeological monitoring shall be conducted for all subsurface excavation work within 100 feet of Presidio #2 in the Presidio of Monterey, and within the areas of known archaeologically sensitive sites in Monterey. At a minimum, the Archaeological Monitoring Plan shall:  Detail the cultural resources training program that shall be completed by all construction and field workers involved in ground disturbance;  Designate the person(s) responsible for conducting monitoring activities, including Native American monitor(s), if deemed necessary;  Establish monitoring protocols to ensure monitoring is conducted in accordance with current professional standards provided by the California Office of Historic Preservation;  Establish the template and content requirements for monitoring reports;  Establish a schedule for submittal of monitoring reports and person(s) responsible for review and approval of monitoring reports;  Establish protocols for notifications in case of encountering cultural resources, as well as methods for evaluating significance, developing and implementing a plan to avoid or mitigate significant resource impacts, facilitating Native American participation and consultation, implementing a collection and curation plan, and ensuring consistency with applicable laws including Section 7705.5 of the California Health and Safety Code and Section 5907.99 of the Public Resources Code;  Establish methods to ensure security of cultural resources sites;  Describe the appropriate protocols for notifying the County, Native Americans, and local authorities (i.e. Sheriff, Police) should site looting and other illegal activities occur during construction with re	Lake El Estero Diversion Site and CalAm Distribution System: Alternative Monterey Pipeline	Prior to and during project construction	MRWPCA (for Lake El Estero Diversion only), CalAm, qualified archaeologist	During project construction	MRWPCA, CalAm, qualified archaeologist

<sup>&</sup>lt;sup>3</sup> A modification to this mitigation measure has been made to clarify its applicability to the Staff-Recommendation Alternative of the GWR Project. Specifically, the text highlighted in gray has been added and the following text deleted: "in downtown Monterey on W. Franklin Street between High and Figuero Streets, and at potentially sensitive archaeological sites at Lake El Estero"

Pure Water Monterey GWR Project – Staff Recommended Alternative

October 2015

Mitigation Monitoring and Reporting Program

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	within the portions of the archaeological resources located within the project Area of Potential Effects; would preserve any significant historical information obtained; and will identify the scientific/historic research questions applicable to the resources, the data classes the resource is expected to possess, and how the expected data classes would address the applicable research questions. The results of the investigation shall be documented in a technical report that provides a full artifact catalog, analysis of items collected, results of any special studies conducted, and interpretations of the resource within a regional and local context. All technical documents shall be placed on file at the Northwest Information Center of the California Historical Resources Information System.					
	Mitigation Measure CR-2b: Discovery of Archaeological Resources or Human Remains. If archaeological resources or human remains are unexpectedly discovered during any construction, work shall be halted within 50 meters (±160 feet) of the find until it can be evaluated by a qualified professional archaeologist. If the find is determined to be significant, appropriate mitigation measures shall be formulated and implemented. The County Coroner shall be notified in accordance with provisions of Public Resources Code 5097.98-99 in the event human remains are found and the Native American Heritage Commission shall be notified in accordance with the provisions of Public Resources Code section 5097 if the remains are determined to be of Native American origin.	All components	During project construction	MRWPCA, CalAm, and qualified archaeologists	During project construction	MRWPCA, CalAm, and qualified archaeologist
	Mitigation Measure CR-2c: Native American Notification. Because of their continuing interest in potential discoveries during construction, all listed Native American Contacts shall be notified of any and all discoveries of archaeological resources in the project area.	All components	During project construction	MRWCPA, CalAm and qualified archaeologist	During project construction	MRWCPA, CalAm and qualified archaeologist
Impact EN-1: Construction Impacts due to Temporary Energy Use	Mitigation Measure EN-1: Construction Equipment Efficiency Plan. MRWPCA (for all components except the CalAm Distribution System) or CalAm (for the Cal Am Distribution System) shall contract a qualified professional (i.e., construction planner/energy efficiency expert) to prepare a Construction Equipment Efficiency Plan that identifies the specific measures that MRWPCA or CalAm (and its construction contractors) will implement as part of project construction to increase the efficient use of construction equipment. Such measures shall include, but not necessarily be limited to: procedures to ensure that all construction equipment is properly tuned and maintained at all times; a commitment to utilize existing electricity sources where feasible rather than portable diesel-powered generators; consistent compliance with idling restrictions of the state; and identification of procedures (including the use of routing plans for haul trips) that will be followed to ensure that all materials and debris hauling is conducted in a fuel-efficient manner.	All components	Prior to project construction	MRWPCA, CalAm. energy efficiency expert, construction contractors	During project construction	MRWPCA and CalAm
Impact HH-2: Accidental Release of Hazardous	Mitigation Measure HH-2a: Environmental Site Assessment. If required by local jurisdictions and property owners with approval responsibility for construction of each component, MRWPCA and CalAm shall conduct a Phase I Environmental Site Assessment in conformance with ASTM Standard 1527-05 to identify potential locations where hazardous material contamination may be encountered. If an Environmental Site Assessment indicates that a release of hazardous materials could have affected soil or groundwater quality at a project site, a Phase II environmental site assessment shall be conducted to determine the extent of contamination and to prescribe an appropriate course of remediation, including but not limited to removal of contaminated soils, in conformance with state and local guidelines and regulations. If the results of the subsurface investigation(s) indicate the presence of hazardous materials, additional site remediation may be required by the applicable state or local regulatory agencies, and the contractors shall be required to comply with all regulatory requirements for facility design or site remediation.	Lake El Estero Diversion, Product Water Conveyance RUWAP Pipeline Alignment, Injection Well Facilities and the CalAm Distribution System: Alternative Monterey Pipeline	Prior to project construction (if presence of hazardous materials is identified, site remediation or design changes may be required)	MRWPCA and CalAm project engineers, construction contractors	Only needed until owner/contra ctor deems each construction site is deemed safe for required construction	MRWPCA and CalAm
Materials During Construction	<ul> <li>Mitigation Measure HH-2b: Health and Safety Plan. The construction contractor(s) shall prepare and implement a project-specific Health and Safety Plan (HSP) for each site on which construction may occur, in accordance with 29 CFR 1910 to protect construction workers and the public during all excavation, grading, and construction. The HSP shall include the following, at a minimum:</li> <li>A summary of all potential risks to construction workers and the maximum exposure limits for all known and reasonably foreseeable site chemicals (the HSP shall incorporate and consider the information in all available existing Environmental Site Assessments and remediation reports for properties within ¼-mile using the EnviroStor Database);</li> <li>Specified personal protective equipment and decontamination procedures, if needed;</li> </ul>	Lake El Estero Diversion, Product Water Conveyance RUWAP Pipeline Alignment, the Injection Well Facilities, and the CalAm Distribution System:	Prior to project construction	Construction contactors	During project construction	MRWPCA, CalAm, Monterey County Dept. of Environmental Health

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	• Emergency procedures, including route to the nearest hospital;  Procedures to be followed in the event that evidence of potential soil or groundwater contamination (such as soil staining, noxious odors, debris or buried storage containers) is encountered. These procedures shall be in accordance with hazardous waste operations regulations and specifically include, but are not limited to, the following: immediately stopping work in the vicinity of the unknown hazardous materials release, notifying Monterey County Department of Environmental Health, and retaining a qualified environmental firm to perform sampling and remediation; and  The identification and responsibilities of a site health and safety supervisor.	Alternative Monterey Pipeline				
	Mitigation Measure HH-2c: Materials and Dewatering Disposal Plan. MRWPCA and CalAm and/or their contractors shall develop a materials disposal plan specifying how the contractor will remove, handle, transport, and dispose of all excavated material in a safe, appropriate, and lawful manner. The plan must identify the disposal method for soil and the approved disposal site, and include written documentation that the disposal site will accept the waste. For areas within the Seaside munitions response areas called Site 39 (coincident with the Injection Well Facilities component), the materials disposal plans shall be reviewed and approved by FORA and the City of Seaside. The contractor shall develop a groundwater dewatering control and disposal plan specifying how the contractor will remove, handle, and dispose of groundwater impacted by hazardous substances in a safe, appropriate, and lawful manner. The plan must identify the locations at which potential contaminated groundwater dewatering are likely to be encountered (if any), the method to analyze groundwater for hazardous materials, and the appropriate treatment and/or disposal methods. If the dewatering effluent contains contaminants that exceed the requirements of the General WDRs for Discharges with a Low Threat to Water Quality (Order No. R3-2011-0223, NPDES Permit No. CAG993001), the construction contractor shall contain the dewatering effluent in a portable holding tank for appropriate offsite disposal or discharge. The contractor can either dispose of the contaminated effluent at a permitted waste management facility or discharge the effluent, under permit, to the Regional Treatment Plant.	Lake El Estero Diversion, Product Water Conveyance: RUWAP Pipeline Alignment, the Injection Well Facilities, and the CalAm Distribution System: Alternative Monterey Pipeline	Prior to and during project construction	MRWPCA, CalAm, construction contractors	During project construction	MRWPCA and CalAm; FORA and the City of Seaside for areas within Site 39
Impact HS-4: Operational Surface Water Quality Impacts due to Source Water Diversions	Mitigation Measure HS-4: Management of Surface Water Diversion Operations. Rapid, imposed water-level fluctuations shall be avoided when operating the Reclamation Ditch Diversion pumps to minimize erosion and failure of exposed (or unvegetated), susceptible banks. This can be accomplished by operating the pumps at an appropriate flow rate, in conjunction with commencing operation of the pumps only when suitable water levels or flow rates are measured in the water body. Proper control shall be implemented to ensure that mobilized sediment would not impair downstream habitat values and to prevent adverse impacts due to water/soil interface adjacent to the Reclamation Ditch and Tembladero Slough. During planned routine maintenance at the Reclamation Ditch Diversion, maintenance personnel shall inspect the diversion structures within the channel for evidence of any adverse fluvial geomorphological processes (for example, undercutting, erosion, scour, or changes in channel cross-section). If evidence of any substantial adverse changes is noted, the diversion structure shall be redesigned and the project proponents shall modify it in accordance with the new design.	Reclamation Ditch Diversion	During project operations	MRWPCA	During project operations	MRWPCA
Cumulative impacts to marine water quality	Mitigation Measure HS-C: Implement Measures to Avoid Exceedances over Water Quality Objectives at the Edge of the Zone of Initial Dilution (ZID). As part of the amendment process to modify the existing MRWPCA NPDES Permit (Order No. R3-2014-0013, NPDES Permit No. CA0048551) per 40 Code of Regulations Part 122.62, it would be necessary to conduct an extensive assessment in accordance with requirements to be specified by the RWQCB. It is expected that the assessment would include, at a minimum, an evaluation of the minimum probable initial dilution at the point of discharge based on likely discharge scenarios and any concomitant impacts on water quality and beneficial uses per the Ocean Plan. Prior to operation of the MPSWP desalination plant, the discharger(s) will be required to test the MPSWP source water in accordance with protocols approved by the RWQCB. If the water quality assessment indicates that the water at the edge of the ZID will exceed the Ocean Plan water quality objectives, the MRWPCA will not accept the desalination brine discharge at its outfall, and the following design features and/or operational measures shall be employed, individually or in combination, to reduce the concentration of constituents to below the Ocean Plan water quality objectives at the edge of the ZID:	Ocean discharges upon implementation of cumulative project (specifically, the MPWSP with 6.4 mgd desalination plant)	Prior to operation of the MPWSP (with 6.4 mgd desalination plant)	MRWPCA	During operations of the MPWSP with 6.4 mgd desalination plant	MRWPCA (under regulations by the RWQCB)
	Additional pre-treatment of MPWSP source water at the Desalination Plant: Feasible methods to remove PCBs and other organic compounds from the MPWSP source water at the desalination plant include additional filtration or use of granular activated carbon (GAC). GAC acts as a very strong sorbent and can effectively remove PCBs and other organic compounds from the desalination plant source water (Luthy, Richard G., 2015).  **Reproject - Staff Recommended Alternative**  16					October 2015

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	• Treatment of discharge at the Desalination Plant: Feasible methods to remove residual compounds from the discharge to comply with water quality objectives at the edge of the ZID are use of GAC (similar to that under the additional pre-treatment of MPWSP source water) and advanced oxidation with ultraviolet light with concurrent addition of hydrogen peroxide. The method of using advanced oxidation with ultraviolet light with concurrent addition of hydrogen peroxide is used for the destruction of a variety of environmental contaminants such as synthetic organic compounds, volatile organic compounds, pesticides, pharmaceuticals and personal care products, and disinfection byproducts. This process is energy intensive, but requires a relatively small construction footprint.					
	• Short-term storage and release of brine at the Desalination Plant: When sufficient quantities of treated wastewater from the Regional Treatment Plant to prevent an exceedance of Ocean Plan objectives at the edge of the ZID are not available, brine from the desalination plant would be temporarily stored at the MPWSP site in the brine storage basin,23 and discharged (pumped) in pulse flows (up to the capacity of the existing outfall), such that the flow rate allows the discharge to achieve a dilution level that meets Ocean Plan water quality objectives at the edge of the ZID.					
	• Biologically Active Filtration at the Regional Treatment Plant: As part of the proposed AWT Facility at the Regional Treatment Plant, the GWR Project includes the potential for use of upflow biologically active filtration following ozone treatment to reduce the concentration of ammonia and residual organic matter present in the ozone effluent and to reduce the solids loading on the membrane filtration process. The biologically active filtration system would consist of gravity-feed filter basins with approximately 12 feet of granular media, and a media support system. Ancillary systems would include an alkalinity addition system for pH control, backwash waste water basin (also used for membrane filtration backwash waste water), backwash pumps, an air compressor and supply system for air scour, an air compressor and supply system for process air, and a wash water basin to facilitate filter backwashing (the wash water basin may be combined with the membrane filtration flow equalization basin). This biologically active filtration system may be needed to meet Ocean Plan water quality objectives at the edge of the ZID (if and/or when discharges from the Project are combined with discharges from the MPWSP with 6.4 million gallon per day, or mgd, desalination plant). This optional component of the Project is described in Chapter 2, Project Description (see Section 2.8.1.3), would become a required process if the MPWSP with 6.4 mgd desalination project is in operation and the other components of the mitigation do not achieve Ocean Plan compliance.					
Impact LU-1: Temporary Farmland Conversion during Construction	<ul> <li>Mitigation Measure LU-1: Minimize Disturbance to Farmland. To support the continued productivity of designated Prime Farmland and Farmland of Statewide Importance, the following provisions shall be included in construction contract specifications:</li> <li>Construction contractor(s) shall minimize the extent of the construction disturbance, including construction access and staging areas, in designated important farmland areas.</li> <li>Prior to the start of construction, the construction contractor(s) shall mark the limits of the construction area and ensure that no construction activities, parking, or staging occur beyond the construction limits.</li> <li>Upon completion of the active construction, the site shall be restored to pre-construction conditions.</li> </ul>	Salinas Treatment Facility and a portion of the Blanco Drain Diversion	During project construction	Construction contractor	During project construction	MRWPCA
Impact LU-2: Operational Consistency with Plans, Policies, and Regulations	See the following mitigation measures: AQ-1, BF-1a, BF-1b, BF-1c, BF-2a or Alternate BF-2a, BT-1a through BT-1q, BT-2a through BT-2c, CR-2a through CR-2c, EN-1, NV-1a through NV-1d, NV-2a, NV-2b, PS-3, TR-2, TR-3, and TR-4.	All components	See other rows for specific timing of each mitigation measure	See other lines for responsibilities for each mitigation measure	See other rows for specific timing of each mitigation measure	See other rows for responsibilities for each mitigation measure
Cumulative impacts to marine	Mitigation Measure MR-C. Implement Measures to Avoid Exceedances over Water Quality Objectives at the Edge of the Zone of Initial Dilution. Implement Mitigation Measure HS-C above.	Ocean discharges upon implementation of cumulative project	Prior to operation of MPWSP (with	MRWPCA	During operations of the MPWSP	MRWPCA (under regulations by the RWQCB)

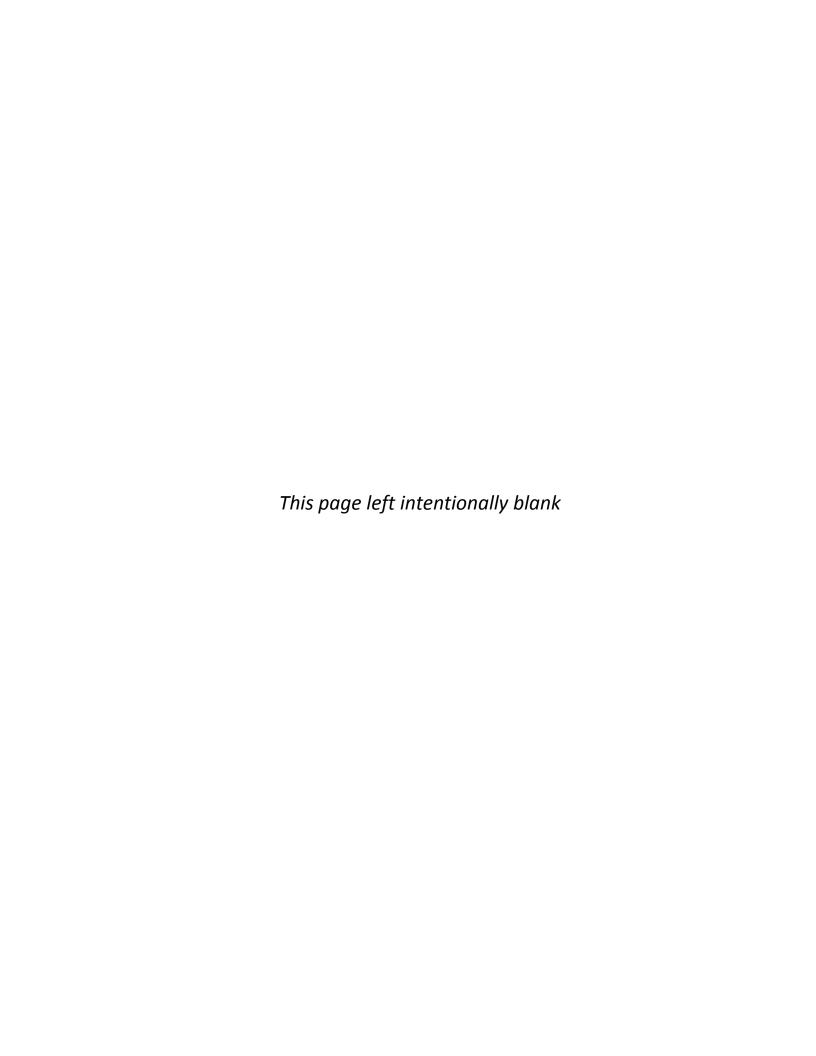
Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
biological resources		(specifically, the MPWSP with 6.4 mgd desalination plant)	6.4 mgd desalination plant)		with 6.4 mgd desalination plant	
Impact NV-1: Construction Noise	Mitigation Measure NV-1a: Drilling Contractor Noise Measures. Contractor specifications shall include a requirement that drill rigs located within 700 feet of noise-sensitive receptors shall be equipped with noise reducing engine housings or other noise reducing technology and the line of sight between the drill rig and nearby sensitive receptors shall be blocked by portable acoustic barriers and/or shields to reduce noise levels such that drill rig noise levels are no more 75 dBA (or, A-Weighted Sound Level) at 50 feet. This would reduce the nighttime noise level to less than 60 dBA Leq (Equivalent Noise Level) at the nearest residence. The contractor shall submit to the MRWPCA and the Seaside Building Official, a "Well Construction Noise Control Plan" for review and approval. The plan shall identify all feasible noise control procedures that would be implemented during night-time construction activities. At a minimum, the plan shall specify the noise control treatments to achieve the specified above noise performance standard.	Injection Well Facilities	Prior to and during project construction	Construction contractors	During project construction	MWRPCA, Seaside building official
	Mitigation Measure NV-1b: Monterey Pipeline Noise Control Plan for Nighttime Pipeline Construction. CalAm shall submit a Noise Control Plan for all nighttime pipeline work to the California Public Utilities Commission for review and approval prior to the commencement of project construction activities. The Noise Control Plan shall identify all feasible noise control procedures to be implemented during nighttime pipeline installation in order to reduce noise levels to the extent practicable at the nearest residential or noise sensitive receptor. At a minimum, the Noise Control Plan shall require use of moveable noise screens, noise blankets, or other suitable sound attenuation devices be used to reduce noise levels during nighttime pipeline installation activities.	CalAm Distribution System: Alternative Monterey Pipeline	Prior to project construction	CalAm	During project construction	CalAm, CPUC and City of Monterey
	Mitigation Measure NV-1c: Neighborhood Notice. Residences and other sensitive receptors within 900 feet of a nighttime construction area shall be notified of the construction location and schedule in writing, at least two weeks prior to the commencement of construction activities. The notice shall also be posted along the proposed pipeline alignments, near the proposed facility sites, and at nearby recreational facilities. The contractor shall designate a noise disturbance coordinator who would be responsible for responding to complaints regarding construction noise. The coordinator shall determine the cause of the complaint and ensure that reasonable measures are implemented to correct the problem. A contact number for the noise disturbance coordinator shall be conspicuously placed on construction site fences and included in the construction schedule notification sent to nearby residences. The notice to be distributed to residences and sensitive receptors shall first be submitted, for review and approval, to the MRWPCA and city and county staff as may be required by local regulations.	Injection Well Facilities and CalAm Distribution System: Alternative Monterey Pipeline	Prior to project construction	MRWPCA, CalAm, construction contractor, noise disturbance coordinator	Prior to project construction	MRWPCA and CalAm

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	<ul> <li>Mitigation Measure NV-1d: RUWAP Pipeline Construction Noise. The following measures will be implemented by the project proponents in response to comments from the Marina Coast Water District for the RUWAP alignment option of the Product Water Conveyance Pipeline:</li> <li>The construction contractor shall limit exterior construction related activities to the hours of restriction consistent with the noise ordinance of, and encroachment permits issued by, the relevant land use jurisdictions.</li> <li>The contractor shall locate all stationary noise-generating equipment as far as possible from nearby noise-sensitive receptors. Where possible, noise generating equipment shall be shielded from nearby noise-sensitive receptors by noise-attenuating buffers. Stationary noise sources located 500 feet from noise-sensitive receptors shall be equipped with noise reducing engine housings. Where possible and required by the local jurisdiction, portable acoustic barriers shall be placed around stationary noise generating equipment that is located less than 200 feet from noise-sensitive receptors.</li> <li>The contractor shall assure that construction equipment powered by gasoline or diesel engines have sound control devices at least as effective as those provided by the original equipment manufacturer (OEM). No equipment shall be permitted to have an unmuffled exhaust.</li> <li>The contractor shall assure that noise-generating mobile equipment and machinery are shut-off when not in use.</li> <li>Residences within 500 feet of a construction area shall be notified of the construction schedule in writing, prior to construction. The project proponent(s) and contractor shall designate a noise disturbance coordinator who would be responsible for responding to complaints regarding construction noise. The coordinator shall determine the cause of the complaint and ensure that reasonable measures are implemented to correct the problem. A contact number for the noise disturbance coordinator shall be conspicuously placed on cons</li></ul>	RUWAP Pipeline Alignment	Prior to project construction	MRWPCA, construction contractor, noise disturbance coordinator	Prior to project construction	MRWPCA
Impact NV-2: Construction Noise That Exceeds or Violate Local Standards	<ul> <li>Mitigation Measure NV-2a: Construction Equipment. Contractor specifications shall include a requirement that the contractor shall:</li> <li>Assure that construction equipment with internal combustion engines has sound control devices at least as effective as those provided by the original equipment manufacturer. No equipment shall be permitted to have an un-muffled exhaust.</li> <li>Impact tools (i.e., jack hammers, pavement breakers, and rock drills) used for project construction shall be hydraulically or electrically powered wherever possible to avoid noise associated with compressed air exhaust from pneumatically powered tools. Where use of pneumatic tools is unavoidable, an exhaust muffler shall be placed on the compressed air exhaust to lower noise levels by approximately 10 dBA. External jackets shall be used on impact tools, where feasible, in order to achieve a further reduction of 5 dBA. Quieter procedures shall be used, such as drills rather than impact equipment, whenever feasible.</li> <li>The construction contractor(s) shall locate stationary noise sources (e.g., generators, air compressors) as far from nearby noise-sensitive receptors as possible.</li> <li>For Product Water Conveyance pipeline segments within the City of Marina, noise controls shall be sufficient to not exceed 60 decibels for more than twenty-five percent of an hour.</li> </ul>	Reclamation Ditch Diversion, Tembladero Slough Diversion, Blanco Drain Diversion, Product Water Conveyance: (RUWAP Pipeline) segments within the City of Marina and RUWAP Booster Station	During project construction	MRWPCA construction contractor	During project construction	MRWPCA
	<b>Mitigation Measure NV-2b: Construction Hours.</b> The construction contractor shall limit all noise-producing construction activities within the City of Marina to between the hours of 7:00 AM and 7:00 PM on weekdays and between 9:00 AM and 7:00 PM Saturdays.	Product Water Conveyance: RUWAP Pipeline and Booster Pump Station in Marina	During project construction	Construction contractor	During project construction	MRWPCA
Impact PS-3: Construction Solid Waste Policies and Regulations	Mitigation Measure PS-3: Construction Waste Reduction and Recycling Plan. The construction contractor(s) shall prepare and implement a construction waste reduction and recycling plan identifying the types of construction debris the Project will generate and the manner in which those waste streams will be handled. In accordance with the California Integrated Waste Management Act of 1989, the plan shall emphasize source reduction measures, followed by recycling and composting methods, to ensure that construction and demolition waste generated by the project is managed consistent with applicable statutes and regulations. In accordance with the California Green Building Standards Code and local regulations, the plan shall specify that all trees, stumps, rocks, and associated vegetation and soils, and 50% of all other nonhazardous construction and demolition waste, be diverted from landfill disposal. The plan shall be prepared in coordination with	All components	Prior to, during, and after project construction	MRWPCA and CalAm construction contractors	Upon project completion	MRWPCA and CalAm

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	the Monterey Regional Waste Management District and be consistent with Monterey County's Integrated Waste Management Plan. Upon project completion, MRWPCA and CalAm shall collect the receipts from the contractor(s) to document that the waste reduction, recycling, and diversion goals have been met.					
Impact TR-2: Construction- Related Traffic Delays, Safety and Access Limitations	Mitigation Measure TR-2: Traffic Control and Safety Assurance Plan. Prior to construction, MRWPCA and/or its contractor shall prepare and implement a traffic control plan or plans for the roadways and intersections affected by MRWPCA control plan (or the CalAm Distribution System Improvements (Transfer and Montercy pipelines). The traffic control plan for the roadways and intersections affected by the CalAm Distribution System Improvements (Transfer and Montercy pipelines). The traffic control plan for the roadways and intersections affected by the CalAm Distribution System Improvements and will be based on detailed design plans. For all project construction activities that could affect the public right-of-way (e.g., roadways, sidewalks, and walkways), the plan shall include measures that would provide for continuity of vehicular, pedestrian, and bicyclist access: circluce the potential for traffic accidents, and ensure worker spic in construction zones. Where project construction activities could disrupt mobility and access for bicyclists and pedestrians, the plan shall include measures to ensure safe and convenient access would be maintained. The traffic control and safety assurance plan shall be developed on the basis of detailed design plans for the approved project. The plan shall include, but not necessarily be limited to, the elements listed below:  General  a. Develop circulation and detour plans to minimize impacts on local streets. As necessary, signage and/or flaggers shall be used to guide vehicles to detour routes and/or through the construction work areas.  b. Implement a public information program to notify motorists, bicyclists, nearby residents, and adjacent businesses of the impending construction activities (e.g., media coverage, email notices, websites, etc.). Notices of the location(s) and timing of lane closures string construction activities (e.g., media coverage, email notices, websites, etc.). Notices of the location(s) and timing of lane closures string peak hours. Travel lane closures	Product Water Conveyance: RUWAP Pipeline and CalAm Distribution System: Alternative Monterey Pipeline	Prior to project construction	MRWPCA and CalAm construction contractor	During project construction	MRWPCA, CalAm, and local jurisdictions

# Mitigation Monitoring and Reporting Program – Pure Water Monterey Groundwater Replenishment Project: Staff-Recommended Alternative

Impacts	Mitigation Measures	Applicable Components	Timing of Implemen- tation	Implemen- tation Responsi- bility <sup>1</sup>	Timing of Monitoring	Responsibility for Compliance Monitoring <sup>1</sup>
	retrieve all notice materials.  *Emergency Access*  m. Maintain access for emergency vehicles at all times. Coordinate with facility owners or administrators of sensitive land uses such as police and fire stations, transit stations, hospitals, and schools.  n. Provide advance notification to local police, fire, and emergency service providers of the timing, location, and duration of construction activities that could affect the movement of emergency vehicles on area roadways.  o. Avoid truck trips through designated school zones during the school drop-off and pickup hours.					
Impact TR-3: Construction- Related Roadway Deterioration	Mitigation Measure TR-3: Roadway Rehabilitation Program. Prior to commencing project construction, MRWPCA (for all components other than the CalAm Distribution System Improvements) and CalAm (for CalAm Distribution System Improvements) shall detail the preconstruction condition of all local construction access and haul routes proposed for substantial use by project-related construction vehicles. The construction routes surveyed must be consistent with those identified in the construction traffic control and safety assurance plan developed under Mitigation Measure TR-2. After construction is completed, the same roads shall be surveyed again to determine whether excessive wear and tear or construction damage has occurred. Roads damaged by project-related construction vehicles shall be repaired to a structural condition equal to, or greater than, that which existed prior to construction activities. In the City of Marina, the construction in the city rights-way must comply with the City's design standards, including restoration of the streets from curb to curb, as applicable. In the City of Monterey, asphalt pavement of full travel lanes will be resurfaced without seams along wheel or bike paths.	All components	Prior to project construction, after project construction	MRWPCA and CalAm construction contractors	After project construction	MRWPCA, CalAm, and local jurisdictions
Impact TR-4: Construction Parking Interference	Mitigation Measure TR-4: Construction Parking Requirements. Prior to commencing project construction, the construction contractor(s) shall coordinate with the potentially affected jurisdictions to identify designated worker parking areas that would avoid or minimize parking displacement in congested areas of Marina, Seaside, and downtown Monterey. The contractors shall provide transport between the designated parking location and the construction work areas. The construction contractor(s) shall also provide incentives for workers that carpool or take public transportation to the construction work areas. The engineering and construction design plans shall specify that contractors limit time of construction within travel lanes and public parking spaces and provide information to the public about locations of alternative spaces to reduce parking disruptions.	Product Water Conveyance: RUWAP Pipeline Alignment in Marina and Seaside and CalAm Distribution System: Alternative Monterey Pipeline	Prior to project construction	MRWPCA and CalAm construction contractor	During project construction	MRWPCA City of Marina, City of Seaside, City of Monterey



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RUWAP EIR Mitigation Measure with text edits to apply specifically to the RWP shown in strikeout for deleted text and underline for added text.	Timing of Imple- mentation	Responsibility for Implementation	Verified for Compliance by:
<b>4.1-R1:</b> Prior to the finalization of project specific plans, the design engineer and MCWD should ensure that the design and placement of the final treatment and filtration facility and pump/lift stations will minimize impacts on the aesthetic nature of their surrounding areas, by providing screening using decorative fencing, vegetation, and painting new buildings and facilities in a color that will blend in with the surrounding landscape.	Prior to finalizing project design	Contractor and MCWD	MCWD
<ul> <li>4.3-R1: The contractors shall adhere to the following requirements as required to reduce particulate matter emissions below the MBUAPCD threshold:</li> <li>water all active construction areas as required with non-potable sources to the extent feasible; frequency should be based on the type of operation, soil, and wind exposure and minimized to prevent wasteful use of water.</li> <li>prohibit grading activities during periods of high wind (over 15 mph).</li> <li>cover all trucks hauling soil, sand, and other loose materials or require all trucks to maintain at least two feet of freeboard,</li> <li>pave or apply water three times daily or apply non-toxic soil stabilizers on all unpaved access roads, parking areas &amp; staging areas at construction sites,</li> <li>sweep daily (with water sweepers) all paved access roads, parking areas and staging areas at construction sites,</li> <li>sweep streets daily (with water sweepers) if visible soil material is carried onto adjacent public streets,</li> <li>hydroseed or apply (non-toxic) soil stabilizers to inactive construction areas (previously graded areas inactive for ten days or more),</li> <li>enclose, cover, water twice daily or apply (non-toxic) soil binders to exposed stockpiles (dirt, sand, etc.),</li> <li>limit traffic speeds on unpaved roads to 15 mph,</li> <li>install appropriate best management practices or other erosion control measures to prevent silt runoff to public roadways,</li> <li>replant vegetation in disturbed areas as quickly as possible,</li> <li>install wheel washers for all exiting trucks, or wash off the tires or tracks of all trucks and equipment leaving the site,</li> <li>limit traffic speeds on unpaved roads to 15 mph,</li> <li>opst a publicly visible sign which specifies the telephone number and person to contact regarding dust complaints (the person shall respond to complaints and take corrective action within 48 hours), and</li> <li>ensure that the phone number of MBUAPCD is visible to ensure compliance with Rule 402 (Nuisance). (Pl</li></ul>	During Construction	Contractor and MCWD	MBUAPCD
<b>4.3-R2:</b> Subject to approval by the MBUAPCD prior to and, as needed, during project construction approval and implementation, MCWD and the contractor shall implement measures to reduce or eliminate diesel exhaust emissions to meet identified thresholds of significance, such as reduction in hours of operation of equipment contributing to such emissions or by utilizing oxidation catalysts or catalytic particulate matter filters on all diesel powered equipment above 50 horsepower that require CARB-certified low-sulfur diesel fuel (less than or equal to 15 parts per million by weight (ppmw)). Site-specific risk assessment may be required to determine the appropriate measures to implement.	Confirm with MBUAPCD prior to project construction; implement measures during	Contractor and MCWD	MBUAPCD
<ul> <li>4.4-R1: Conduct Pre-Construction Survey. A qualified biologist shall conduct a pre-construction survey for Hickman's onion special-status plant species to determine presence of this these species. The biologist shall prepare a report that provides the results of the survey, including a description of the baseline habitat conditions, and, if found, the number of individuals and location of the populations identified within the area of impact. If no individual are found, no further mitigation is necessary. If individuals are found, the following measures shall be implemented:</li> <li>Based on the results of the report, the design of the alternative shall avoid individuals to the maximum extent possible.</li> <li>If avoidance is not feasible, a Rare Plant Restoration Plan shall be prepared by a qualified biologist and implemented. The plan shall include, but is not limited to, the following: <ul> <li>a description of the baseline conditions of the habitats within the area of impact, including the presence of any special-status species, their locations, and densities;</li> <li>procedures to control non-native species invasion and elimination of existing non-native species within the area of impact;</li> <li>provisions for ongoing training of facility maintenance personnel to ensure compliance with the requirements of the plan;</li> <li>a detailed description of on-site and off-site restoration areas, salvage of seed and/or soil bank, plant salvage, seeding and planting specifications; and</li> <li>a monitoring program that describes annual monitoring efforts which incorporate success criteria and contingency plans if success criteria are not met.</li> </ul> </li> </ul>	Prior to project construction (within 30 days)	Qualified Biologist and Contractor	MCWD
<b>4.4-R2:</b> Conduct Pre-Construction Surveys for Burrowing Owls and Implement CDFG Guidelines. Pre-construction surveys shall be conducted to locate active nesting burrows. Surveys will consist of visually checking the area within 500 feet of the proposed storage reservoir site within 30 days of initiating construction. If active nests are found, no-disturbance buffers shall be established around all active nesting burrows during the breeding season, and the CDFG burrowing owl guidelines shall be implemented during the non-breeding season. If no burrowing owls are found, no further mitigation measures are required.  Breeding season: If active nests are found, biologist shall establish a 250-foot buffer zone around each burrow. No construction activities shall be permitted within the zone until after the breeding season, which extends from February 1 to August 21, or until it is determined that the young have fledged.  Winter Season: Adult burrowing owls can occupy burrows year-round. Therefore, before construction activities begin in the vicinity of active burrows (and following the breeding season), CDFG mitigation guidelines for burrowing owls (1995) shall be implemented. The guidelines require that one-way doors be installed at least 48 hours before construction at all active burrows that exist	Prior to project construction (within 30 days)	Qualified Biologist and Contractor	MCWD

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RUWAP EIR Mitigation Measure with text edits to apply specifically to the RWP shown in strikeout for deleted text and underline for added text.	Timing of	Responsibility	Verified for	X
<u></u>	Imple-	for	Compliance	
	mentation	Implementation	by:	
within the construction area so that the burrows are not occupied during construction. The guidelines also require installation of two artificial burrows for each occupied burrow that is removed.				
Qualified wildlife biologists shall conduct pre-construction surveys for burrowing owls within 30 days of initiating construction activities. The one-way doors shall be installed at that time to ensure				
that the owls can get out of the burrows and not back in. Artificial burrows shall be constructed within the area prior to installation of the one-way doors.				
(Please note that mitigation measure 4.4-R2 is consistent with mitigation measure BT-11 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).				

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RUWAP EIR Mitigation Measure with text edits to apply specifically to the RWP shown in strikeout for deleted text and underline for added text.	Timing of Imple- mentation	Responsibility for Implementation	Verified for Compliance by:
<b>1.4-R3:</b> A Memorandum of Understanding (MOU) with CDFG shall be obtained to allow a qualified biologist to remove and relocate coast horned lizards from the construction area if encountered luring construction activities. The MOU shall include, but is not limited to, the methods of capture and handling, an estimation of the number expected to be captured and handled, the duration of capture and handling, and a description of the established relocation area. If the relocation is proposed to occur outside of the project site, MCWD must coordinate and obtain approval from the andowner. Details of this procedure shall be reviewed by CDFG and implemented by a qualified biologist.	Prior to construction	Qualified Biologist and MCWD	CDFG
1.4-R4: Conduct Construction Monitoring Program for coast horned lizards, which includes procedures for capture and release. A qualified biologist shall remain on-site during initial grading activities to salvage and move coast horned lizards that may be uncovered during earthmoving activities. Recovered individuals shall be placed in appropriate habitat outside of the within the project in accordance with the MOU with CDFG. The monitor shall walk alongside the grading equipment in each new area of disturbance, and shall have the authority to halt construction temporarily if necessary to capture and relocate an individual. Any individual captured in the grading zone shall be relocated as soon as possible to adjacent suitable habitat outside of the area of impact.	During Construction	Qualified Biologist and Contractor	MCWD
BT-1j: Conduct Pre-Construction Surveys for American Badger <sup>1</sup> . To avoid and reduce impacts to the American badger, the project proponents shall retain a qualified biologist to conduct focused pre-construction surveys for badger dens in all suitable habitat proposed for construction, ground disturbance, or staging no more than two weeks prior to construction. If no potential badger dens are present, no further mitigation is required. If potential dens are observed, the following measures are required to avoid potential significant impacts to the American badger:			
If the qualified biologist determines that potential dens are inactive, the biologist shall excavate these dens by hand with a shovel to prevent badgers from reusing them during construction.  If the qualified biologist determines that potential dens may be active, the den shall be monitored for a period sufficient (as determined by a qualified biologist) to determine if the den is a maternity den occupied by a female and her young, or if the den is occupied by a solitary badger.  Maternity dens occupied by a female and her young shall be avoided during construction and a minimum buffer of 200 feet in which no construction activities shall occur shall be maintained around the den. After the qualified biologist determines that badgers have stopped using active dens within the project boundary, the dens shall be hand-excavated with a shovel to prevent reuse during construction.	Prior to project construction	MCWD construction contractors and qualified biologists	MCWD qualified biologist
Solitary male or female badgers shall be passively relocated by blocking the entrances of the dens with soil, sticks, and debris for three to five days to discourage the use of these dens prior to project construction disturbance. The den entrances shall be blocked to an incrementally greater degree over the three to five day period. After the qualified biologist determines that badgers have stopped using active dens within the project boundary, the dens shall be hand-excavated with a shovel to prevent re-use during construction.	Dri - u 4-	Owlife	MCWD
3T-1k: Conduct Pre-Construction Surveys for Protected Avian Species, including, but not limited to, white-tailed kite and California horned lark. Prior to the start of construction activities, a qualified biologist shall conduct pre-construction surveys for suitable nesting habitat within the Project Study Area and within a suitable buffer area from the Project Study Area. The qualified biologist shall determine the suitable buffer area based on the avian species with the potential to nest at the site. In areas where nesting habitat is present within the project area or within the determined suitable buffer area, construction activities that may directly (e.g., vegetation removal) or indirectly (e.g., noise/ground disturbance) affect protected nesting avian species shall be timed to avoid the breeding and nesting season. Specifically, vegetation and/or tree removal can be scheduled after September 6 and before January 31. Alternatively, a qualified biologist shall be retained by the project proponents to conduct pre-construction surveys for nesting raptors and other protected avian species where nesting habitat was identified and within the suitable buffer area if construction commences between February 1 and September 15. Pre-construction surveys shall be conducted no more than 4 days prior to the start of construction activities during the early part of the breeding season (February through April) and no more than 30 days prior to the initiation of these activities during the ate part of the breeding season (May through August). Because some bird species nest early in spring and others nest later in summer, surveys for nesting birds may be required to continue during construction to address new arrivals, and because some species breed multiple times in a season. The necessity and timing of these continued surveys shall be determined by the qualified biologist assed on review of the final construction plans.  If active raptor or other protected avian species nests are identified during the preconstruction surv	Prior to Construction if it occurs between Aug. 1 & Apr. 14	Qualified Biologist and MCWD	MCWD
4.4-R6: Conduct Pre-Construction Surveys for Coast Horned Larks and Loggerhead Shrike. A qualified biologist shall perform pre-construction surveys for active nests of these two species prior to construction (within 30 days of construction initiation). If active nests are found, a suitable construction buffer shall be established by a qualified biologist until the young of the year have fledged. Alternatively, construction activities that may affect nesting raptors can be timed to avoid the nesting season (generally the nesting season is April 15 to August 1).	Prior to Construction if it occurs between Aug. 1 & Apr. 14	Qualified Biologist and MCWD	MCWD
1.4-R7: A Revegetation Plan shall be prepared by a qualified biologist to revegetate and restore impacted habitat. This plan shall include a list of appropriate species, planting specifications,	Prior to	Qualified	MCWD

<sup>&</sup>lt;sup>1</sup> Mitigation Measure BT-1j was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance, The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015. Denise Duffy & Associates, Inc. Page 3 of 13

October 18, 2006, amended November 18, 2016

NOTES: Section 21081.6 of the Public Resources Code requires all state and local agencies to establish monitoring or reporting programs whenever approval of a project relies upon an environmental impact report (EIR). The purpose of the monitoring or reporting program is to ensure implementation of the measures being imposed to mitigate or avoid the significant adverse environmental impacts identified in the EIR as amended in Addendum No. 1 to the certified Final EIR for the MCWD Regional Urban Water Augmentation Project.

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WAF EIR whitigation weasure with text edits to apply specifically to the RWP shown in salkeout for defeted text and underline for added text.	Imple- mentation	for Implementation	Compliance by:
monitoring procedures, success criteria, and contingency plan if success criteria are not met.	construction	Biologist and Contractor	
<b>4.4-R8:</b> Conduct an Employee Education Program for Construction Crew and MCWD staff prior to construction activities. A qualified biologist (if necessary, the biological monitor) shall meet with the construction crew at the onset of construction to educate the construction crew on the following: 1) the appropriate access route in and out of the construction area; 2) how biological monitor will examine the area and agree upon a method which will ensure the safety of the monitor during such activities, 3) the special-status species that may be present; 4) the specific mitigation measures that will be incorporated into the construction effort; 5) the general provisions and protections afforded by the USFWS and CDFW; and 6) the proper procedures if a special-status animal or any other animal is encountered within the project site. Refer to Mitigation Measure 4.4 D8 above.  (Please note that mitigation measure 4.4-R8 is consistent with mitigation measure BT-1s #1 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	Prior to construction	Qualified Biologist and Contractor	MCWD
<b>4.4-R9:</b> Trees and vegetation not planned for removal shall be protected during construction to the maximum extent possible. This includes the use of exclusionary fencing of herbaceous and shrubby vegetation, such as hay bales, and protective wood barriers for trees. Only certified weed-free straw shall be used to avoid the introduction of non-native, invasive species. A biological monitor shall supervise the installation of protective fencing and monitor at least once per week until construction is complete to ensure that the protective fencing remains intact. (Please note that mitigation measure 4.4-R9 is consistent with mitigation measure BT-1s #2 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	Prior, during, and post construction	Contractor	MCWD
<b>4.4-R10:</b> Following construction, disturbed areas shall be restored to pre-project contours to the maximum extent possible and revegetated using locally-occurring native species and native erosion control seed mix, per the requirements of the Revegetation Plan. (Please note that mitigation measure 4.4-R10 is consistent with mitigation measure BT-1s #4 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	Following construction	Contractor	MCWD
<b>4.4-R11:</b> Protective fencing shall be placed prior to and during construction so as to keep construction vehicles and personnel from impacting vegetation adjacent to the project site outside of work limits. A biological monitor shall supervise the installation of protective fencing and monitor at least once per week until construction is complete to ensure that the protective fencing remains intact. (Please note that mitigation measure 4.4-R11 is consistent with mitigation measure BT-1s #3 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	Prior, during, and post construction	Contractor	MCWD
<b>4.4-R12:</b> Grading, excavating, and other activities that involve substantial soil disturbance shall be planned and carried out in consultation with a qualified hydrologist, engineer, or erosion control specialist, and shall utilize standard erosion control techniques to minimize erosion and sedimentation to native vegetation.  (Please note that mitigation measure 4.4-R12 is consistent with mitigation measure BT-1a #5 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	Prior, during, and post construction	Contractor & qualified hydrologist/engineer	MCWD
<b>4.4-R13:</b> A representative shall be appointed by MCWD who will be the contact source for any employee or contractor who may inadvertently kill or injure a special-status species or find one dead, injured, or trapped. The representative shall be notified immediately to notify USFWS and CDFG. The representative shall be identified during the Employee Education Program and his/her contact information shall be provided to USFWS and CDFG.	Prior to construction	Appointed Representative and Contractor	MCWD
<b>4.4-R14:</b> If maintenance activities require ground disturbance, the impacts shall be subject to the requirements of the Revegetation Plan described in Mitigation Measure 4.4-R7.	Ongoing if maintenance requires ground disturbance	MCWD	MCWD
4.4-R15: Conduct an Employee Education Program for Maintenance Construction Crew and other MCWD staff prior to project implementation construction activities. A biological monitor shall meet with the maintenance crew at the onset of project operations to educate the crew on the following: 1) the appropriate access route in and out of the facility area; 2) how biological monitor will examine the area and agree upon a method which will ensure the safety of the monitor during such activities, 3) the special-status species that may be present; 4) the specific mitigation measures that will apply to maintenance activities; 5) the general provisions and protections afforded by the USFWS and CDFW; and 6) the proper procedures if a special-status animal or any other animal is encountered within the project site. Refer to Mitigation Measure 4.4-D8 above.  (Please note that mitigation measure 4.4-R8 is consistent with mitigation measure BT-1a #1 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	Prior to construction	Qualified Biologist and MCWD	MCWD
BT-1a: Implement Construction Best Management Practices <sup>2</sup> . The following best management practices shall be implemented during all identified phases of construction (i.e., pre-, during, and post-) to reduce impacts to special-status plant and wildlife species:  1. No firearms shall be allowed on the construction sites at any time.  2. To protect against spills and fluids leaking from equipment, the project proponent shall require that the construction contractor maintains an on-site spill plan and on-site spill containment measures that can be easily accessed.  3. Refueling or maintaining vehicles and equipment should only occur within a specified staging area that is at least 100 feet from a waterbody (including riparian and wetland habitat) and that has sufficient management measures that will prevent fluids or other construction materials including water from being transported into waters of the state. Measures shall include confined concrete washout areas, straw wattles placed around stockpiled materials and plastic sheets to cover materials from becoming airborne or otherwise transported due to wind or rain into surface waters.	Prior to, during and after project construction	MCWD construction contractors and qualified biologist	MCWD qualified biologist and construction biological monitor;

<sup>&</sup>lt;sup>2</sup> Mitigation Measure BT-1a was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance, The other components of BT-1a as identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP, are identified within this MMRP. The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015.

Denise Duffy & Associates, Inc.

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Verified for X

Compliance

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RUWAP EIR Mitigation Measure with text edits to apply specifically to the RWP shown in strikeout for deleted text and underline for added text.

BT-1b: Implement Construction-Phase Monitoring<sup>3</sup>. The project proponents shall retain a qualified biologist to monitor all ground disturbing construction activities (i.e., vegetation removal, grading, excavation, or similar activities) to protect any special-status species encountered. Any handling and relocation protocols of special-status wildlife species shall be determined in coordination with CDFW prior to any ground disturbing activities, and conducted by a qualified biologist with appropriate scientific collection permit. After ground disturbing project activities are complete, the qualified biologist shall train an individual from the construction crew to act as the on-site construction biological monitor. The construction biological monitor shall be the contact for any special-status wildlife species encounters, shall conduct daily inspections of equipment and materials stored on site and any holes or trenches prior to the commencement of work, and shall ensure that all installed fencing stays in place throughout the construction period. The qualified biologist shall then conduct regular scheduled and unscheduled visits to ensure the construction biological monitor is satisfactorily implementing all appropriate mitigation protocols. Both the qualified biologist and the construction biological monitor shall have the authority to stop and/or redirect project activities to ensure protection of resources and compliance with all environmental permits and conditions of the project. The qualified biologist and the construction monitor shall complete a daily log summarizing activities and environmental compliance throughout the duration of the project. The log shall also include any special-status wildlife species observed and relocated.

BT-1c: Implement Non-Native, Invasive Species Controls<sup>4</sup>. The following measures shall be implemented to reduce the introduction and spread of non-native, invasive species:

- 1. Any landscaping or replanting required for the project shall not use species listed as noxious by the California Department of Food and Agriculture (CDFA).
- 2. Bare and disturbed soil shall be landscaped with CDFA recommended seed mix or plantings from locally adopted species to preclude the invasion on noxious weeds in the Project Study Area.
- 3. Construction equipment shall be cleaned of mud or other debris that may contain invasive plants and/or seeds and inspected to reduce the potential of spreading noxious weeds, before mobilizing to arrive at the construction site and before leaving the construction site.
- 4. All non-native, invasive plant species shall be removed from disturbed areas prior to replanting.

BT-1d: Conduct Pre-Construction Surveys for California Legless Lizard<sup>5</sup>. The project proponents shall retain a qualified biologist to prepare and implement a legless lizard management plan in coordination with CDFW, which shall include, but is not limited to, the protocols for pre-construction surveys, construction monitoring, and salvage and relocation. The management plan shall include, but is not limited to, the following:

- Pre-Construction Surveys. Pre-construction surveys for legless lizards shall be conducted in all suitable habitat proposed for construction, ground disturbance, or staging. The qualified biologist shall hold or obtain a CDFW scientific collection permit for this species. The pre-construction surveys shall use a method called "high-grading." The high grading method shall include surveying the habitat where legless lizards are most likely to be found, and the survey must occur under the conditions when legless lizards are most likely to be seen and captured (early morning, high soil moisture, overcast, etc.). The intensity of a continued search may then be adjusted, based on the results of the first survey in the best habitat. A "three pass method" shall be used to locate and remove as many legless lizards as possible. A first pass shall locate as many legless lizards as possible, a second pass should locate fewer lizards than the first pass, and a third pass should locate fewer lizards than the second pass. All search passes shall be conducted in the early morning when legless lizards are easiest to capture. Vegetation may be removed by hand to facilitate hand raking and search efforts for legless lizards in the soil under brush. If lizards are found during the first pass, an overnight period of no soil disturbance must occur before the second pass, and the same requirement shall be implemented after the second pass. If no lizards are found during the second pass, a third pass is not required. Installation of a barrier, in accordance with the three pass method, shall be required if legless lizards are found at the limits of construction (project boundaries) and sufficient soft sand and vegetative cover are present to suspect additional lizards are in the immediate vicinity on the adjacent property. A barrier shall prevent movement of legless lizards into the property. All lizards discovered shall be handled according to the salvage procedures outlined below.
- Construction Monitoring. Monitoring by a qualified biologist shall be ongoing during construction. The onsite monitor shall be present during all ground disturbing construction activities. To facilitate the careful search for lizards during construction, vegetation may need to be removed. If removal by hand is impractical, equipment such as a chainsaw, string trimmer, or skid-steer may be used, if a monitor and crew are present. The task of the vegetation removal is to remove plants under the direction of the monitor, allowing the monitor to watch for legless lizards. After plants are removed, the monitor and crew shall search the exposed area for legless lizards. If legless lizards are found during preconstruction surveys or construction monitoring, the protocols for salvage and relocation identified below shall be followed. Upon completion of pre-construction surveys, construction monitoring, and any resulting salvage and relocation actions, a report shall be submitted to the CDFW. The CDFW must be notified at least 48 hours before any field activity begins.
- Salvage and Relocation. Only experienced persons may capture or handle legless lizards. The monitor must demonstrate a basic understanding, knowledge, skill, and experience with this species and its habitat. Once captured, a lizard shall be placed in a lidded, vented box containing clean sand. Areas of moist and dry sand need to be present in the box. The boxes must be kept out of direct sunlight and protected from temperatures over 72°F. The sand must be kept at temperatures under 66°F. Ideal temperatures are closer to 60°F. On the same day as capture, the lizards shall be examined for injury and data recorded on location where found as well as length, color, age, and tail condition. Once data is recorded, lizards shall be relocated to appropriate habitat, as determined through coordination with the CDFW, qualified biologist, and potential landowners.

<sup>4</sup> Mitigation Measure BT-1c was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance, The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015.

Page 5 of 13

<sup>&</sup>lt;sup>3</sup> Mitigation Measure BT-1b was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance, The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015.

<sup>&</sup>lt;sup>5</sup> Mitigation Measure BT-1d was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance, The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015.

NOTES: Section 21081.6 of the Public Resources Code requires all state and local agencies to establish monitoring or reporting programs whenever approval of a project relies upon an environmental impact report (EIR). The purpose of the monitoring or reporting program is to ensure implementation of the measures being imposed to mitigate or avoid the significant adverse environmental impacts identified in the EIR as amended in Addendum No. 1 to the certified Final EIR for the MCWD Regional Urban Water Augmentation Project.

For those project features outside of MCWD's service areas (specifically at the Monterey Regional Water Pollution Control Association Regional Treatment Plant and within the Monterey Peninsula/Cal-Am Service Area) the lead agency and/or project

RUWAP EIR Mitigation Measure with text edits to apply specifically to the RWP shown in strikeout for deleted text and underline for added text.	Timing of Imple- mentation	Responsibility for Implementation	Verified for Compliance by:
Suitability of habitat for lizard release must be evaluated and presented in a management plan. The habitat must contain habitat factors most important to the health and survival of the species such as appropriate habitat based on soils, vegetated cover, native plant species providing cover, plant litter layer and depth, soil and ambient temperature, quality and composition of invertebrate population and prey availability. Potential relocation sites that contain the necessary conditions may exist within the habitat reserves on the former Fort Ord, including the Fort Ord National Monument. Lizards shall be marked with a unique tag (pit or tattoo) prior to release. Release for every lizard shall be recorded with GPS. GPS locations shall be submitted as part of the survey result report to			V
document the number and locations of lizards relocated.			
BT-1e: Prepare and Implement Rare Plant Restoration Plan to Mitigate Impacts to Sandmat Manzanita, Monterey Ceanothus, Monterey Spineflower, Eastwood's Goldenbush, Coast Wallflower, and Kellogg's Horkelia <sup>6</sup> . Impacts to rare plant species individuals shall be avoided through project design and modification, to the extent feasible while taking into consideration other site and engineering constraints. If avoidance is not possible, the species shall be replaced at a 1:1 ratio for area of impact through preservation, restoration, or combination of both. A Rare Plant Restoration Plan, approved by the lead agency prior to commencing construction on the project site, shall be prepared and implemented by a qualified biologist. The plan shall include, but is not limited to, the following:  a. A detailed description of on-site and/or off-site mitigation areas, salvage of seed and/or soil bank, plant salvage, seeding and planting specifications, including, if appropriate, increased planting ratio to ensure the applicable success ratio. Specifically, seed shall be collected from the on-site individuals that would be impacted and grown in a local greenhouse, and then transplanted within the mitigation area. Plants shall be transplanted while they are young seedlings in order to develop a good root system. Alternatively, the mitigation area may be broadcast seeded in fall; however, if this method is used, some seed shall be retained in the event that the seeding fails to produce viable plants and contingency measures need to be employed.	Prior to project construction	Project engineers, project biologist, MCWD	MCWD qualified biologist
<ul> <li>A description of a 3-year monitoring program, including specific methods of vegetation monitoring, data collection and analysis, restoration goals and objectives, success criteria, adaptive management if the criteria are not met, reporting protocols, and a funding mechanism.</li> <li>The mitigation area shall be preserved in perpetuity through a conservation easement or other legally enforceable land preservation agreement. Exclusionary fencing shall be installed around the mitigation area to prevent disturbance until success criteria have been met.</li> </ul>			
BT-1g: Conduct Pre-Construction Surveys for Special-Status Bats <sup>7</sup> . To avoid and reduce impacts to special-status bat species, the project proponents shall retain a qualified bat specialist or wildlife biologist to conduct site surveys during the reproductive season (May 1 through September 15) to characterize bat utilization of the site and potential species present (techniques utilized to be determined by the biologist) prior to tree or building removal. Based on the results of these initial surveys, one or more of the following shall occur:			
• If it is determined that bats are not present at the site, no additional mitigation is required.			
• If it is determined that bats are utilizing the site and may be impacted by the Project, pre-construction surveys shall be conducted no more than 30 days prior to any tree or building removal (or any other suitable roosting habitat) within 100 feet of construction limits. If, according to the bat specialist, no bats or bat signs are observed in the course of the pre-construction surveys, tree and building removal may proceed. If bats and/or bat signs are observed during the pre-construction surveys, the biologist shall determine if disturbance would jeopardize a maternity roost or another type of roost (i.e., foraging, day, or night).	Prior to project construction	MCWD qualified biologist (bat/wildlife specialist)	MCWD and qualified biologist
• If a single bat and/or only adult bats are roosting, removal of trees, buildings, or other suitable habitat may proceed after the bats have been safely excluded from the roost. Exclusion techniques shall be determined by the biologist and would depend on the roost type.		specialist)	
If an active maternity roost is detected, avoidance is preferred. Work in the vicinity of the roost (buffer to be determined by biologist) shall be postponed until the biologist monitoring the roost determines that the young have fledged and are no longer dependent on the roost. The monitor shall ensure that all bats have left the area of disturbance prior to initiation of pruning and/or removal of trees that would disturb the roost. If avoidance is not possible and a maternity roost must be disrupted, authorization from CDFW shall be required prior to removal of the roost.			
BT-1h: Implementation of Mitigation Measures BT-1a and BT-1b to Mitigate Impacts to the Monterey Ornate Shrew, Coast Horned Lizard, Coast Range Newt, Two-Striped Garter Snake, and Salinas Harvest Mouse <sup>8</sup> . If these species are encountered, implementation of Mitigation Measures BT-1a and BT-1b, which avoid and minimize impacts through implementing construction best management practices and monitoring, would reduce potential impacts to these species to a less-than-significant level.	Prior to and during project construction	MCWD contractors and qualified biologists	MCWD qualified biologist
BT-1i: Conduct Pre-Construction Surveys for Monterey Dusky- Footed Woodrat <sup>9</sup> . To avoid and reduce impacts to the Monterey dusky-footed woodrat, the project proponents shall retain a qualified biologist to conduct pre-construction surveys in suitable habitat proposed for construction, ground disturbance, or staging within three days prior to construction for woodrat nests within the	Prior to project construction	MCWD contractors and	MCWD

<sup>&</sup>lt;sup>6</sup> Mitigation Measure BT-1e was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance. The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015.

Page 6 of 13

Mitigation Measure BT-1g was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance. The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015.

<sup>8</sup> Mitigation Measure BT-1h was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance. The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015.

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**RUWAP EIR Mitigation Measure** with text edits to apply specifically to the RWP shown in strikeout for deleted text and underline for added text.

Verified for X

Responsibility

Timing of

Implefor Compliance mentation **Implementation** by: project area and in a buffer zone 100 feet out from the limit of disturbance. All woodrat nests shall be flagged for avoidance of direct construction impacts and protection during construction, where qualified feasible. Nests that cannot be avoided shall be manually deconstructed prior to land clearing activities to allow animals to escape harm. If a litter of young is found or suspected, nest material shall be biologists replaced, and the nest left alone for 2-3 weeks before a re-check to verify that young are capable of independent survival before proceeding with nest dismantling. **4.4-R18:** A Memorandum of Understanding (MOU) with CDFG shall be obtained for a qualified biologist to remove and relocate black legless lizards, coast horned lizards, and globose dune beetles Prior to Qualified **CDFG** from the construction area if encountered during construction activities. The MOU shall include, but is not limited to, the methods of capture and an estimation of the number of individuals expected Biologist and construction to be captured and handled, the duration of capture and handling, and a description of the established relocation area. If the relocation is proposed to occur outside of the project site, MCWD must MCWD coordinate and obtain approval from the landowner. Details of this procedure shall be reviewed by CDFG and implemented by a qualified biologist. 4.4-R19: Conduct Construction Monitoring Program for Black Legless Lizards, which includes procedures for capture and release. A qualified biologist shall remain on-site during initial grading Oualified **MCWD** During activities to salvage and move lizards that may be uncovered during earthmoving activities. Recovered individuals shall be placed in appropriate habitat outside of the within the project site in Construction Biologist and accordance with the MOU with CDFG. The monitor shall walk alongside the grading equipment in each new area of disturbance, and shall have the authority to halt construction temporarily if Contractor necessary to capture and relocate an individual. Any individual captured in the grading zone shall be relocated as soon as possible to adjacent suitable habitat outside of the area of impact. 4.4-R22: All food-related and other trash shall be disposed of in closed containers and removed from the project area at least once a week during the construction period, or more often if trash is MCWD During Contractor attracting avian or mammalian predators. Construction personnel shall not feed or otherwise attract wildlife to the area. construction (Please note that mitigation measure 4.4-R22 is consistent with mitigation measure BT-1a #7 from Final Pure Water Monterey Groundwater Replenishment Project MMRP). BT-4. HMP Plant Species Salvage<sup>10</sup>. For impacts to the HMP plant species within the Project Study Area that do not require take authorization from USFWS or CDFW, salvage efforts for these species shall be evaluated by a qualified biologist per the requirements of the HMP and BO. A salvage plan shall be prepared and implemented by a qualified biologist, which shall would include, but is not limited to: a description and evaluation of salvage opportunities and constraints; a description of the appropriate methods and protocols of salvage and relocation efforts; identification of Prior to, during. MCWD relocation and restoration areas; and identification of qualified biologists approved to perform the salvage efforts, including the identification of any required collection permits from USFWS and/or and after MCWD Biologist qualified CDFW. Where proposed, seed collection shall occur from plants within the Project Study Area and topsoil shall be salvaged within occupied areas to be disturbed. Seeds shall be collected during the construction biologist appropriate time of year for each species by qualified biologists. At the time of seed collection, a map shall also be prepared that identifies the specific locations of the plants for any future topsoil preservation efforts. The collected seeds shall be used to revegetate temporarily disturbed construction areas and reseeding and restoration efforts on- or off-site, as determined appropriate in the salvage plan. 4.6-R1 See Note 1 4.6-R2: If buried human remains are encountered during construction, work within 50 meters (±160 feet) of the find must halt and the archaeologist and the coroner immediately notified. If the find **MCWD** During Qualified is determined to be significant, appropriate mitigation measures shall be formulated and implemented. If the remains are determined to be Native American, then the NAHC must be notified within 24 construction Archaeologist and hours as required by Public Resources Code 5097. The NAHC will notify designated Most Likely Descendants who will provide recommendations for the treatment of the remains within 24 hours. MCWD The NAHC will mediate any disputes regarding treatment of remains. (Please note that mitigation measure 4.6-R2 is consistent with mitigation measure BT-1s #1 from Final Pure Water Monterey Groundwater Replenishment Project MMRP). CR-2c: Native American Notification<sup>11</sup>. Because of their continuing interest in potential discoveries during construction, all listed Native American Contacts shall be notified of any and all MCWD and MCWD and discoveries of archaeological resources in the project area. qualified During project qualified construction archaeologis archaeologist t MCWD 4.6-R3: MCWD shall comply with the policies and programs for the Cities of Marina, Seaside, and Monterey, and Monterey County relating to protecting resources and identifying additional All phases of Oualified Archaearchaeological sites that may be affected by project implementation. project ologist & MCWD 4.6-R4: Unsurveyed areas within the areas proposed for ground disturbance or other construction activities shall be inventoried for the presence of cultural resources. This would include surface Qualified MCWD Prior to and examination of the project site. Cultural resources, if found, shall be recorded on State Forms DPR 523 depending on the type of resource. After field studies are completed, an Archaeological during Archaeologist and Survey Report will be prepared, as appropriate, for documenting the type(s) of resources encountered. construction MCWD 4.6-R5: If cultural resources cannot be avoided, they shall be evaluated for CEQA significance. The purpose of which would be to define a course of action to satisfy CEQA requirements for an All phases of Oualified MCWD Assessment of Effects. If cultural resources are considered significant resources per CEOA, then a data recovery program shall be implemented to reduce impacts to less-than-significant levels as Archaeologist and project required by CEOA Guidelines. MCWD

Page 7 of 13

<sup>9</sup> Mitigation Measure BT-1i was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance, The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015.

<sup>10</sup> Mitigation Measure BT-4 was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance. The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015.

<sup>11</sup> Mitigation Measure CR-2c was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance, The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015. Denise Duffy & Associates, Inc.

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RUWAP EIR Mitigation Measure with text edits to apply specifically to the RWP shown in strikeout for deleted text and underline for added text.	Timing of Imple- mentation	Responsibility for Implementation	Verified for Compliance by:
<b>4.6-R6:</b> To insure that no inadvertent damage occurs to cultural resources, the resource boundaries should be marked as exclusion zones both on the ground and on construction maps. Construction supervisory personnel should be notified of the existence of these resources and required to keep personnel and equipment away from these areas. Periodic monitoring of cultural resources to be avoided should be completed by MCWD to insure that no inadvertent damage to the resources occurs as a result of construction or construction-related activities.	Prior to construction	Qualified Archaeologist and MCWD	MCWD
<b>4.6-R7:</b> Prior to the initiation of construction or ground-disturbing activities adjacent to cultural resources, all construction personnel should be alerted to the possibility of buried cultural remains. Personnel should be instructed that upon discovery of cultural materials, no collection is to be undertaken and work in the immediate area of the find should be halted and MCWD be notified. During construction and operation, personnel and equipment will be restricted to the corridor surveyed for archaeological resources.	All phases of project	Qualified Archaeologist and MCWD	MCWD
<b>4.6-R8:</b> Unsurveyed areas within proposed areas of ground disturbance or other construction activities shall be inventoried for the presence of historical resources. This would include surface examination of the project site. Historical resources, if found, shall be recorded on State Forms DPR 523 depending on the type of resource. The proposed alternative shall comply with the Office of Historic Preservation's instructions for recording historical resources. Refer to http://www.ohp.parks.ca.gov/ for more information.	All phases of project	Qualified Archaeologist and MCWD	
<b>4.6-R9:</b> If historical resources cannot be avoided, they shall be evaluated for CEQA significance and eligibility for the CRHP. The purpose of which would be to define a course of action to satisfy CEQA requirements for an Assessment of Effects. Historical resource mitigation measures may include further study to evaluate the sites, detailed recording, and/or excavation. If the historical resources per CEQA are significant or eligible for the CRHP, then a data recovery program shall be implemented to reduce impacts to less-than-significant levels as required by CEQA Guidelines.	When resources are encountered	Qualified Archaeologist and MCWD	
<b>4.6-R10:</b> Prior to the initiation of construction or ground-disturbing activities adjacent to cultural resources, all construction personnel should be alerted to the possibility of buried cultural remains. This would include prehistoric and/or historic resources. Personnel should be instructed that upon discovery of prehistoric and/or historic resources, no collection is to be undertaken and work in the immediate area of the find should be halted and MCWD be notified.	Prior to construction	Qualified Archaeologist and MCWD	MCWD
EN-1: Construction Equipment Efficiency Plan <sup>12</sup> . MCWD shall contract a qualified professional (i.e., construction planner/energy efficiency expert) to prepare a Construction Equipment Efficiency Plan that identifies the specific measures that MCWD (and its construction contractors) will implement as part of project construction to increase the efficient use of construction equipment. Such measures shall include, but not necessarily be limited to: procedures to ensure that all construction equipment is properly tuned and maintained at all times; a commitment to utilize existing electricity sources where feasible rather than portable diesel-powered generators; consistent compliance with idling restrictions of the state; and identification of procedures (including the use of routing plans for haul trips) that will be followed to ensure that all materials and debris hauling is conducted in a fuel-efficient manner.	Prior to project construction	MCWD energy efficiency expert, construction contractors	MCWD
4.7-R1: To minimize the potential effects from strong seismic ground shaking on the project, a project specific geotechnical analysis shall be performed by a registered professional engineer with geotechnical expertise prior to the development of project level plans. The recommendations of the geotechnical analysis shall be incorporated into project plans and implemented during construction, as appropriate.	Prior to final design	Registered geotechnical engineer	MCWD
<b>4.7-R2:</b> The engineer shall develop project level plans based upon and in response to the observations and recommendations made in the project specific geotechnical analysis.	Prior to final design and after geotech	Design engineer and MCWD	
4.7-R3: MCWD, the contractor and engineer (as appropriate) shall develop emergency response procedures in order to control and stop the release of recycled water in the event that seismic ground shaking causes a leak or rupture in the earthen or tank reservoirs or pipelines.	Prior to project completion	MCWD, engineer, contractor, as appropriate	MCWD
HH-2a: Environmental Site Assessment <sup>13</sup> . If required by local jurisdictions and property owners with approval responsibility for construction, MCWD shall conduct a Phase I Environmental Site Assessment in conformance with ASTM Standard 1527-05 to identify potential locations where hazardous material contamination may be encountered. If an Environmental Site Assessment indicates that a release of hazardous materials could have affected soil or groundwater quality at a project site, a Phase II environmental site assessment shall be conducted to determine the extent of contamination and to prescribe an appropriate course of remediation, including but not limited to removal of contaminated soils, in conformance with state and local guidelines and regulations. If the results of the subsurface investigation(s) indicate the presence of hazardous materials, additional site remediation may be required by the applicable state or local regulatory agencies, and the contractors shall be required to comply with all regulatory requirements for facility design or site remediation.	Prior to project construction (if presence of hazardous materials is identified, site remediation or design changes	MCWD project engineers, construction contractors	MCWD

Page 8 of 13

<sup>12</sup> Mitigation Measure EN-1 was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance, The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015.

<sup>13</sup> Mitigation Measure HH-2a was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance, The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015. Denise Duffy & Associates, Inc.

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Timing of Responsibility Verified for **RUWAP EIR Mitigation Measure** with text edits to apply specifically to the RWP shown in strikeout for deleted text and underline for added text. Implefor Compliance mentation **Implementation** by: required) HH-2b: Health and Safety Plan<sup>14</sup>. The construction contractor(s) shall prepare and implement a project-specific Health and Safety Plan (HSP) for each site on which construction may occur, in accordance with 29 CFR 1910 to protect construction workers and the public during all excavation, grading, and construction. The HSP shall include the following, at a minimum: MCWD A summary of all potential risks to construction workers and the maximum exposure limits for all known and reasonably foreseeable site chemicals (the HSP shall incorporate and consider the information in all available existing Environmental Site Assessments and remediation reports for properties within 1/4-mile using the EnviroStor Database); Monterey Prior to project Construction County Specified personal protective equipment and decontamination procedures, if needed; construction contactors Dept. of • Emergency procedures, including route to the nearest hospital; Environme Procedures to be followed in the event that evidence of potential soil or groundwater contamination (such as soil staining, noxious odors, debris or buried storage containers) is encountered. These ntal Health procedures shall be in accordance with hazardous waste operations regulations and specifically include, but are not limited to, the following: immediately stopping work in the vicinity of the unknown hazardous materials release, notifying Monterey County Department of Environmental Health, and retaining a qualified environmental firm to perform sampling and remediation; and The identification and responsibilities of a site health and safety supervisor. HH-2c: Materials and Dewatering Disposal Plan<sup>15</sup>. MCWD and/or their contractors shall develop a materials disposal plan specifying how the contractor will remove, handle, transport, and dispose of all excavated material in a safe, appropriate, and lawful manner. The plan must identify the disposal method for soil and the approved disposal site, and include written documentation that the disposal site will accept the waste. The contractor shall develop a groundwater dewatering control and disposal plan specifying how the contractor will remove, handle, and dispose of groundwater impacted by hazardous substances in Prior to and Contractor and a safe, appropriate, and lawful manner. The plan must identify the locations at which potential contaminated groundwater dewatering are likely to be encountered (if any), the method to analyze during project **MCWD** MCWD groundwater for hazardous materials, and the appropriate treatment and/or disposal methods. If the dewatering effluent contains contaminants that exceed the requirements of the General WDRs for construction Discharges with a Low Threat to Water Quality (Order No. R3-2011-0223, NPDES Permit No. CAG993001), the construction contractor shall contain the dewatering effluent in a portable holding

tank for appropriate offsite disposal or discharge. The contractor can either dispose of the contaminated effluent at a permitted waste management facility or discharge the effluent, under permit, to the

Regional Treatment Plant.

<sup>&</sup>lt;sup>14</sup> Mitigation Measure HH-2b was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance, The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015.

<sup>15</sup> Mitigation Measure HH-2c was identified in the Final Pure Water Monterey Groundwater Replenishment Project MMRP as mitigation necessary for the construction and project implementation of the RWP and therefore has been added to this MMRP to ensure compliance. The Pure Water Monterey Groundwater Replenishment Project Final EIR and MMRP approved and certified by Monterey Regional Water Pollution Control Agency (MRWPCA) on October 8, 2015. Denise Duffy & Associates, Inc. Page 9 of 13

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proponent shall replace "MCWD" with their name each time it occurs prior to implementation of those project components.	TD: • 6	D 11 1114	¥7 +6* 1.6	T = 7
<b>RUWAP EIR Mitigation Measure</b> with text edits to apply specifically to the RWP shown in strikeout for deleted text and underline for added text.	Timing of Imple- mentation	Responsibility for Implementation	Verified for Compliance by:	X
<b>4.8-R1:</b> The MCWD shall require review of construction plans for the pipeline by the Fort Ord BRAC office to confirm that construction is planned in cleared areas cleared of Military Munitions	Prior and during	MCWD and	MCWD	
(MM) before construction is initiated. An Army-approved MM monitor shall be present during grading in areas where excavation exceeds two feet and any MM encountered shall be properly	to construction	Contractors		
managed. Access shall be restricted to adjacent areas by means of temporary fencing and signage.				
4.8-R2: For areas recommended or required by Army's BRAC Fort Ord (see EPA Superfund Record of Decision; EPA ID CA7210020676, dated 4/6/05), the MCWD shall require that all pipeline	Prior and during	MCWD and	MCWD	
construction workers receive an Army OE MM safety briefing from the BRAC Fort Ord office prior to starting construction and, as needed thereafter. In the event OE MM is suspected or discovered,	to construction	Contractors		
the following actions shall be taken:				
• MCWD and their contractors shall immediately suspend actions which may affect the item,				
• the item shall not be touch or disturbed, work shall be stopped immediately,				
• the location shall be clearly marked, all personnel evacuated, and				
• the local law enforcement agency (Presidio of Monterey (POM) Police or applicable City Police Department) shall be contacted immediately for further investigation. Upon notification, the				
police shall secure the area and make arrangements to have the item identified and destroyed.				L
<b>4.11-R1:</b> The construction contractor shall limit exterior construction related activities to the hours of restriction consistent with the noise ordinance of, and encroachment permits issued, by the	Prior to	MCWD and	MCWD	
relevant land use jurisdictions between 7:00 a.m. and 7:00 p.m. on weekdays and Saturdays, and between 10:00 a.m. and 7:00 p.m. on Sundays and holidays. If alternative traffic control measures are	construction	Contractors		
unavailable and if approved by staff of the relevant City identified below through their encroachment permit, nighttime construction may be conducted for the following segments of road (as				
identified in the Higgins' Associates letter dated October 17, 2006) provided that sensitive receptors (in this case, residences, nursing homes, and hotels/motels) are located an adequate distance from				
construction activities (as determined by the relevant land use jurisdiction):				
• Reservation Road between Seacrest Avenue and Crescent Avenue [Marina - preferred alignment]				
• Fremont Street between Kimball Avenue and Airport Boulevard [Seaside – preferred alignment]				
• Del Monte Avenue between Park Avenue and Camino Aguajito [Monterey – alternative alignment]				
<ul> <li>Del Monte Avenue between Camino Aguajito and Figueroa Street [Monterey – preferred alignment]</li> </ul>				
(Please note that mitigation measure 4.11-R1 is consistent with mitigation measure NV-1d from Final Pure Water Monterey Groundwater Replenishment Project MMRP).				
<b>4.11-R2:</b> The contractor shall locate all stationary noise-generating equipment as far as possible from nearby noise-sensitive receptors. Where possible, noise-generating equipment shall be shielded	During	Contractor	MCWD	
from nearby noise-sensitive receptors by the use of noise-attenuating buffers. Stationary noise sources located 500 feet from noise-sensitive receptors shall be equipped with noise reducing engine	construction			
housings. Portable acoustic barriers shall be placed around noise-generating equipment that is located less than 200 feet from noise-sensitive receptors.				
(Please note that mitigation measure 4.11-R2 is consistent with mitigation measure NV-1d from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	ъ.	Q	MOND	₩
4.11-R3: The contractor shall assure that construction equipment powered by gasoline or diesel engines have sound control devices at least as effective as those provided by the original equipment	During	Contractor	MCWD	
manufacturer (OEM). No equipment shall be permitted to have an un-muffled exhaust.	construction			
(Please note that mitigation measure 4.11-R3 is consistent with mitigation measure NV-1d from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	D : : .	Q:		╄
NV-2b: Construction Hours. The construction contractor shall limit all noise-producing construction activities within the City of Marina to between the hours of 7:00 AM and 7:00 PM on weekdays	During project	Construction	MCWD	
and between 9:00 AM and 7:00 PM Saturdays.	construction	contractor	MOND	╄
<b>4.11-R4:</b> The contractor shall assure that noise-generating mobile equipment and machinery are shut-off when not in use. (Please note that mitigation measure 4.11-R4 is consistent with mitigation measure NV-1d from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	During	Contractor	MCWD	
	construction	MCWD and	MCWD	₩
4.11-R5: Residences within 500 feet of a construction area shall be notified of the construction schedule in writing, prior to construction. The Project Applicant MCWD and contractor shall design the applicant of the construction area shall be notified of the construction schedule in writing, prior to construction. The project Applicant MCWD and contractor shall design the construction area shall be notified of the construction schedule in writing, prior to construction. The project Applicant MCWD and contractor shall design the construction area shall be notified of the construction schedule in writing, prior to construction.	Prior to and		MCWD	
designate a noise disturbance coordinator who would be responsible for responding to complaints regarding construction noise. The coordinator shall determine the cause of the complaint and ensure that reasonable measures are implemented to correct the problem. A contact number for the noise disturbance coordinator shall be conspicuously placed on construction site fences and written into	during	Contractor		
the construction notification schedule sent to nearby residences.	construction			
NV-2a: Construction Equipment. Contractor specifications shall include a requirement that the contractor shall:				H
<ul> <li>Assure that construction equipment with internal combustion engines has sound control devices at least as effective as those provided by the original equipment manufacturer. No equipment shall be permitted to have an un-muffled exhaust.</li> </ul>				
• Impact tools (i.e., jack hammers, pavement breakers, and rock drills) used for project construction shall be hydraulically or electrically powered wherever possible to avoid noise associated with compressed air exhaust from pneumatically powered tools. Where use of pneumatic tools is unavoidable, an exhaust muffler shall be placed on the compressed air exhaust to lower noise levels by approximately 10 dBA. External jackets shall be used on impact tools, where feasible, in order to achieve a further reduction of 5 dBA. Quieter procedures shall be used, such as drills rather than impact equipment, whenever feasible.	During project construction	Contractor and MCWD	MCWD	
• The construction contractor(s) shall locate stationary noise sources (e.g., generators, air compressors) as far from nearby noise-sensitive receptors as possible.				
- The constitution conductor(s) shall focute stationary noise sources (e.g., generators, an compressors) as fair from hearby noise-sensitive receptors as possible.			1	1

For Product Water Conveyance pipeline segments within the City of Marina, noise controls shall be sufficient to not exceed 60 decibels for more than twenty-five percent of an hour.

NOTES: Section 21081.6 of the Public Resources Code requires all state and local agencies to establish monitoring or reporting programs whenever approval of a project relies upon an environmental impact report (EIR). The purpose of the monitoring or reporting program is to ensure implementation of the measures being imposed to mitigate or avoid the significant adverse environmental impacts identified in the EIR as amended in Addendum No. 1 to the certified Final EIR for the MCWD Regional Urban Water Augmentation Project.

For those project features outside of MCWD's service areas (specifically, at the Monterey Regional Water Pollution Control Association, Regional Treatment Plant and within the Monterey Peninsula/Cal-Am Service Area) the lead agency and/or project proponent shall replace "MCWD" with their name each time it occurs prior to implementation of those project components.

proponent shall replace MCVD with their name each time it occurs prior to implementation of those project components.			
RUWAP EIR Mitigation Measure with text edits to apply specifically to the RWP shown in strikeout for deleted text and underline for added text.	Timing of	Responsibility	Verified for X
	Imple-	for	Compliance
	mentation	Implementation	by:
<b>4.13-R1:</b> During construction, the contractor shall insure that adequate access to open space, park and public areas is made available to the public at all times. If construction activities require	During	Contractor and	MCWD/
temporary closing of an existing entrance or exit, the contractor shall provide an alternate entrance/exit for the duration of construction within the vicinity. The appropriate City or County shall	construction	MCWD	staff at
approve the alternate entrance/exit prior to installation. The contractor shall also provide adequate noticing and/or signage, as directed by the City or County, for public notification and safety.			affected City
			or County
PS-3: Construction Waste Reduction and Recycling Plan. The construction contractor(s) shall prepare and implement a construction waste reduction and recycling plan identifying the types of			
construction debris the Project will generate and the manner in which those waste streams will be handled. In accordance with the California Integrated Waste Management Act of 1989, the plan shall			
emphasize source reduction measures, followed by recycling and composting methods, to ensure that construction and demolition waste generated by the project is managed consistent with applicable	Prior to, during,	C 1	
statutes and regulations. In accordance with the California Green Building Standards Code and local regulations, the plan shall specify that all trees, stumps, rocks, and associated vegetation and soils,	and after project	Contractor and	MCWD
and 50% of all other nonhazardous construction and demolition waste, be diverted from landfill disposal. The plan shall be prepared in coordination with the Monterey Regional Waste Management	construction	MCWD	
District and be consistent with Monterey County's Integrated Waste Management Plan. Upon project completion, MCWD shall collect the receipts from the contractor(s) to document that the waste			
reduction, recycling, and diversion goals have been met.			
4.14-R1: The construction contractor shall prepare traffic control/management management plans for construction of the pipeline within each of the affected jurisdictions including the Cities of	Prior to	Contractor and	MCWD and
Monterey, Seaside and Marina, Monterey County, and Caltrans as appropriate. These traffic control plans shall be reviewed and approved by the affected public agency prior to the commencement of	construction	MCWD	staff at
work and an encroachment permit obtained based upon the traffic control plan(s) or other information prepared by a qualified traffic engineer. The traffic control/management plan shall specify the	within each		affected City
times during which construction activities would occur and times when travel lanes cannot be blocked (e.g., peak traffic periods as directed by the affected City Engineer). The plans shall provide	jurisdiction		or County
details regarding the placement of traffic control and warning devices, detours, and that the trench must be covered and/or plated during times of non-construction.	v		
(Please note that mitigation measure 4.14-R1 is consistent with mitigation measure TR-2 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).			
4.14-R2: The traffic control/management plan must include a program that provides continual coordination program with the affected Agencies to allow for adjustments and refinements to the plan	During	Contractor and	MCWD and
once construction is underway.	construction	MCWD	staff at

(Please note that mitigation measure 4.14-R2 is consistent with mitigation measure TR-2 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).

affected City

or County

within each jurisdiction

NOTES: Section 21081.6 of the Public Resources Code requires all state and local agencies to establish monitoring or reporting programs whenever approval of a project relies upon an environmental impact report (EIR). The purpose of the monitoring or reporting program is to ensure implementation of the measures being imposed to mitigate or avoid the significant adverse environmental impacts identified in the EIR as amended in Addendum No. 1 to the certified Final EIR for the MCWD Regional Urban Water Augmentation Project.

For those project features outside of MCWD's service areas (specifically, at the Monterey Regional Water Pollution Control Association, Regional Treatment Plant and within the Monterey Peninsula/Cal-Am Service Area) the lead agency and/or project proponent shall replace "MCWD" with their name each time it occurs prior to implementation of those project components.

proponent shall replace "MCWD" with their name each time it occurs prior to implementation of those project components.			
RUWAP EIR Mitigation Measure with text edits to apply specifically to the RWP shown in strikeout for deleted text and underline for added text.	Timing of Imple- mentation	Responsibility for Implementation	Verified for Compliance by:
<b>4.14-R3:</b> As a supplement to the traffic control/management plan, the construction contractor shall coordinate with the affected agencies to determine the need for a public information program that would inform area residents, employers, and business owners of the details concerning construction schedules and expected travel delays. The public information program could utilize various media venues (e.g. newspaper, radio, television, telephone hot lines, Internet, etc.) to disseminate information such as: 1) Overview of construction project. 2) Updates on location of construction zone. 3) Identification on street(s) locations anticipated to be affected by construction. 4) Times when construction activities would occur and when traffic delays can be expected. 5) Identification of alternate travel routes that could be used to avoid construction delays.  (Please note that mitigation measure 4.14-R3 is consistent with mitigation measure TR-2 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	Prior to and during construction within each jurisdiction	Contractor and MCWD	MCWD and staff at affected City or County
<b>4.14-R4:</b> During the preparation and implementation of traffic control/management plans, special consideration shall be given to the locations where direct driveway access is being impacted. Measures shall be developed and coordinated with the individual property owners who are affected by project construction to minimize access disruption to their private residences and/or businesses. (Please note that mitigation measure 4.14-R4 is consistent with mitigation measure TR-2 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	During the preparation / implementation of traffic control/manage ment plans	Contractor and MCWD	MCWD
<b>4.14-R5:</b> A component of the traffic control/management plan public information program shall include provisions for informing area residents, major employers, and commercial businesses that access restrictions/disruptions would occur. Additional information shall be prepared to advise the affected public of alternative access routes if local affected agencies determine that such a plan is necessary.  (Please note that mitigation measure 4.14-R5 is consistent with mitigation measure TR-2 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	During the preparation / implementation of traffic control/manage ment plans	Contractor and MCWD	MCWD
<b>4.14-R6:</b> The construction contractor shall coordinate with MST to identify routes affected by the pipeline construction. It is suggested that MST post notices at bus stops and on buses along affected routes to notify passengers of potential delays or service adjustments on these routes. Sufficient notification as to the exact dates when delays can be expected or service adjustments would be necessary would be given to MST to allow for timely posting of these notices. (Please note that mitigation measure 4.14-R6 is consistent with mitigation measure TR-2 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	During construction along MST routes	Contractor and MCWD	MST
<b>4.14-R7:</b> Traffic control/management plans which need to be prepared for the affected jurisdictions or agencies shall identify all bus stops in the immediate vicinity of construction zones and shall make provisions for these bus stops to remain accessible throughout the duration of the localized construction impact. In cases where the blockage of existing bus stops cannot be avoided the construction contractor shall coordinate with MST to provide temporary bus stop locations.  (Please note that mitigation measure 4.14-R7 is consistent with mitigation measure TR-2 from Final Pure Water Monterey Groundwater Replenishment Project MMRP).	During construction along MST routes	Contractor and MCWD	MST
<b>TR-3: Roadway Rehabilitation Program.</b> Prior to commencing project construction, MCWD shall detail the preconstruction condition of all local construction access and haul routes proposed for substantial use by project-related construction vehicles. The construction routes surveyed must be consistent with those identified in the construction traffic control and safety assurance plan developed under Mitigation Measure TR-2. After construction is completed, the same roads shall be surveyed again to determine whether excessive wear and tear or construction damage has occurred. Roads damaged by project-related construction vehicles shall be repaired to a structural condition equal to, or greater than, that which existed prior to construction activities. In the City of Marina, the construction in the city rights-way must comply with the City's design standards, including restoration of the streets from curb to curb, as applicable. In the City of Monterey, asphalt pavement of full travel lanes will be resurfaced without seams along wheel or bike paths.	Prior to project construction, after project construction	MCWD construction contractors	MCWD, and local jurisdictions
<b>TR-4:</b> Construction Parking Requirements. Prior to commencing project construction, the construction contractor(s) shall coordinate with the potentially affected jurisdictions to identify designated worker parking areas that would avoid or minimize parking displacement in congested areas of Marina, and Seaside. The contractors shall provide transport between the designated parking location and the construction work areas. The construction contractor(s) shall also provide incentives for workers that carpool or take public transportation to the construction work areas. The engineering and construction design plans shall specify that contractors limit time of construction within travel lanes and public parking spaces and provide information to the public about locations of alternative spaces to reduce parking disruptions.	Prior to project construction	MCWD construction contractor	MCWD, City of Marina, City of Seaside,
CUM-R2: Conduct pre-construction and post-construction biological surveys for special-status plant and wildlife species and their habitat for projects affecting undeveloped dune-habitat, compensate for losses, and conduct construction monitoring. Each project proponent for other projects that would contribute to this cumulative impact (see Table 5.3-1) will retain a qualified botanist to conduct pre-construction and post construction surveys for Hickman's onion to quantify the number of plants and size of the population removed by construction and to determine appropriate habitat compensation. The project proponent will compensate for habitat loss related to dune habitats by contributing to the habitat restoration and enhancement program implemented by the California Department of Parks and Recreation at the Marina State Beach. Each project proponent MCWD will retain a qualified biologist to conduct pre-construction and post-construction surveys for burrowing owl, loggerhead shrike, California horned lark, California horned lizard, black legless lizards, and raptors to determine whether species are present. The project proponent MCWD will implement the recommendations of the biologist. Recommendations could include relocating the species, altering the construction schedule to avoid breeding season, educating construction workers, and monitoring construction activities. These measures are described in more detail in Chapter 4.4 (see Mitigation Measures 4.4-R1, through 4.4-R23).	Prior to, during and after construction	Qualified Biologist and MCWD	MCWD
CUM-R3: MCWD and/or MRWPCA shall coordinate with Relevant Local Agencies to Develop and Implement a Phased Construction Plan to Reduce Cumulative Traffic, and Noise Impacts. The MCWD and/or MRWPCA will contact local agencies that have projects planned in the same area (i.e., project sites within 1 mile or projects that affect the same roadways) and that have construction	Prior to construction	Contractor and MCWD	MCWD and staff at

NOTES: Section 21081.6 of the Public Resources Code requires all state and local agencies to establish monitoring or reporting programs whenever approval of a project relies upon an environmental impact report (EIR). The purpose of the monitoring or reporting program is to ensure implementation of the measures being imposed to mitigate or avoid the significant adverse environmental impacts identified in the EIR as amended in Addendum No. 1 to the certified Final EIR for the MCWD Regional Urban Water Augmentation Project.

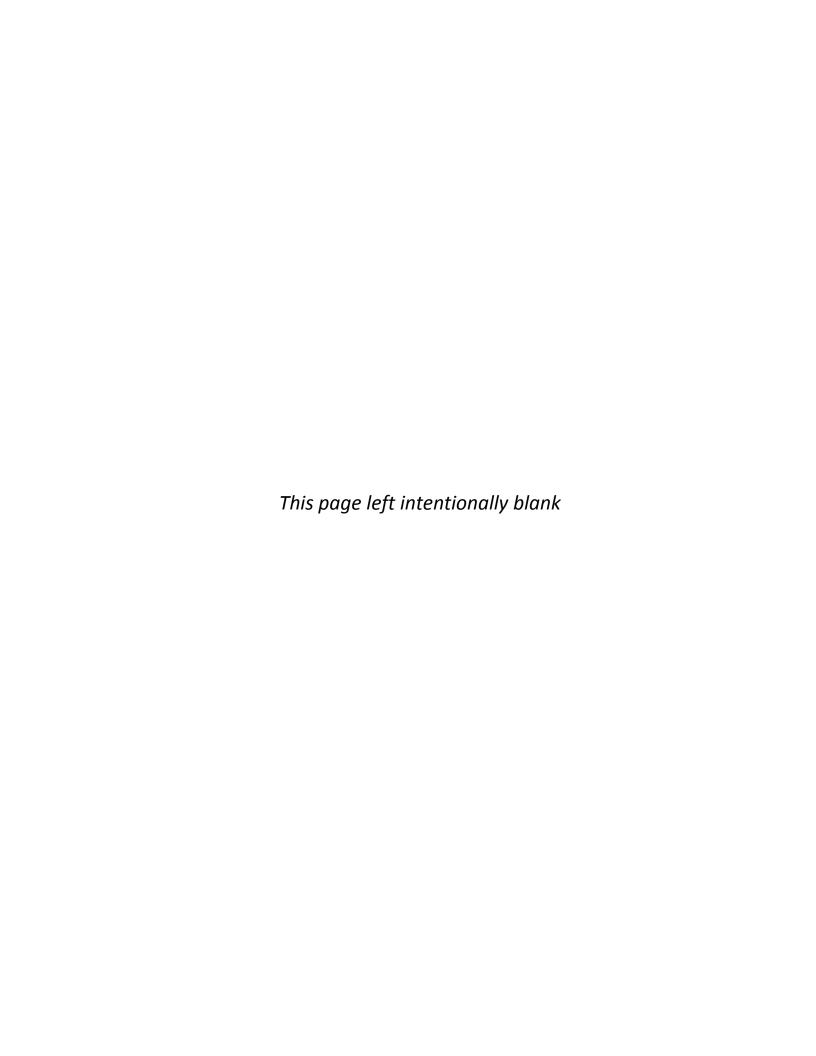
For those project features outside of MCWD's service areas (specifically, at the Monterey Regional Water Pollution Control Association, Regional Treatment Plant and within the Monterey Peninsula/Cal-Am Service Area) the lead agency and/or project proponent shall replace "MCWD" with their name each time it occurs prior to implementation of those project components.

<b>RUWAP EIR Mitigation Measure</b> with text edits to apply specifically to the RWP shown in strikeout for deleted text and underline for added text.	Timing of	Responsibility	Verified for	X
g	Imple-	for	Compliance	
	mentation	Implementation	by:	
schedules that overlap with construction of the Recycled Water Alternative. MCWD (or their contractor) will coordinate with local agencies responsible for said projects to develop a phased	within each		affected City	
construction plan that includes the following components.	jurisdiction		or County	
• Evaluate roadways affected by construction activities and minimize roadway and traffic disturbance (e.g., lane closures and detours) and the number of construction vehicles using the roadways.				
This may involve scheduling some construction activities simultaneously or phasing.				
• Prepare compatible traffic control plans for construction projects. If one traffic control plan cannot be prepared, the construction contractor for the Recycled Water Alternative and the relevant local				
agencies (or their construction contractors) will ensure that the traffic control plans for projects affecting the same roadways are compatible. The traffic control plan can be modeled after that required				
for the Recycled Water Alternative (refer to Mitigation 4.14-R1 through 4.14-R3).				
• Implement noise reductions measures for each project with overlapping construction timeframes. These measures, which are described in more detail in Section 4.11, include: limiting hours of				

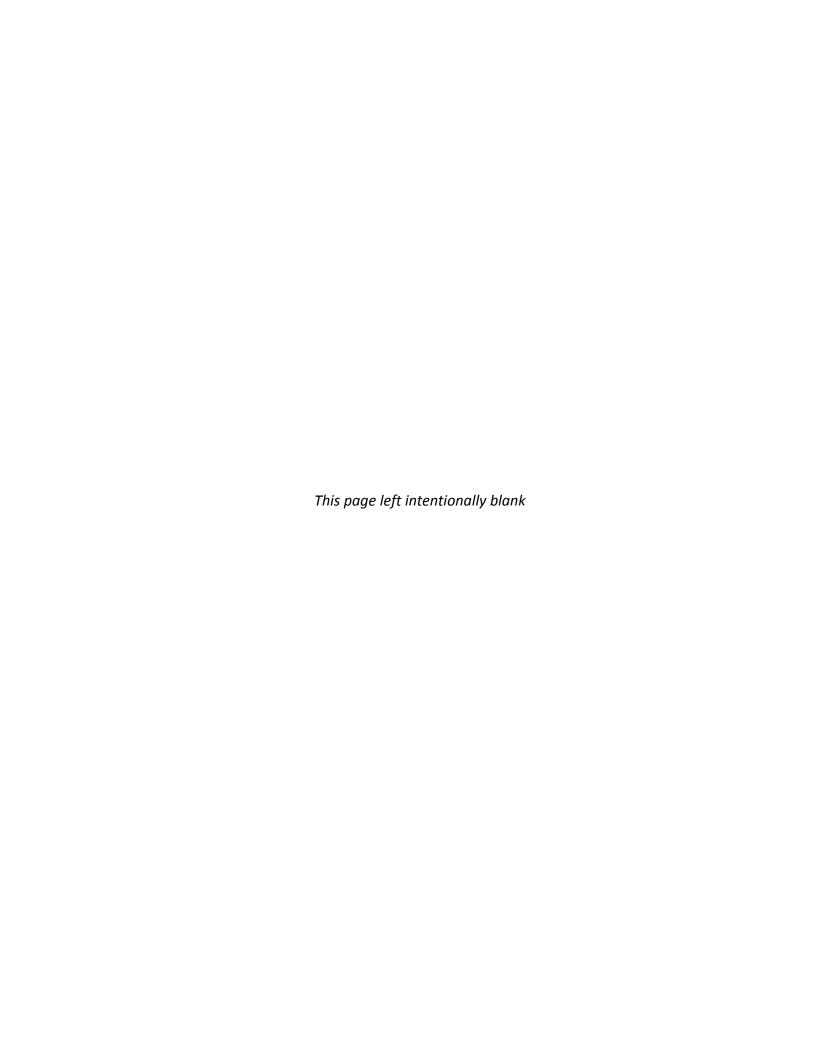
#### NOTES

**Note 1**: A preliminary archaeological survey for the project Areas of Potential Effects will be completed in October 2006. At this time, no resources have been identified in or near the Ord Community and Central Marina segments of the project. The portion of the pipeline within the City of Monterey has been revised to avoid impacts to the cultural resources identified in and near the alignment proposed by the RURWDP and RUWAP. It is preferred the impacts to cultural resources be avoided wherever possible and mitigated where avoidance is not feasible. A survey of the Armstrong Ranch alignment is under way and should be completed in October 2006.

construction activities, employing noise-control construction practices, and implementing a noise control plan (4.11-R1 through 4.11-R5).



Appendix C:
Trussell Tech September 2017 Ocean Plan Compliance Assessment for the PWM/GWR Project



# Ocean Plan Compliance Assessment for the Pure Water Monterey Groundwater Replenishment Project

**Technical Memorandum** 

September 2017

# Prepared for:





# Ocean Plan Compliance Assessment for the Pure Water Monterey Groundwater Replenishment Project

# **Technical Memorandum**



Prepared By:

Trussell Technologies, Inc.

Brie Webber, P.E. Elaine Howe, P.E. (NM) John Kenny, P.E. Rhodes Trussell, Ph.D., P.E., BCEE



# **Table of Contents**

1 Executive Summary	
2 Introduction	3
2.1 Treatment through the RTP and AWPF	3
2.2 California Ocean Plan	5
2.3 Objective of Technical Memorandum	5
3 Methodology for Ocean Plan Compliance Assessment	5
3.1 Methodology for Determination of Discharge Water Qu	ıality6
3.1.1 Future Secondary Effluent	8
3.1.2 GWR RO Concentrate	11
3.1.3 Hauled Waste	
3.1.4 Combined Ocean Discharge Concentrations	12
3.2 Ocean Modeling and Ocean Plan Compliance Analysis N	Methodology12
4 Ocean Plan Compliance Results	
4.1 Water Quality of Combined Discharge	14
4.2 Ocean Modeling Results	
4.3 Ocean Plan Compliance Results	18
4.4 Toxicity	24
5 Conclusions	24
6 References	25

# **1 Executive Summary**

Monterey Regional Water Pollution Control Agency (MRWPCA) and the Monterey Peninsula Water Management District ("Project Partners") are implementing the Pure Water Monterey Groundwater Replenishment Project ("Project"). The Project involves treating secondary effluent from MRWPCA's Regional Treatment Plant (RTP) through the proposed Advanced Water Purification Facility (AWPF) and then injecting this highly purified recycled water into the Seaside Groundwater Basin, with subsequent withdrawal for use as a municipal water supply. The Project will also provide additional tertiary recycled water for agricultural irrigation in the northern Salinas Valley as part of the Castroville Seawater Intrusion Project (CSIP). A waste stream, the reverse osmosis concentrate ("RO concentrate"), will be generated by the AWPF and discharged through the existing MRWPCA ocean outfall, which currently discharges secondary effluent from the RTP. The goal of this technical memorandum is to analyze whether discharge of the Project's RO concentrate to the Pacific Ocean (Monterey Bay) through the existing outfall would comply with numeric water quality objectives in the California Ocean Plan to protect marine aquatic life and human health.

The California Ocean Plan sets forth numeric and narrative water quality objectives for ocean waters with the intent of protecting the ocean's beneficial uses, which include recreation, aesthetics, navigation, fishing, mariculture, areas of special biological significance, rare and endangered species, habitat, fish migration, fish spawning, and shellfish harvesting (SWRCB, 2015). For typical wastewater discharges, when released from an outfall, the wastewater and ocean water undergo rapid mixing due to the momentum and buoyancy of the discharge. The mixing that occurs in the rising plume is affected by the buoyancy and momentum of the discharge, a process referred to as initial dilution (NRC, 1993). The numeric Ocean Plan objectives are to be met after the initial dilution of the discharge into the ocean. The initial dilution occurs in an area known as the zone of initial dilution (ZID), and the Ocean Plan objectives are to be met at the edge of the ZID. The extent of dilution in the ZID is quantified as the minimum probable initial dilution ( $D_m$ ). The water quality objectives established in the Ocean Plan are adjusted by the  $D_m$  to derive NPDES permit limits that are applied to a wastewater discharge prior to ocean dilution.

Trussell Technologies, Inc. (Trussell Tech) estimated worst-case in-pipe discharge water quality (*i.e.*, prior to being discharged through the outfall and diluted in the ocean) for the Project and used the dilution modeling results determined by Dr. Philip Roberts to provide an assessment of whether the Project would consistently meet Ocean Plan water quality objectives. The resulting concentrations for each constituent in each scenario were compared to its minimum Ocean Plan objective to assess compliance. The estimated concentrations for eight different flow scenarios are presented in the following technical memorandum (TM) (Tables 3 and 4). None of the constituents are expected to exceed their Ocean Plan objective<sup>1</sup>. Ammonia is estimated to reach a concentration closest to its minimum objective, with the highest estimated concentration at the edge of the ZID at 71% of the objective.

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<sup>&</sup>lt;sup>1</sup> Aldrin, benzidine, 3,3-dichlorobenzidine and heptachlor were not detected in any source waters, however their MRLs are greater than the Ocean Plan objective. Therefore, no percentages are presented Table 4 as no compliance conclusions can be drawn for these constituents. This is a common occurrence for ocean discharges since the MRL is higher than the Ocean Plan objective for some constituents.

The purpose of the analysis documented in this TM was to assess the ability of the Project to comply with the Ocean Plan objectives. Trussell Tech used a conservative approach to estimate the water qualities of the RTP secondary effluent, RO concentrate, and hauled waste (blended with secondary effluent) for the Project. These water quality data were then combined for various discharge scenarios, and a concentration at the edge of the ZID was calculated for each constituent and discharge scenario. Compliance assessments could not be made for selected constituents due to analytical limitations, but this is a common occurrence for these Ocean Plan constituents. Based on the data, assumptions, modeling, and analytical methodology presented in this technical memorandum, the Project will comply with all numeric Ocean Plan objectives.

#### 2 Introduction

Monterey Regional Water Pollution Control Agency (MRWPCA) and the Monterey Peninsula Water Management District ("Project Partners") are in the process of implementing the Pure Water Monterey Groundwater Replenishment Project ("Project"). The Project involves treating secondary effluent from MRWPCA's Regional Treatment Plant (RTP) through the proposed Advanced Water Purification Facility (AWPF) and then injecting this highly purified recycled water into the Seaside Groundwater Basin, with subsequent withdrawal for use as a municipal water supply. The Project will also provide additional tertiary recycled water for agricultural irrigation in the northern Salinas Valley as part of the Castroville Seawater Intrusion Project (CSIP). A waste stream, the reverse osmosis concentrate ("RO concentrate"), will be generated by the AWPF and discharged through the existing MRWPCA ocean outfall, which currently discharges secondary effluent from the RTP. The goal of this technical memorandum is to analyze whether discharge of the Project's RO concentrate to the Pacific Ocean (Monterey Bay) through the existing outfall would comply with numeric water quality objectives in the California Ocean Plan to protect marine aquatic life and human health.

The original version of this document (Trussell Technologies, 2015b) and an addendum report to that document (Trussell Technologies, 2015c) was included in the Project's Consolidated Final Environmental Impact Report (CFEIR). This version has been updated to reflect an increase in capacity of the AWPF to produce more product water and thus more RO concentrate. In addition, new water quality data have been included since the original analysis (including years 2012 – 2017), and the ocean dilution modeling has correspondingly been revised. Further details regarding these updates are included in the following sections.

# 2.1 Treatment through the RTP and AWPF

The existing RTP treatment process includes screening, primary sedimentation, secondary biological treatment through trickling filters (TFs), followed by a solids contactor (*i.e.*, bioflocculation), and then clarification (Figure 1). Much of the secondary effluent undergoes tertiary treatment (coagulation, flocculation, granular media filtration and disinfection) to produce recycled water used for agricultural irrigation. The unused secondary effluent is discharged to the Monterey Bay through an existing ocean outfall. The RTP also accepts trucked brine waste ("hauled waste") for ocean disposal, which is stored in a pond and mixed with secondary effluent prior to being discharged.

The AWPF will include several advanced treatment technologies for purifying the secondary effluent water: ozone (O<sub>3</sub>), membrane filtration (MF), reverse osmosis (RO), an advanced oxidation process (AOP) using ultraviolet light (UV) and hydrogen peroxide, and finished water stabilization. The Project Partners conducted a pilot-scale study of the ozone, MF, and RO processes of the AWPF from December 2013 through July 2014, successfully demonstrating the ability of the various treatment processes to produce highly-purified recycled water that complies with the California Water Recycling Criteria for Indirect Potable Reuse: Groundwater Replenishment – Subsurface Application (Groundwater Replenishment Regulations) (SWRCB, 2014) and Central Coast Water Quality Control Plan (Basin Plan) standards, objectives and guidelines for groundwater (CCWQCB, 2011). After the pilot-scale study, an advanced water purification demonstration facility was built to gain additional experience operating ozone, MF, and RO processes; the new facility also includes a UV/hydrogen peroxide AOP and stabilization treatment. The demonstration facility is operated and maintained by MRWPCA.

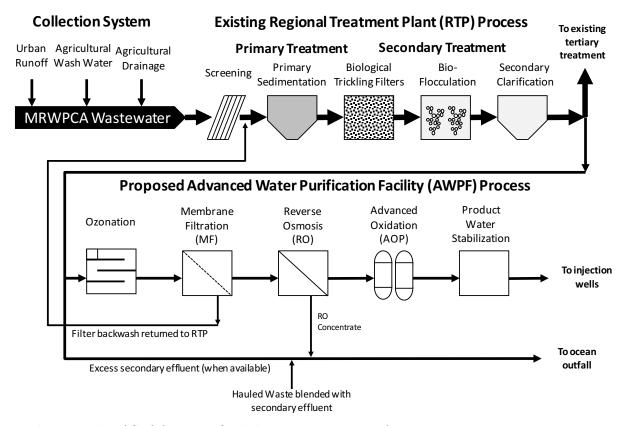


Figure 1 - Simplified diagram of existing MRWPCA RTP and Future AWPF treatment processes

Reverse osmosis is an excellent removal process, separating out most dissolved constituents from the recycled water. The dissolved constituents removed through RO are concentrated into a waste stream known as the RO concentrate. Unlike the waste from the MF, the RO concentrate cannot be recycled back to the RTP headworks and would be discharged through the existing ocean outfall. Discharges through the outfall are subject to National Pollution Discharge Elimination System (NPDES) permitting based on requirements specified in the California State Water Resources Control Board 2015 Ocean Plan ("Ocean Plan") (SWRCB, 2015). Monitoring of the RO concentrate was conducted during the Project's pilot-scale study.

#### 2.2 California Ocean Plan

The California Ocean Plan sets forth numeric and narrative water quality objectives for ocean waters with the intent of protecting the ocean's beneficial uses, which include recreation, aesthetics, navigation, fishing, mariculture, areas of special biological significance, rare and endangered species, habitat, fish migration, fish spawning, and shellfish harvesting (SWRCB, 2015). For typical wastewater discharges, when released from an outfall, the wastewater and ocean water undergo rapid mixing due to the momentum and buoyancy of the discharge. The mixing that occurs in the rising plume is affected by the buoyancy and momentum of the discharge, a process referred to as initial dilution (NRC, 1993). The numeric Ocean Plan objectives are to be met after the initial dilution of the discharge into the ocean. The initial dilution occurs in an area known as the zone of initial dilution (ZID), and the Ocean Plan objectives are to be met at the edge of the ZID. The extent of dilution in the ZID is quantified as the minimum probable initial dilution ( $D_m$ ). The water quality objectives established in the Ocean Plan are adjusted by the  $D_m$  to derive NPDES permit limits that are applied to a wastewater discharge prior to ocean dilution.

The current RTP wastewater discharge is governed by Order No. R3-2014-0013 (NPDES permit No. CA0048551) issued by the Central Coast Regional Water Quality Control Board (RWQCB). Because the current NPDES permit for the existing ocean outfall must be amended to include RO concentrate in the waste discharge, comparing future discharge concentrations to current NPDES permit limits would not be an appropriate metric or threshold for determining whether the Project would have a significant impact on marine water quality. Instead, compliance with the Ocean Plan objectives was selected as an appropriate threshold for determining whether the Project would result in a significant impact requiring mitigation. Dilution modeling of the Project's ocean discharge was conducted by Dr. Philip Roberts

, a Professor in the School of Civil and Environmental Engineering at the Georgia Institute of Technology, to determine  $D_m$  values for the various discharge scenarios at different ambient ocean conditions. The dilution modeling results were combined with projected discharge water quality to assess compliance with the Ocean Plan.

#### 2.3 Objective of Technical Memorandum

Trussell Technologies, Inc. (Trussell Tech) estimated worst-case in-pipe discharge water quality (*i.e.*, prior to being discharged through the outfall and diluted in the ocean) for the Project and used the dilution modeling results determined by Dr. Roberts to provide an assessment of whether the Project would consistently meet Ocean Plan water quality objectives. The purpose of this technical memorandum (TM) is to summarize the assumptions, methodology, results and conclusions of the Ocean Plan compliance assessment.

# 3 Methodology for Ocean Plan Compliance Assessment

To analyze impacts due to ocean discharge of RO concentrate, the Project technical team (Trussell Tech with MRWPCA staff) conducted a thorough water quality and flow characterization of the current secondary effluent and the new sources of water to be diverted

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<sup>&</sup>lt;sup>2</sup> Municipal wastewater effluent, being low in salinity, is less dense than seawater and thus rises (due to buoyancy) while it mixes with ocean water.

into the wastewater collection system. After primary and secondary treatment, this effluent will be used as influent to the AWPF. The team collected all available water quality data for secondary effluent and water quality monitoring results for the Project's new source waters through a one-year monitoring program conducted from July 2013 to June 2014. The new source waters included in the monitoring program were agricultural wash water, and waters from the Blanco Drain, Lake El Estero, and Tembladero Slough. Regular monthly and quarterly sampling was carried out for the RTP secondary effluent, agricultural wash water, and Blanco Drain drainage water. Limited sampling of stormwater from Lake El Estero was performed due to seasonal availability, and there was one sampling event for the Tembladero Slough drainage water. Additional data from routine monitoring of the Reclamation Ditch and Salinas Urban Stormwater Runoff was also incorporated into the analysis (for years 2012 to 2017).

Lake El Estero and the Tembladero Slough are no longer included as new source waters for the Project, and so the monitoring data for those source waters were not included in this analysis. For the Reclamation Ditch, water quality data related to the Ocean Plan were only available for ammonia, copper, zinc, arsenic, cadmium, lead, nickel, and total phenols. For the remaining constituents identified in the Ocean Plan, the concentrations in the Reclamation Ditch waters were conservatively assumed to be the higher of either the Blanco Drain or Tembladero Slough concentrations

Using the full suite of data, the team estimated the future worst-case water quality of the combined ocean discharge. With the results of dilution modeling, concentrations at the edge of the ZID were estimated to determine the ability of the Project to comply with the Ocean Plan objectives. The purpose of this section is to outline the methodology used to make this determination. A summary of the methodology is presented in Figure 2.

#### 3.1 Methodology for Determination of Discharge Water Quality

Water quality data for three types of discharge waters were used to estimate the future combined water quality in the ocean outfall discharge under Project conditions: (1) the RTP secondary effluent, (2) hauled waste (discussed in Section 3.1.3), and (3) the Project RO concentrate. First, Trussell Tech estimated the potential influence of the new source waters (*e.g.*, agricultural wash water, stormwater and agricultural drainage waters) on the worst-case water quality for each of the three types of discharge water. The volumetric contribution of each new source water will change under the different flow scenarios that can occur under the Project. MRWPCA staff worked with Schaaf and Wheeler consultants to estimate the available volume of source waters for each month of the different types of operational years for the Project (Andrew Sterbenz, Schaaf and Wheeler, June 05, 2017). The monthly flows for each source water were estimated for three types of operational years: (1) wet/normal years where a drought reserve is being built, (2) wet/normal years where the drought reserve has been met, and (3) a drought year. All the different flow scenarios were considered in developing the assumed worst-case concentrations

for the Ocean Plan constituents in the secondary effluent. This conservative approach used the highest observed concentrations from all data sources for each source water in the analysis<sup>3</sup>.

Cyanide has been detected in the RTP effluent and other new source waters (Agricultural Wash Water and the Blanco Drain) at relatively high levels compared to the discharge requirements. The maximum detected value in the RTP effluent was 81  $\mu$ g/L; the maximum seen in the Agricultural Wash Water and the Blanco Drain was 89  $\mu$ g/L and 127  $\mu$ g/L, respectively.

Several investigations have been conducted into the accuracy of sampling, preservation, and analytical methods for cyanide. These have shown that sample holding time and preservation have a significant impact on measured cyanide concentrations. Pandit et al. (2006) demonstrated that when sodium hydroxide was added to adjust the pH higher than 12, as specified in accepted methods for cyanide measurement in order to preserve the sample, the measured cyanide concentrations were consistently higher than those for samples preserved at pH 10 to 11. Pandit et al. also showed that cyanide levels increased within the recommended holding times of the approved cyanide methods (at pH 12).

In addition, the 2015 California Ocean Plan specifies the following:

If a discharger can demonstrate to the satisfaction of the Regional Water Board (subject to EPA approval) that an analytical method is available to reliably distinguish between strongly and weakly complexed cyanide, effluent limitations for cyanide may be met by the combined measurement of free cyanide, simple alkali metal cyanides, and weakly complexed organometallic cyanide complexes. In order for the analytical method to be acceptable, the recovery of free cyanide from metal complexes must be comparable to that achieved by the approved method in 40 CFR PART 136, as revised May 14, 1999.

Based on the above information, it is recommended that additional cyanide sampling be conducted using different methods (*e.g.*, analysis within 15 minutes with no preservation) to determine if the current laboratory method leads to inaccurately high cyanide values. It is also recommended to determine if a method can be performed that distinguishes between weakly and strongly complexed cyanide. Until this evaluation is completed, all cyanide concentrations presently available are used in this Ocean Plan compliance assessment.

It was also assumed that no constituent removal occurred through the RTP when considering the new source waters, and so the concentration detected through the source water monitoring program was used to calculate the concentration in the RTP secondary effluent. The exceptions to this statement are dieldrin and DDT. RTP sampling and bench-scale testing were conducted for these constituents to determine removal through the RTP, ozone and MF processes. The minimum removal through the RTP and ozone process was observed to be 91% and 96% for dieldrin and DDT, respectively (Trussell Tech, 2016b). The MF process was observed to remove

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<sup>&</sup>lt;sup>3</sup> The exception to this statement is copper. The median copper concentration was used to estimate the water quality impact of the additional source waters, as the maximum values detected appear to be outliers. Additionally, the minimum Ocean Plan objective for copper is a 6-month median value, and so it is reasonable to use the median value detected from the new source waters to estimate compliance.

a minimum of 97% and 92% for dieldrin and DDT, respectively (Trussell Tech, 2016b). However, the MF system only removes the constituents from the RO concentrate, as the MF backwash water is returned to the RTP headworks.

Once the estimated worst-case water quality was determined for the RTP secondary effluent, these values were used in estimating the worst-case water qualities for the hauled waste and the RO concentrate, as appropriate. The methodology for each type of water is further described in the following sections.

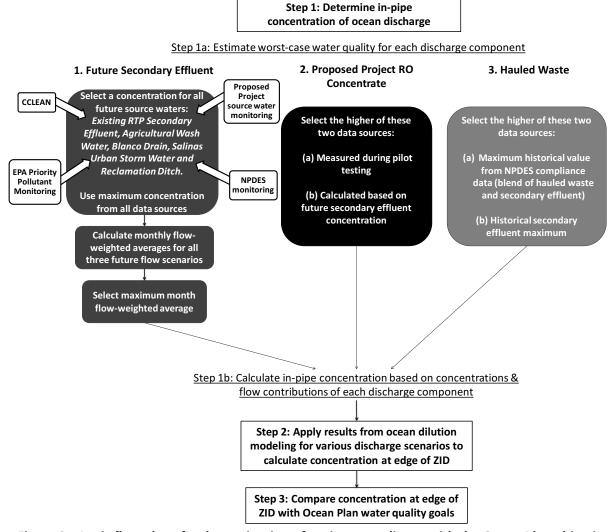


Figure 2 – Logic flow-chart for determination of project compliance with the Ocean Plan objectives

#### 3.1.1 Future Secondary Effluent

The Project involves bringing new source waters into the RTP, and so the water quality of those source waters, as well as the existing secondary effluent, was taken into account to estimate the water quality of the future secondary effluent. Although the new source waters will be brought into the RTP influent, it was assumed that no removal of constituents occurred through the RTP

when calculating the secondary effluent concentration (except dieldrin and DDT, as described in the previous section). The following sources of data were considered for selecting an existing secondary effluent concentration for each constituent in the analysis:

- Source water monitoring conducted for the Project from July 2013 through June 2014
- NPDES storm water discharge monitoring for the City of Salinas (2012 2017) and the Salinas Industrial Ponds (2017)
- RTP historical NPDES compliance data collected semi-annually by MRWPCA (2005–Spring 2017)
- Historical NPDES RTP Priority Pollutant data collected annually by MRWPCA (2004-2016)
- Data collected semi-annually by the Central Coast Long-Term Environmental Assessment Network (CCLEAN) (2008-2016)

The existing secondary effluent concentration for each constituent selected for the analysis was the maximum reported value from the above sources.

Limited data sources were available for several of the new source waters (*i.e.*, agricultural wash water, Blanco Drain, and the Reclamation Ditch). Agricultural wash water and Blanco Drain water quality data was collected during the source water monitoring conducted for the Project. NPDES storm water discharge monitoring for the City of Salinas (2012 – 2017) and Salinas Industrial Ponds monitoring (2017) provided additional data for the Reclamation Ditch and the agricultural wash water. For these new source waters, the maximum observed concentration was selected for Ocean Plan compliance analysis.<sup>4</sup>

Source water flows used for calculation of blended future secondary effluent concentrations were taken from the three projected operational conditions prepared by MRWPCA: (a) normal/wet year, building reserve, (b) normal/wet year, full reserve, and (c) drought year. For each constituent, a total of 36 future concentrations were calculated – 12 months of the year for the three projected future source water flow contributions. Of these concentrations, the maximum monthly flow-weighted concentration was selected for each constituent to be used for the Ocean Plan compliance analysis.

When a constituent could not be quantified or was not detected, it was reported as less than the Method Reporting Limit (<MRL).<sup>5</sup> Because the actual concentration could be any value equal to or less than the MRL, the conservative approach is to use the value of the MRL in the flow-

<sup>&</sup>lt;sup>4</sup> Except for copper, where instead the median was calculated from the data for each new source water because the maximum values detected seemed to be outliers, and the Ocean Plan objective for copper considered in this assessment is the 6-month median concentration.

<sup>&</sup>lt;sup>5</sup> The lowest amount of an analyte in a sample that can be quantitatively determined with stated, acceptable precision and accuracy under stated analytical conditions (*i.e.*, the lower limit of quantitation). Therefore, acceptable quality control and quality assurance procedures are calibrated to the MRL, or lower. To take into account day-to-day fluctuations in instrument sensitivity, analyst performance, and other factors, the MRL is established at three times the Method Detection Limit (or greater). The Method Detection Limit is the minimum concentration of a substance that can be measured and reported with 99% confidence that the analyte concentration is greater than zero. (40 Code of Federal Regulations Section136 Appendix B).

weighting calculations. In some cases, constituents were not detected above the MRL in any of the source waters, so the concentrations for these constituents were reported as ND (<MRL) in this TM. In cases where the analysis of a constituent was detected but was not quantifiable, the results were also reported in this TM as less than the Method Reporting Limit, ND (<MRL). For some non-detected constituents, the MRL exceeds the Ocean Plan objective, and thus no compliance determination could be made.<sup>6</sup>

The following approaches were used for addressing the cases where a constituent was reported as less than the MRL:

- Aggregate constituents with multiple congeners or sub-components: Some Ocean Plan constituents are a combination of multiple congeners or sub-components (*e.g.*, chlordane, PAHs, PCBs, and TCDD equivalents, among others). Per the Ocean Plan, if individual congeners or sub-components are below the MRL, they are assumed to be zero for the purposes of calculating the aggregate parameter.
- Combining different types of waters: The same approach was used for both combining different source waters (*i.e.*, estimating future secondary effluent concentrations based on a flow-weighted average of source water contributions) and for combining the different discharge components (*i.e.*, RTP secondary effluent, hauled waste, and RO concentrate). For each constituent:
  - When all waters had maximum values reported above the MRL: The flowweighted average of the maximum detected concentrations was used when all waters had values reported above the MRL.
  - When some or all waters had maximum values reported as less than the MRL:
    - When the MRL was at least two orders of magnitude greater (i.e., at least 100 times greater) than the highest detected value from the other waters, the waters with maximum concentrations below the MRL were ignored. This case is exclusive to times when CCLEAN data were reported as detections for the RTP secondary effluent, and all the other source waters were below the MRL<sup>7</sup> (i.e., hexachlorobutadiene was detected at a concentration of  $9.0 \times 10^{-6} \,\mu\text{g/L}$  in the secondary effluent via CCLEAN, and the MRL of all other source waters was 0.5 µg/L). The analytical methods used for CCLEAN can detect concentrations many orders of magnitude below the detection limits for traditional methods, and thus to include the MRL value from the other methods would overshadow the CCLEAN data. Additionally, in cases where the traditional analytical method had an MRL greater than the Ocean Plan objective, performing the analysis using the high MRL from the non-CCLEAN methods would result in an inability to make a compliance determination for these constituents.

<sup>&</sup>lt;sup>6</sup> This phenomenon is common in the implementation of the Ocean Plan where for some constituents, suitable analytical methods are not capable of measuring low enough to quantify the minimum toxicologically relevant concentrations. For these constituents, a discharge is considered compliant if the monitoring results are less than the MRL.

<sup>&</sup>lt;sup>7</sup> Specifically, this case applies to endrin, fluoranthene, chlordane, heptachlor epoxide, hexachlorobenzene, hexachlorobutadiene, PCBs, and toxaphene.

When the MRL was less than two orders of magnitude greater (*i.e.*, less than 100 times greater) than the highest detected value from the other waters, the constituents were reported as less than the MRL and were assumed to have a concentration equal to the MRL for the purposes of calculating a flow-weighted average (*i.e.*, mercury was detected in the secondary effluent at a concentration of 0.019 μg/L, but was not detected in any other source waters, where the MRL was 0.2 μg/L).

#### 3.1.2 GWR RO Concentrate

Two potential worst-case estimates of constituent concentrations were available for assessing the Project's RO concentrate:

- Measured in the concentrate during pilot testing
- Calculated from the blended future secondary effluent concentration, using the following treatment assumptions<sup>8</sup>:
  - No removal prior to the RO process (*i.e.*, no removal through the RTP or AWPF ozone or MF), except for dieldrin and DDT
  - o 81% RO recovery (*i.e.*, of the water feeding into the RO system, 81% is product water, also known as permeate, and 19% is the RO concentrate)
  - o Complete rejection of each constituent by the RO membrane (i.e., 100% of the constituent is in the RO concentrate)

The higher of these two values was selected as the final concentration of the RO concentrate for all constituents, except as noted in the Table 1 footnotes.

#### 3.1.3 Hauled Waste

Currently, small volumes of brine are trucked to the RTP and blended with secondary effluent in a brine pond. The blended waste from this pond ("hauled waste") is then discharged along with the secondary effluent bound for ocean discharge (when there is excess secondary effluent to discharge). For the Project, the hauled waste will be discharged with both secondary effluent and RO concentrate (see Figure 1). The point where the hauled waste is added to the ocean discharge water is downstream of the AWPF intake, and thus will not impact the quality of the Project product water or the RO concentrate. Currently, all sampling of the hauled waste takes place after dilution by secondary effluent in the brine pond, so the data represent a mix of secondary effluent and brine water. It is appropriate to use these data for the hauled waste quality since the practice of diluting with secondary effluent will continue in the future. Two potential values were available for the hauled waste constituent concentrations:

- Historical NPDES compliance data collected semi-annually by MRWPCA (2005-Spring 2017) of hauled waste water diluted with existing secondary effluent
- Calculated future secondary effluent constituent concentrations, as previously described.

The higher of these two values was selected for all constituents; because the hauled waste is diluted by secondary effluent prior to discharge, it is also appropriate to use future secondary effluent concentrations to represent the concentration within the hauled waste. Even if a

<sup>&</sup>lt;sup>8</sup> Based on the treatment assumptions, the RO concentrate would equal 5.3 times the AWPF influent (*i.e.*, blended future secondary effluent) concentration.

constituent was not present in the hauled waste, if it was present in the secondary effluent it would be present in the combined discharge.

#### **3.1.4 Combined Ocean Discharge Concentrations**

Having calculated the worst-case future concentrations for each of the three discharge components (i.e., secondary effluent, RO concentrate, blended hauled waste), the combined concentration prior to discharge was determined as a flow-weighted average of the contributions of each of these three discharge components. Depending on drought conditions and water usage for agricultural irrigation, the amount of secondary effluent discharged to the ocean will vary. A range of potential discharge scenarios was considered to encompass the worst-case water quality conditions of the combined discharge, as described in Section 4.2.

# 3.2 Ocean Modeling and Ocean Plan Compliance Analysis Methodology

In order to determine Ocean Plan compliance, Trussell Tech used the following information: (1) the in-pipe concentration (*i.e.*, pre-ocean dilution) of a constituent ( $C_{\text{in-pipe}}$ ) that was calculated as discussed in the previous section, (2) the minimum probable dilution for ocean mixing ( $D_{\text{m}}$ ) for the relevant discharge flow scenarios that was modeled by Dr. Roberts<sup>9</sup> (Roberts, P. J. W, 2017), and (3) the background concentration of the constituent in the ocean ( $C_{\text{Background}}$ ) that is specified in the Ocean Plan's "Table 3." With this information, the concentration at the edge of the zone of initial dilution ( $C_{\text{ZID}}$ ) was calculated using the following equation:

$$C_{\text{ZID}} = \frac{C_{\text{In-pipe}} + D_{\text{m}} * C_{\text{Background}}}{1 + D_{\text{m}}} \tag{1}$$

The  $C_{ZID}$  was then compared to the Ocean Plan objectives<sup>10</sup> in the Ocean Plan's "Table 1" (SWRCB, 2015). As described previously, the in-pipe concentration was estimated as a flow-weighted average of the future secondary effluent, Project RO concentrate, and hauled waste with the concentrations determined as discussed above. The  $D_m$  values for various flow scenarios were determined by modeling. Note that this approach could not be applied for some constituents (*e.g.*, acute toxicity, chronic toxicity, and radioactivity<sup>11</sup>).

 $<sup>^9</sup>$  The Ocean Plan defines  $D_m$  differently than Dr. Roberts. Dr. Roberts provided results defined as  $S = [total \ volume \ of a \ sample]/[volume \ of effluent contained in the sample]. The <math>D_m$  referenced in Equation 1 of the California Ocean Plan is defined as  $D_m = S - 1$ . A value of 1 was subtracted from the dilution estimates provided by Dr. Roberts prior to using Equation 1.

<sup>&</sup>lt;sup>10</sup> Note that the Ocean Plan (see Ocean Plan Table 2) also defines effluent limitations for oil and grease, suspended solids, settable solids, turbidity, and pH. These parameters were not evaluated in this assessment. It is assumed that, if necessary, the pH of the water would be adjusted to be within acceptable limits prior to discharge; the current AWPF design does not include to ability to change the RO concentrate pH because pilot testing and RO performance modeling indicated it was not necessary. Oil and grease, suspended solids, settable solids, and turbidity in the RO concentrate would be significantly lower than the secondary effluent. Prior to the RO treatment, the process flow would be treated by MF, which will reduce these parameters, and the waste stream from the MF will be returned to RTP headworks.

<sup>&</sup>lt;sup>11</sup> Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based on the nature of the constituents. These constituents were measured individually for the RO concentrate, and these individual concentrations would comply with the Ocean Plan objectives (Trussell

Two methods were used when modeling the ocean mixing: (1) the mathematical model  $UM_3$  in the United States Environmental Protection Agency's (EPA's) Visual Plume suite, and (2) the NRFIELD model (for positively buoyant plumes only), also from the EPA's Visual Plume suite (Roberts, P. J. W., 2017). When results were provided from both methods, the  $D_m$  value estimated with the  $UM_3$  model was selected for consistency, such that all dilution results used for this analysis were determined using the same model.

Dr. Roberts documented the dilution modeling assumptions and results in a technical memorandum (Roberts, P. J. W., 2017, Appendix A). Additional analysis assumptions were made as follows:

- Flow: A sensitivity analysis of the relationship between D<sub>m</sub> and flow rate was performed for the various discharge types. The greatest D<sub>m</sub> sensitivity to flow changes was determined to be from variations in the RTP secondary effluent flow. To simplify the analysis, the flow scenarios used in the compliance analysis only considered the maximum flows for the hauled waste and the RO concentrate because these flows result in the lowest D<sub>m</sub>, thus making the analysis conservative. The flows considered for each discharge type are as follows:
  - o **Secondary effluent:** a range of conditions was modeled that reflect realistic future discharge scenarios (minimum flow, moderate flow, and maximum flow).
  - Project RO concentrate: 1.17 million gallons per day (mgd), which would be the resulting RO concentrate flow when the AWPF is producing 5.0 mgd of highly-purified recycled water (corresponding AWPF influent is 6.86 mgd of RTP secondary effluent). Although the AWPF will not be operated at this influent flowrate year-round, this is the highest potential RO concentrate flow and therefore the most conservative assessment.
  - Hauled waste: A sensitivity analysis was conducted to determine the impacts of hauled waste on the modeled D<sub>m</sub> results. It was concluded that neither the flow nor TDS from the addition of hauled waste had a significant impact on the modeled D<sub>m</sub> result, and was therefore excluded when determining the D<sub>m</sub> value. However, the impact of hauled waste on assumed in-pipe water quality was still assessed. A hauled waste flow of 0.03 mgd blended with secondary effluent for a total flow of 0.1 mgd was used for calculating the in-pipe concentrations of each constituent.
- **Total Dissolved Solids (TDS)**: the greatest dilution is achieved when the salinity of the discharge water is lower and the most different from the ambient ocean salinity; therefore, the most conservative TDS will be the highest (*i.e.*, closest to ambient salinity) of:
  - o **Secondary effluent:** 1,100 milligram per liter (mg/L), which is the maximum expected future TDS, taking into account the flow contribution of each source water and the maximum observed TDS value from each source water

Technologies, 2015c and 2016a). Current discharges of the secondary effluent and hauled waste are monitored semiannually for acute toxicity, chronic toxicity, and radioactivity per the existing NPDES permit. See section 4.4.

- o **Project RO concentrate:** 5,800 mg/L, which is the maximum expected future TDS based on the maximum expected future secondary effluent TDS and the RO treatment assumptions listed in the section above (*i.e.* in a drought year).
- Ocean salinity: 33,340 mg/L 33,890 mg/L, depending on the ocean condition
- Temperature:

Secondary effluent: 20°C
Project RO concentrate: 20°C

An additional consideration of the ocean dilution modeling is the variation in ocean conditions throughout the year. Three conditions were modeled for all flow scenarios: Davidson (December to February), Upwelling (March to September), and Oceanic (October to November)<sup>12</sup>. To conservatively demonstrate Ocean Plan compliance, the lowest  $D_m$  from the applicable ocean conditions was used for each flow scenario.

Ocean dilution modeling covered the range of potential operating conditions, and the results showed that Ocean Plan compliance would be achieved when considering all potential secondary effluent flowrates. To simplify the calculation and presentation of these results, representative flowrate ranges were chosen. To select the representative flow scenarios for compliance assessment, the balance between in-pipe dilution and dilution through the outfall was considered. In general, higher secondary effluent flows discharged to the ocean would provide dilution of the Project RO concentrate; however, greater dilution due to ocean water mixing would be provided at lower wastewater discharge flows. The balance of these influences was considered in determining compliance under the eight representative discharge conditions that are described in Section 4.2 for the Project.

# 4 Ocean Plan Compliance Results

#### 4.1 Water Quality of Combined Discharge

As described above, the first step in the Ocean Plan compliance analysis was to estimate the worst-case water quality for each of the three future discharge components: future RTP effluent, Project RO concentrate, and blended hauled waste. A summary of the estimated water qualities of these components is given in Table 1. Additional considerations and assumptions for each constituent are documented in the Table 1 notes section.

Table 1 – Summary of estimated worst-case water quality for the three waste streams that would be discharged through the ocean outfall

Constituent	Units	Secondary Effluent	Hauled Waste	RO Concentrate	Notes				
Ocean Plan water quality objectives for protection of marine aquatic life									
Arsenic	μg/L	45	45	12	1,12				
Cadmium	μg/L	1.2	1.2	6.5	2,11				
Chromium (Hexavalent)	μg/L	2.5	130	13	2,11				

<sup>&</sup>lt;sup>12</sup> Note that these ranges assign the transitional months (March, September, and November) to the ocean condition that is typically more restrictive at relevant discharge flows.

Constituent	Units	Secondary	Hauled Waste	RO Concentrate	Notes
Copper	μg/L	Effluent 11	39	58	2,11,17
Lead	μg/L	2.69	2.69	14.2	2,11,17
Mercury	μg/L	0.085	0.085	0.510	5,12
Nickel	μg/L	12.2	12.2	64	2,11
Selenium	μg/L	6.4	75	34	2,11
Silver	μg/L	0.77	0.77	4.05	5,11
Zinc	μg/L	57.5	170	303	2,11
Cyanide	μg/L	89.7	89.7	143	2,12,13
Total Chlorine Residual	μg/L	ND(<200)	ND(<200)	ND(<200)	10
Ammonia (as N), 6-month median	μg/L	42,900	42,900	225,789	1,11,18
Ammonia (as N), daily maximum	μg/L	49,000	49,000	257,895	1,11,18
Acute Toxicity	ΤUa	2.3	2.3	0.77	7,12,13
Chronic Toxicity	TUc	40	40	100	7,12,13
Phenolic Compounds (non-chlorinated)	μg/L	69	69	363	1,9,11
Chlorinated Phenolics					
Endosulfan	μg/L	ND(<20) 0.046	ND(<20) 0.046	ND(<20) 0.24	4,14
Endrin	μg/L		0.046		5,9,11
-	μg/L	0.000112		0.00059	3,11
HCH (Hexachlorocyclohexane)	μg/L	0.059	0.059	0.312	5,9,11
Radioactivity (Gross Beta)	pCi/L	32	307	34.8	1,7,12,13
Radioactivity (Gross Alpha)	pCi/L	18 •	457	14.4	1,7,12,13
Objectives for protection of human h	1		0.0		0.44
Acrolein	μg/L	8.3	8.3	44	2,11
Antimony	μg/L	0.78	0.78	4.1	2 ,11
Bis (2-chloroethoxy) methane	μg/L	ND(<4.0)	ND(<4.0)	ND(<1)	4,14
Bis (2-chloroisopropyl) ether	μg/L	ND(<4.0)	ND(<4.0)	ND(<1)	4,14
Chlorobenzene	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	4,14
Chromium (III)	μg/L	6.9	87	36	2,11
Di-n-butyl phthalate	μg/L	ND(<7)	ND(<7)	ND(<1)	4,14
Dichlorobenzenes	μg/L	1.6	1.6	8	5,11
Diethyl phthalate	μg/L	ND(<5)	ND(<5)	ND(<1)	4,14
Dimethyl phthalate	μg/L	ND(<2)	ND(<2)	ND(<0.5)	4,14
4,6-dinitro-2-methylphenol	μg/L	ND(<19)	ND(<19)	ND(<5)	4,14
2,4-dinitrophenol	μg/L	ND(<9)	ND(<9)	ND(<5)	4,14
Ethylbenzene	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	4,14
Fluoranthene	μg/L	0.00684	0.00684	0.0360	3,11
Hexachlorocyclopentadiene	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.05)	4,14
Nitrobenzene	μg/L	ND(<2.1)	ND(<2.1)	ND(<1)	4,14
Thallium	μg/L	0.68	0.68	3.6	2,11
Toluene	μg/L	0.48	0.48	2.5	5,11
Tributyltin	μg/L	ND(<0.05)	ND(<0.05)	ND(<0.02)	8,14
1,1,1-trichloroethane	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	4,14
Objectives for protection of human healt					
Acrylonitrile	μg/L	2.5	2.5	13	2,11
Aldrin	μg/L	ND(<0.007)	ND(<0.007)	ND(<0.01)	4,14
Benzene	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	4,14
Benzidine	μg/L	ND(<18.6)	ND(<18.6)	ND(<0.05)	4,14
Beryllium	μg/L	ND(<0.68)	0.0052	ND(<0.5)	4,14
Bis(2-chloroethyl)ether	μg/L	ND(<4.0)	ND(<4.0)	ND(<1)	4,14
Bis(2-ethyl-hexyl)phthalate	μg/L	78	78	411	1,11
Carbon tetrachloride	μg/L	0.50	0.50	2.66	2,11
Chlordane	μg/L	0.00122	0.00122	0.0064	3,9,11
Chlorodibromomethane	μg/L	2.2	2.2	12	2,11



Constituent	Units	Secondary Effluent	Hauled Waste	RO Concentrate	Notes
Chloroform	μg/L	34	34	180	2,11
DDT	μg/L	0.001	0.001	0.0003	2,9,11,15
1,4-dichlorobenzene	μg/L	1.6	1.6	8.4	1,11
3,3-dichlorobenzidine	μg/L	ND(<18)	ND(<18)	ND(<2)	4,14
1,2-dichloroethane	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	4,14
1,1-dichloroethylene	μg/L	ND(<0.5)	0.5	ND(<0.5)	4,14
Dichlorobromomethane	μg/L	2.4	2.4	12	2,11
Dichloromethane (methylenechloride)	μg/L	0.88	0.88	4.6	2,11
1,3-dichloropropene	μg/L	0.56	0.56	3.0	2,11
Dieldrin	μg/L	0.0015	0.0015	0.0001	2,11,15
2,4-dinitrotoluene	μg/L	ND(<2)	ND(<2)	ND(<0.1)	4,14
1,2-diphenylhydrazine (azobenzene)	μg/L	ND(<4)	ND(<4)	ND(<1)	4,14
Halomethanes	μg/L	1.3	1.3	6.9	2,9,11
Heptachlor	μg/L	ND(<0.01)	ND(<0.01)	ND(<0.01)	4,14
Heptachlor epoxide	μg/L	0.000088	0.000088	0.000463	3,11
Hexachlorobenzene	μg/L	0.000078	0.000078	0.000411	3,11
Hexachlorobutadiene	μg/L	0.000009	0.000009	0.000047	3,11
Hexachloroethane	μg/L	ND(<2.1)	ND(<2.1)	ND(<0.5)	4,14
Isophorone	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	4,14
N-Nitrosodimethylamine	μg/L	0.086	0.086	0.150	2,12,13
N-Nitrosodi-N-Propylamine	μg/L	0.076	0.076	0.019	1,12,13
N-Nitrosodiphenylamine	μg/L	ND(<2.1)	ND(<2.1)	ND(<1)	4,14
PAHs	μg/L	0.04	0.04	0.21	2,9,11
PCBs	μg/L	0.00068	0.00068	0.00357	3,9,11
TCDD Equivalents	μg/L	1.39E-7	1.39E-7	7.29E-7	2,8,9,11
1,1,2,2-tetrachloroethane	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	4,14
Tetrachloroethylene	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	4,14
Toxaphene	μg/L	0.0071	0.0071	0.0373	3,11
Trichloroethylene	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	4,14
1,1,2-trichloroethane	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	4,14
2,4,6-trichlorophenol	μg/L	ND(<2.1)	ND(<2.1)	ND(<1)	4,14
Vinyl chloride	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	4,14

#### **Table 1 Notes:**

#### RTP Effluent and Hauled Waste Data

- <sup>1</sup> Existing RTP effluent exceeds concentrations observed in other proposed source waters; the value reported is the existing secondary effluent value.
- <sup>2</sup> The proposed new source waters may increase the secondary effluent concentration; the value reported is based on estimated source water blends.
- <sup>3</sup> RTP effluent value is based on CCLEAN data; no other source waters were considered due to MRL differences.
- <sup>4</sup> MRL provided represents the maximum flow-weighted MRL based on the blend of source waters.

<sup>6</sup> Additional source water data are not available; the reported value is for RTP effluent.

<sup>8</sup> Agricultural Wash Water data are based on an aerated sample, instead of a raw water sample.

<sup>&</sup>lt;sup>5</sup> The only water with a detected concentration was the RTP effluent, however the flow-weighted concentration increases due to higher MRLs for the proposed new source waters.

<sup>&</sup>lt;sup>7</sup> Calculation of the flow-weighted concentration was not feasible due to the constituent, and so the maximum observed value is reported.

<sup>&</sup>lt;sup>9</sup> This value in the Ocean Plan is an aggregate of several congeners or compounds. Per the approach described in the Ocean Plan, for cases where the individual congeners/compounds were less than the MRL, a value of 0 is assumed in calculating the aggregate value.

<sup>10</sup> For all waters, dechlorination will be provided when needed such that the total chlorine residual will be below detection.

#### RO Concentrate Data

- <sup>11</sup> The value presented represents a calculated value assuming no removal prior to RO, complete rejection through RO membrane, and an 81% RO recovery.
- <sup>12</sup> The value represents the maximum value observed during the pilot testing study.
- <sup>13</sup> The calculated value for the RO concentrate data (described in note 11) was not used in the analysis because it was not considered representative. It is expected that the value would increase as a result of treatment through the AWPF (*e.g.* formation of N-Nitrosodimethylamine as a disinfection by-product), or that it will not concentrate linearly through the RO (*e.g.* toxicity and radioactivity).
- <sup>14</sup> The MRL provided represents the limit from the source water and pilot testing monitoring programs.
- <sup>15</sup> The value presented represents a calculated value assuming 93% and 84% removal through primary and secondary treatment for DDT and dieldrin, respectively, 36% and 44% removal through ozone for DDT and dieldrin, respectively, 92% and 97% removal through MF for DDT and dieldrin, respectively, recycling of the MF backwash to the RTP, complete rejection through the RO membrane, and an 81% RO recovery. The assumed removals are based on results from ozone bench-scale testing of Blanco Drain water blended with secondary effluent and low detection sampling through the RTP.

#### General

- <sup>16</sup> Footnote not used
- <sup>17</sup> The value reported for the secondary effluent was calculated using the median of the data collected for the new source waters and is an estimate of the potential increase in concentration of the secondary effluent based on estimated source water blends. The median value was used because the maximum values detected in new source waters appear to be outliers, and because the Ocean Plan objective is a 6-month median concentration, it is reasonable to use the median value detected from these source waters.
- <sup>18</sup> Ammonia (as N) represents the total ammonia concentration, *i.e.* the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>).

#### 4.2 Ocean Modeling Results

Dr. Roberts performed dilution modeling of various discharge scenarios that included combinations of RTP secondary effluent, hauled waste, and Project RO concentrate (Appendix A, Table C3). Year-round compliance with the Ocean Plan objectives was assessed through the evaluation of eight representative discharge scenarios covering the expected range of secondary effluent discharge flows. All scenarios assume the maximum flow rates for the RO concentrate and hauled waste, which is a conservative assumption in terms of constituent loading and minimum dilution.

To assess potential future discharge compositions, various secondary effluent flow rates were included in this analysis. These scenarios encompass the range of operating conditions that is expected to occur for the Project, as well as the best- and worse-case ocean dilution conditions. The eight scenarios used for the compliance assessment, in terms of secondary effluent flow rates to be discharged with the other waste streams, are shown in Table 2, and include:

- Minimum Wastewater Flow (Upwelling) Scenario 1: the maximum influence of the Project RO concentrate on the ocean discharge (*i.e.*, no secondary effluent discharged). The Upwelling ocean condition was used since it represents the worst-case dilution for this flow scenario.
- Low Wastewater Flow (Upwelling) Scenarios 2-3: significant influence of the Project RO concentrate on the ocean discharge (*i.e.*, minimal secondary effluent discharged). The

Upwelling ocean condition was used as it represents the worst-case dilution for this flow scenario.

- Moderate Wastewater Flow (Upwelling) Scenarios 4-7: conditions with a moderate wastewater flow when the Project RO concentrate has a greater influence on the in-pipe water quality than in Scenario 8, but where the ocean dilution (D<sub>m</sub>) is reduced due to the higher overall discharge flow (*i.e.*, compared to Scenarios 1-3). The Upwelling ocean condition was used as it represents the worst-case dilution for these scenarios.
- **High Wastewater Flow (Upwelling) Scenario 8:** the highest expected flow that will be discharged. The Upwelling ocean condition was used as it represents the worst-case dilution for this flow scenario.

Table 2 – Flow scenarios and modeled D<sub>m</sub> values used for Ocean Plan compliance analysis

No.	Discharge Scenario (Ocean Condition)	Secondary Effluent	RO Concentrate	Blended Hauled Waste <sup>1</sup>	D <sub>m</sub>
1	Minimum wastewater flow (Upwelling)	0	1.17	0	498
2	Low wastewater flow (Upwelling)	0.4	1.17	0	460
3	Low Wastewater Flow (Upwelling)	0.6	1.17	0	442
4	Moderate wastewater flow (Upwelling)	2	1.17	0	358
5	Moderate wastewater flow (Upwelling)	4	1.17	0	299
6	Moderate wastewater flow (Upwelling)	4.5	1.17	0	289
7	Moderate wastewater flow (Upwelling)	5	1.17	0	281
8	High wastewater flow (Upwelling)	23.4	1.17	0	174

 $<sup>^{1}</sup>$ A sensitivity analysis was conducted to determine the impacts of hauled waste on the modeled  $D_{m}$  results. It was concluded that neither the flow nor TDS from the addition of hauled waste had a significant impact on the modeled  $D_{m}$  result, and was therefore excluded from the  $D_{m}$  calculation.

#### 4.3 Ocean Plan Compliance Results

The flow-weighted in-pipe concentration for each constituent was calculated for each modeled discharge scenario using the water quality presented in Table 1 and the flows presented in Table 2. The in-pipe concentration was then used to calculate the concentration at the edge of the ZID using the  $D_m$  values presented in Table  $2^{13}$ . The resulting concentrations for each constituent in each scenario were compared to the Ocean Plan objective to assess compliance. The estimated concentrations for all eight flow scenarios are presented as concentrations at the edge of the ZID

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<sup>&</sup>lt;sup>13</sup> The Ocean Plan defines  $D_m$  differently than Dr. Roberts. Dr. Roberts provided dilution results defined as  $S = [total\ volume\ of\ a\ sample]/[volume\ of\ effluent\ contained\ in\ the\ sample]$ . The  $D_m$  referenced in Equation 1 of the California Ocean Plan is defined as  $D_m = S - 1$ . A value of 1 was subtracted from the dilution estimates provided by Dr. Roberts prior to using Equation 1.

(Table 3) and as a percentage of the Ocean Plan objective (Table 4). As shown, none of the constituents are expected to exceed their Ocean Plan objective <sup>14</sup>. Ammonia is estimated to reach a concentration closest to its objective, where it is 71% of the objective in Scenario 1.

Table 3 - Estimated concentrations of Ocean Plan constituents at the edge of the ZID

Constituent	Units	Ocean Plan	Estillated Concentrations at Edde of ZID by Discharde Scenario							
Constituent	011110	Objective	1	2	3	4	5	6	7	8
Objectives for protection o	f marine	aquatic life	;							
Arsenic	μg/L	8	3.0	3.0	3.0	3.1	3.1	3.1	3.1	3.2
Cadmium	μg/L	1	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
Chromium (Hexavalent)	μg/L	2	0.04	0.04	0.04	0.03	0.02	0.02	0.02	0.02
Copper	μg/L	3	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1
Lead	μg/L	2	0.03	0.02	0.02	0.02	0.02	0.02	0.02	0.02
Mercury	μg/L	0.04	0.001	0.001	0.001	0.001	0.001	0.001	0.001	0.001
Nickel	μg/L	5	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
Selenium	μg/L	15	0.1	0.1	0.1	0.05	0.05	0.05	0.04	0.05
Silver	μg/L	0.7	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
Zinc	μg/L	20	8.6	8.5	8.5	8.4	8.4	8.3	8.3	8.4
Cyanide	μg/L	1	0.3	0.3	0.3	0.3	0.3	0.3	0.4	0.5
Total Chlorine Residual	μg/L	2								
Ammonia (as N) - 6-mo median	μg/L	600	424	371	355	302	278	276	273	295
Ammonia (as N) - Daily Max	μg/L	2,400	484	424	406	345	318	315	312	337
Acute Toxicity <sup>a</sup>	TUa	0.3								
Chronic Toxicity <sup>a</sup>	TUc	1								
Phenolic Compounds (non- chlorinated)	μg/L	30	0.7	0.6	0.6	0.5	0.4	0.4	0.4	0.5
Chlorinated Phenolics	μg/L	1	0.04	0.04	0.05	0.1	0.1	0.1	0.1	0.1
Endosulfan	μg/L	0.009	4.5E-04	4.0E-04	3.8E-04	3.2E-04	3.0E-04	3.0E-04	2.9E-04	3.2E-04
Endrin	μg/L	0.002	1.1E-06	9.7E-07	9.3E-07	7.9E-07	7.3E-07	7.2E-07	7.1E-07	7.7E-07
HCH (Hexachlorocyclohexane)	μg/L	0.004	5.9E-04	5.1E-04	4.9E-04	4.2E-04	3.9E-04	3.8E-04	3.8E-04	4.1E-04
Radioactivity (Gross Beta)a	pci/L	_								
Radioactivity (Gross Alpha) <sup>a</sup>	pci/L	-								
Objectives for protection o	f humar	health - no	ncarcinoge	ns						
Acrolein	μg/L	220	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
Antimony	μg/L	1200	0.01	0.01	0.01	0.01	0.01	0.005	0.005	0.01
Bis (2-chloroethoxy) methane	μg/L	4.4	<0.002	<0.004	<0.005	<0.01	<0.01	<0.01	<0.01	<0.02
Bis (2-chloroisopropyl) ether	μg/L	1200	<0.002	<0.004	<0.005	<0.01	<0.01	<0.01	<0.01	<0.02
Chlorobenzene	μg/L	570	<0.001	<0.001	<0.001	<0.001	<0.002	<0.002	<0.002	<0.003
Chromium (III)	μg/L	190000	0.1	0.1	0.1	0.06	0.05	0.05	0.05	0.05
Di-n-butyl phthalate	μg/L	3500	<0.003	<0.01	<0.01	<0.01	<0.02	<0.02	<0.02	<0.04
Dichlorobenzenes	μg/L	5100	0.02	0.01	0.01	0.01	0.01	0.01	0.01	0.01
Diethyl phthalate	μg/L	33000	<0.003	<0.005	<0.01	<0.01	<0.01	<0.01	<0.02	<0.03

<sup>1.</sup> 

<sup>&</sup>lt;sup>14</sup> Aldrin, benzidine, 3,3-dichlorobenzidine and heptachlor were not detected in any source waters, however their MRLs are greater than the Ocean Plan objective. Therefore, no percentages are presented Table 4 as no compliance conclusions can be drawn for these constituents. This is a common occurrence for ocean discharges since the MRL is higher than the ocean plan objective for some constituents.

Constituent	Units	Ocean Plan	Estimated Concentrations at Edge of ZID by Discharge Scenario							
		Objective	1	2	3	4	5	6	7	8
Dimethyl phthalate	μg/L	820000	<0.001	<0.002	<0.002	<0.00	<0.01	<0.01	<0.01	<0.01
4,6-dinitro-2-methylphenol	μg/L	220	<0.01	<0.02	<0.02	<0.04	<0.1	<0.1	<0.1	<0.1
2,4-Dinitrophenol	μg/L	4.0	< 0.01	<0.01	<0.01	<0.02	< 0.03	< 0.03	< 0.03	< 0.05
Ethylbenzene	μg/L	4100	<0.001	<0.001	<0.001	<0.001	<0.002	< 0.002	<0.002	< 0.003
Fluoranthene	μg/L	15	6.8E-05	5.9E-05	5.7E-05	4.8E-05	4.4E-05	4.4E-05	4.4E-05	4.7E-05
Hexachlorocyclopentadiene	μg/L	58	<0.0002	<0.0004	<0.0005	<0.001	<0.001	<0.001	<0.001	< 0.003
Nitrobenzene	μg/L	4.9	<0.002	<0.003	<0.003	<0.005	<0.01	<0.01	<0.01	<0.01
Thallium	μg/L	2	0.01	0.01	0.01	0.005	0.004	0.004	0.004	0.005
Toluene	μg/L	85000	0.005	0.004	0.004	0.003	0.003	0.003	0.003	0.003
Tributyltin	μg/L	0.0014	<4.5E-05	<6.3E-05	<7.0E-05	<1.1E-04	<1.4E-04	<1.5E-04	<1.6E-04	<2.8E-04
1,1,1-Trichloroethane	μg/L	540000	< 0.001	<0.001	<0.001	<0.001	<0.002	<0.002	<0.002	< 0.003
Objectives for protection of	f humar	n health - ca								
Acrylonitrile	μg/L	0.10	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
Aldrin <sup>b</sup>	μg/L	0.000022	<2.0E-05	<2.0E-05	<2.0E-05	<2.2E-05	<2.6E-05	<2.6E-05	<2.7E-05	<4.1E-05
Benzene	μg/L	5.9	<0.001	<0.001	<0.001	<0.001	<0.002	<0.002	<0.002	<0.003
Benzidine <sup>b</sup>	μg/L	0.000069	<0.003	<0.01	<0.02	<0.03	<0.0	<0.1	<0.1	<0.1
Beryllium	μg/L	0.033	0.0009	0.0011	0.0012	0.0017	0.0021	0.0022	0.0023	0.0038
Bis(2-chloroethyl)ether	μg/L	0.045	<0.002	<0.004	<0.005	<0.01	<0.01	<0.01	<0.01	<0.02
Bis(2-ethyl-hexyl)phthalate	μg/L	3.5	0.8	0.7	0.6	0.5	0.5	0.5	0.5	0.5
Carbon tetrachloride	µg/L	0.90	0.00	0.004	0.004	0.004	0.003	0.003	0.003	0.003
Chlordane	μg/L	0.000023	1.2E-05	1.1E-05	1.0E-05	8.5E-06	7.9E-06	7.8E-06	7.7E-06	8.3E-06
Chlorodibromomethane	µg/L	8.6	0.02	0.02	0.02	0.02	0.01	0.01	0.01	0.02
Chloroform	μg/L	130	0.3	0.3	0.3	0.2	0.2	0.2	0.2	0.2
DDT	μg/L	0.00017	6.3E-07	1.0E-06	1.2E-06	2.0E-06	2.7E-06	2.8E-06	3.0E-06	5.3E-06
1,4-Dichlorobenzene	µg/L	18	0.02	0.01	0.01	0.01	0.01	0.01	0.01	0.01
3,3-Dichlorobenzidineb	μg/L	0.0081	< 0.01	<0.01	<0.02	< 0.03	< 0.05	<0.1	<0.1	<0.1
1,2-Dichloroethane	μg/L	28	<0.001	<0.001	<0.001	<0.001	<0.002	<0.002	<0.002	< 0.003
1,1-Dichloroethylene	μg/L	0.9	0.001	0.001	0.001	0.001	0.002	0.002	0.002	0.003
Dichlorobromomethane	μg/L	6.2	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
Dichloromethane (methylenechloride)	μg/L	450	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
1,3-dichloropropene	μg/L	8.9	0.01	0.005	0.005	0.004	0.004	0.004	0.004	0.004
Dieldrin	μg/L	0.00004	4.9E-07	1.2E-06	1.5E-06	2.8E-06	4.0E-06	4.3E-06	4.5E-06	8.3E-06
2,4-Dinitrotoluene	μg/L	2.6	< 0.001	<0.001	<0.002	<0.004	<0.01	<0.01	<0.01	<0.01
1,2-Diphenylhydrazine (azobenzene)	μg/L	0.16	<0.002	<0.004	<0.005	<0.01	<0.01	<0.01	<0.01	<0.02
Halomethanes	μg/L	130	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
Heptachlor <sup>b</sup>	μg/L	0.00005	<2.0E-05	<2.2E-05	<2.3E-05	<2.8E-05	<3.3E-05	<3.4E-05	<3.5E-05	<5.7E-05
Heptachlor Epoxide	μg/L	0.00002	8.7E-07	7.6E-07	7.3E-07	6.2E-07	5.7E-07	5.7E-07	5.6E-07	6.0E-07
Hexachlorobenzene	μg/L	0.00021	7.7E-07	6.7E-07	6.5E-07	5.5E-07	5.1E-07	5.0E-07	5.0E-07	5.4E-07
Hexachlorobutadiene	μg/L	14	8.9E-08	7.8E-08	7.5E-08	6.3E-08	5.8E-08	5.8E-08	5.7E-08	6.2E-08
Hexachloroethane	μg/L	2.5	<0.001	< 0.002	<0.003	<0.004	<0.01	<0.01	<0.01	<0.01
Isophorone	μg/L	730	<0.001	<0.001	<0.001	<0.001	<0.002	< 0.002	< 0.002	< 0.003
N-Nitrosodimethylamine	μg/L	7.3	0.0003	0.0003	0.0003	0.0003	0.0003	0.0003	0.0003	0.0005
N-Nitrosodi-N-Propylamine	μg/L	0.38	0.00005	0.0001	0.0001	0.0002	0.0002	0.0002	0.0002	0.0004
N-Nitrosodiphenylamine	μg/L	2.5	< 0.002	< 0.003	<0.003	<0.005	<0.01	<0.01	<0.01	<0.01
PAHs	μg/L	0.0088	0.0004	0.0004	0.0003	0.0003	0.0003	0.0003	0.0003	0.0003
PCBs	μg/L	0.000019	6.7E-06	5.9E-06	5.6E-06	4.8E-06	4.4E-06	4.4E-06	4.3E-06	4.7E-06
TCDD Equivalents	μg/L	3.9E-09	1.4E-09	1.2E-09	1.1E-09	9.7E-10	9.0E-10	8.9E-10	8.8E-10	9.5E-10
1,1,2,2-Tetrachloroethane	μg/L	2.3	<0.001	<0.001	<0.001	<0.001	<0.002	<0.002	<0.002	<0.003
Tetrachloroethylene	μg/L	2.0	<0.001	<0.001	<0.001	<0.001	<0.002	<0.002	<0.002	< 0.003
Toxaphene	μg/L	2.1E-04	7.0E-05	6.1E-05	5.9E-05	5.0E-05	4.6E-05	4.6E-05	4.5E-05	4.9E-05
Trichloroethylene	μg/L	27	<0.001	<0.001	<0.001	<0.001	<0.002	<0.002	<0.002	<0.003
	μy/∟	21	0.001							
1,1,2-Trichloroethane		9.4	<0.001	<0.001	<0.001	<0.001	<0.002	<0.002	<0.002	< 0.003
1,1,2-Trichloroethane 2,4,6-Trichlorophenol	μg/L μg/L									<0.003 <0.01

<sup>a</sup> Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based the nature of the constituents. These constituents were measured individually for the secondary effluent and RO concentrate, and these individual concentrations would comply with the Ocean Plan objectives.

<sup>b</sup> All observed values from all data sources were below the MRL, and the flow-weighted average of the MRLs is higher than the Ocean Plan objective. No compliance conclusions can be drawn for these constituents.



Table 4 – Estimated concentrations of all COP constituents, expressed as percent of Ocean Plan Objective

				Obje	ctive					
Constituent	Units	Ocean Plan	Estimated Percentage of Ocean Plan Objective at Edge of ZID by Discharge Scenario							
		Objective	1	2	3	4	5	6	7	8
Objectives for protection o		ie aquatic li								
Arsenic	μg/L	8	38%	38%	38%	39%	39%	39%	39%	40%
Cadmium	μg/L	1	1%	1%	1%	1%	1%	1%	1%	1%
Chromium (Hexavalent)	μg/L	2	2%	2%	2%	1%	1%	1%	1%	1%
Copper	μg/L	3	70%	70%	70%	69%	69%	69%	69%	69%
Lead	μg/L	2	1%	1%	1%	1%	1%	1%	1%	1%
Mercury	μg/L	0.04	4%	3%	3%	3%	3%	3%	3%	3%
Nickel	μg/L	5	2%	2%	2%	2%	2%	2%	2%	2%
Selenium	μg/L	15	0.5%	0.4%	0.4%	0.3%	0.3%	0.3%	0.3%	0.3%
Silver	μg/L	0.7	24%	24%	24%	24%	23%	23%	23%	23%
Zinc	μg/L	20	43%	42%	42%	42%	42%	42%	42%	42%
Cyanide	µg/L	1	28%	28%	28%	30%	34%	35%	35%	53%
Total Chlorine Residual	µg/L	2								
Ammonia (as N) - 6-mo										
median	μg/L	600	71%	62%	59%	50%	46%	46%	46%	49%
Ammonia (as N) - Daily Max	μg/L	2,400	20%	18%	17%	14%	13%	13%	13%	14%
Acute Toxicity <sup>a</sup>	TUa	0.3								
Chronic Toxicitya	TUc	1								
Phenolic Compounds (non- chlorinated)	μg/L	30	2%	2%	2%	2%	1%	1%	1%	2%
Chlorinated Phenolics	μg/L	1	4%	4%	5%	6%	7%	7%	7%	11%
Endosulfan	μg/L	0.009	5%	4%	4%	4%	3%	3%	3%	4%
Endrin	μg/L	0.003	0.1%	0.05%	0.05%	0.04%	0.04%	0.04%	0.04%	0.04%
HCH	µg/L	0.002	0.170	0.0576	0.0576	0.04 /0	0.04 /0	0.04 /0	0.04 /0	0.04 /0
(Hexachlorocyclohexane)	μg/L	0.004	15%	13%	12%	10%	10%	10%	9%	10%
Radioactivity (Gross Beta)a	pci/L									
Radioactivity (Gross Beta)	pci/L	_								
Alpha)a	pci/L	-								
Objectives for protection o	f huma									
Acrolein	μg/L	220	0.04%	0.03%	0.03%	0.03%	0.02%	0.02%	0.02%	0.03%
Antimony	μg/L	1200	0.001%	0.001%	0.001%	0.0005%	0.0004%	0.0004%	0.000%	0.000%
Bis (2-chloroethoxy) methane	μg/L	4.4	<0.1%	<0.1%	<0.1%	<0.2%	<0.3%	<0.3%	<0.3%	<0.5%
Bis (2-chloroisopropyl) ether	μg/L	1200	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Chlorobenzene		570	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
OL : (III)	µg/L	190000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	
Chromium (III)	µg/L									<0.01%
Di-n-butyl phthalate	μg/L	3500	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Dichlorobenzenes	μg/L	5100	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Diethyl phthalate	μg/L	33000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Dimethyl phthalate	μg/L	820000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
4,6-dinitro-2-methylphenol	μg/L	220	<0.01%	<0.01%	<0.01%	<0.02%	<0.02%	<0.02%	<0.03%	<0.0%
2,4-Dinitrophenol	μg/L	4.0	<0.3%	<0.3%	<0.4%	<1%	<1%	<1%	<1%	<1%
Ethylbenzene	μg/L	4100	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
			-0.040/	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Fluoranthene	μg/L	15	<0.01%							
Fluoranthene Hexachlorocyclopentadiene	μg/L	58	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	
Fluoranthene Hexachlorocyclopentadiene Nitrobenzene	μg/L μg/L	58 4.9	<0.01% <0.04%	<0.01% <0.1%	<0.01% <0.1%	<0.1%	<0.1%	<0.1%	<0.1%	<0.01% <0.2%
Fluoranthene Hexachlorocyclopentadiene Nitrobenzene	μg/L	58	<0.01%	<0.01%	<0.01%					
Fluoranthene Hexachlorocyclopentadiene Nitrobenzene Thallium	μg/L μg/L	58 4.9	<0.01% <0.04%	<0.01% <0.1%	<0.01% <0.1%	<0.1%	<0.1%	<0.1%	<0.1%	<0.2%
Fluoranthene Hexachlorocyclopentadiene Nitrobenzene Thallium Toluene Tributyltin	μg/L μg/L μg/L	58 4.9 2	<0.01% <0.04% 0.3%	<0.01% <0.1% 0.3%	<0.01% <0.1% 0.3%	<0.1% 0.2%	<0.1% 0.2%	<0.1% 0.2%	<0.1% 0.2%	<0.2% 0.2%

Constituent	Units	Ocean Plan	Estimated Percentage of Ocean Plan Objective at Edge of ZID by Discharge Scenario <sup>c</sup>									
		Objective	1	2	3	4	5	6	7	8		
Acrylonitrile	μg/L	0.10	25%	21%	21%	17%	16%	16%	16%	17%		
Aldrinb	μg/L	0.000022										
Benzene	μg/L	5.9	<0.02%	<0.02%	<0.02%	<0.02%	<0.03%	<0.03%	<0.03%	<0.0%		
Benzidine <sup>b</sup>	μg/L	0.000069										
Beryllium	μg/L	0.033	3%	3%	4%	5%	6%	7%	7%	12%		
Bis(2-chloroethyl)ether	μg/L	0.045	<5%	<9%	<11%	<18%	<24%	<26%	<27%	<49%		
Bis(2-ethyl-hexyl)phthalate	μg/L	3.5	22%	19%	18%	16%	14%	14%	14%	15%		
Carbon tetrachloride	μg/L	0.90	1%	0.5%	0.5%	0.4%	0.4%	0.4%	0.4%	0.4%		
Chlordane	μg/L	0.000023	52%	46%	44%	37%	34%	34%	34%	36%		
Chlorodibromomethane	μg/L	8.6	0.3%	0.2%	0.2%	0.2%	0.2%	0.2%	0.2%	0.2%		
Chloroform	μg/L	130	0.3%	0.2%	0.2%	0.2%	0.2%	0.2%	0.2%	0.2%		
DDT	μg/L	0.00017	0.4%	1%	1%	1%	2%	2%	2%	3%		
1,4-Dichlorobenzene	μg/L	18	0.1%	0.1%	0.1%	0.1%	0.1%	0.1%	0.1%	0.1%		
3,3-Dichlorobenzidineb	μg/L	0.0081	-	-	-							
1,2-Dichloroethane	μg/L	28	<0.01%	<0.01%	<0.01%	<0.01%	0.01%	0.01%	0.01%	0.01%		
1,1-Dichloroethylene	μg/L	0.9	0.1%	0.1%	0.1%	0.2%	0.2%	0.2%	0.2%	0.3%		
Dichlorobromomethane	μg/L	6.2	0.4%	0.3%	0.3%	0.3%	0.2%	0.2%	0.2%	0.3%		
Dichloromethane (methylenechloride)	μg/L	450	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%		
1,3-dichloropropene	μg/L	8.9	0.1%	0.1%	0.1%	0.04%	0.04%	0.04%	0.04%	0.04%		
Dieldrin	μg/L	0.00004	1%	3%	4%	7%	10%	11%	11%	21%		
2,4-Dinitrotoluene	μg/L	2.6	<0.02%	<0.1%	<0.1%	<0.1%	<0.2%	<0.2%	<0.2%	<0.4%		
1,2-Diphenylhydrazine (azobenzene)	μg/L	0.16	<2%	<3%	<3%	<5%	<7%	<7%	<8%	<14%		
Halomethanes	μg/L	130	0.01%	0.01%	0.01%	0.01%	0.01%	0.01%	0.01%	0.01%		
Heptachlor <sup>b</sup>	μg/L	0.00005	<40%	<43%	<45%	<56%	<67%	<69%	<71%			
Heptachlor Epoxide	μg/L	0.00002	4%	4%	4%	3%	3%	3%	3%	3%		
Hexachlorobenzene	μg/L	0.00021	0.4%	0.3%	0.3%	0.3%	0.2%	0.2%	0.2%	0.3%		
Hexachlorobutadiene	μg/L	14	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%		
Hexachloroethane	μg/L	2.5	<0.05%	<0.1%	<0.1%	<0.2%	<0.2%	<0.2%	<0.3%	<0.5%		
Isophorone	μg/L	730	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%		
N-Nitrosodimethylamine	μg/L	7.3	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	0.01%		
N-Nitrosodi-N-Propylamine	μg/L	0.38	0.01%	0.02%	0.02%	0.0%	0.1%	0.1%	0.1%	0.1%		
N-Nitrosodiphenylamine	μg/L	2.5	<0.1%	<0.1%	<0.1%	<0.2%	<0.3%	<0.3%	<0.3%	<0%		
PAHs	μg/L	0.0088	5%	4%	4%	3%	3%	3%	3%	3%		
PCBs	μg/L	0.000019	35%	31%	30%	25%	23%	23%	23%	25%		
TCDD Equivalents	μg/L	3.9E-09	35%	31%	29%	25%	23%	23%	23%	24%		
1,1,2,2-Tetrachloroethane	μg/L	2.3	<0.04%	<0.05%	<0.05%	<0.1%	<0.1%	<0.1%	<0.1%	<0.1%		
Tetrachloroethylene	μg/L	2.0	<0.1%	<0.1%	<0.1%	<0.1%	<0.1%	<0.1%	<0.1%	<0.1%		
Toxaphene	μg/L	2.1E-04	33%	29%	28%	24%	22%	22%	21%	23%		
Trichloroethylene	μg/L	27	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%		
1,1,2-Trichloroethane	μg/L	9.4	<0.01%	<0.01%	<0.01%	<0.01%	<0.02%	<0.02%	<0.02%	<0.03%		
2,4,6-Trichlorophenol	μg/L	0.29	<1%	<1%	<1%	<2%	<2%	<2%	<2%	<4%		
Vinyl chloride	μg/L	36	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%		

<sup>&</sup>lt;sup>a</sup> Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based the nature of the constituents. These constituents were measured individually for the secondary effluent and RO concentrate, and these individual concentrations would comply with the Ocean Plan objectives (see Section 4.4)

objectives (see Section 4.4).

<sup>b</sup> All observed values from all data sources were below the MRL, and the flow-weighted average of the MRLs is higher than the Ocean Plan objective. No compliance conclusions can be drawn for these constituents.

<sup>&</sup>lt;sup>c</sup> Note that if the percentage was determined to be less than 0.01 percent, then a minimum value is shown as "<0.01%" (*e.g.*, if the constituent was estimated to be 0.000001% of the objective, for simplicity, it is displayed as <0.01%). Also, shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.



## 4.4 Toxicity

The NPDES permit includes daily maximum effluent limitations for acute and chronic toxicity that are based on the current allowable  $D_m$  of 145. The acute toxicity effluent limitation is 4.7 TUa (acute toxicity units) and the chronic toxicity effluent limitation is 150 TUc (chronic toxicity units). The permit requires that toxicity testing be conducted twice per year, with one sample collected during the wet season when the discharge is primarily secondary effluent and once during the dry season when the discharge is primarily trucked brine waste. The MRWPCA ocean discharge has consistently complied with these toxicity limits (CCRWQCB, 2014).

Toxicity testing of RO concentrate generated by the pilot testing was conducted in support of the Project (Trussell Technologies, 2015). On April 9, 2014, a sample of RO concentrate was sent to Pacific EcoRisk for acute and chronic toxicity analysis. Based on these results (RO concentrate values presented in Table 1), the Project concentrate requires a minimum  $D_{\rm m}$  of 16:1 and 99:1 for acute and chronic toxicity, respectively, to meet the Ocean Plan objectives. These  $D_{\rm m}$  values were compared to estimated  $D_{\rm m}$  values for the discharge of RO concentrate only from the Project's full-scale AWPF and the discharge of RO concentrate combined with secondary effluent from the RTP. The minimum dilution modeled for the various Project discharge scenarios was 174:1, which is when the secondary effluent discharge is at the highest expected flow for future discharges. Given that the lowest expected  $D_{\rm m}$  value for the various Project ocean discharge scenarios is greater than the required dilution factor for compliance with the Ocean Plan toxicity objectives, this sample illustrates that the discharge scenarios would comply with Ocean Plan objectives.

## 5 Conclusions

The purpose of the analysis documented in this technical memorandum was to assess the ability of the Project to comply with the numeric Ocean Plan water quality objectives. Trussell Tech used a conservative approach to estimate the water qualities of the RTP secondary effluent, RO concentrate, and hauled waste (blended with secondary effluent) for the Project. These water quality data were then combined for various discharge scenarios, and a concentration at the edge of the ZID was calculated for each constituent and scenario. Compliance assessments could not be made for select constituents, as noted, due to analytical limitations, but this is a common occurrence for these Ocean Plan constituents. Based on the data, assumptions, modeling, and analytical methodology presented in this technical memorandum, the Project would comply with all Ocean Plan objectives.

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## Appendix A

# Modeling Brine Disposal into Monterey Bay – Supplement

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Final Report

Prepared for
ESA | Environmental Science Associates
San Francisco, California

September 22, 2017

## **CONTENTS**

Contents	i
Executive Summary	ii
List of Figures	iii
List of Tables	iv
1. Introduction	1
Modeling Scenarios	2 2
3. Outfall Hydraulics	7
4. Dense Discharge Dilution	10 10 14
5. Buoyant Discharge Dilution	17
6. Dilution Mitigation – Effect of Nozzle Angle	20 20
References	25
Appendix A. Density Profiles	26
Appendix B. Additional Scenarios	27
Appendix C. Effect of Nozzle Angle on Dilution	,

#### **EXECUTIVE SUMMARY**

Additional dilution simulations are presented for the disposal of brine concentrate resulting from reverse osmosis (RO) seawater desalination into Monterey Bay, California. The report is a supplement to Roberts (2016) and addresses new flow scenarios and other issues that have been raised.

It has been suggested to replace the opening in the end gate of the diffuser with a check valve. A 6-inch valve was proposed, and analyses of the internal hydraulics of the diffuser and outfall were conducted. The check valve had minimal effect on the flow distribution between the diffuser ports and minimal effect on head loss. The flow from the end gate was reduced slightly and the exit velocity considerably increased. The effect of the valve orientation on dilution of brine discharges was investigated. It was found that any upward angle greater than about 20° would result in dilutions that meet the BMZ salinity requirements. The optimum angle to maximize dilution is 60°.

Dilutions were computed for all new flow scenarios assuming the 6-inch check valve was installed in the end gate.

The effect of currents on the brine jets was addressed. Dilutions were predicted using the mathematical model UM3 for the pure brine discharges for various anticipated current speeds. Jets discharging into the currents were bent back and dilutions were increased by the current. Jets discharging with the current were swept downstream and impacted the seabed farther from the diffuser. All dilutions with currents were greater than those with zero current, and all impact points were well within the BMZ.

It has been suggested to orient the nozzles along the diffuser upwards (from their present horizontal angles) to increase the dilution of dense effluents. This would decrease the dilution of buoyant effluents, however. Dilutions were predicted for dense and buoyant effluents. For dense effluents, increasing the nozzle angle increased dilution considerably; for buoyant effluents, the dilutions reduced slightly.

## **LIST OF FIGURES**

Figure 1.	Seasonally averaged density profiles used for dilution simulations	. 3
Figure 2.	Characteristics of 6-inch TideFlex check valve Hydraulic Code 355	.8
Figure 3.	Typical port flow distributions with and without the endgate check valve for cases T1 and T2.	. 9
Figure 4.	Screen shots of UM3 simulations of dense jet trajectories (Case T2) in counter- and co-flowing currents. Red: zero current; Blue: 10 cm/s; Green: 20 cm/s.	14
Figure 5.	Central plane tracer concentrations for dense jets at various nozzle angles from $15^{\circ}$ to $85^{\circ}$ . After Abessi and Roberts (2015)	20
Figure 6.	Effect of nozzle angle on normalized dilution of dense jets. After Abessi and Roberts (2015).	21
Figure 7.	Effect of nozzle angle on dilution of dense jets, case T2.	22
Figure 8.	Effect of nozzle angle on dilution for selected buoyant discharge scenarios	23

## LIST OF TABLES

Table 1. Seasonally Averaged Properties at Diffuser Depth	3
Table 2. Assumed Properties of Effluent Constituents	3
Table 3. Modeled Discharge Scenarios – Project (no GWR)	5
Table 4. Modeled Discharge Scenarios – Variant	6
Table 5. Summary of Dilution Simulations for Dense Effluent Scenarios – Project (no GWR)1	1
Table 6. Summary of Dilution Simulations for Dense Effluent Scenarios – Variant12	2
Table 7. UM3 Simulations of Case T2 with Current15	5
Table 8. Effect of Nozzle Angle on Impact Dilution for Flow from End Gate Check Valve for Case T2 (14.08 mgd, 1045.1 kg/m³)16	6
Table 9. Summary of Dilution Simulations for Buoyant Effluent Scenarios – Project and Variant	8
Table 10. Effect of Nozzle Angle on Dense Jets Case T2. (for conditions, see Table 3) 22	2
Table 11. Effect of nozzle Angle on Dilution for Selected Buoyant Effluent Scenarios 24	4
Table B1. Additional Modeled Discharge Scenarios28	3
Table B2. Summary of Dilution Simulations for Dense Additional Scenarios29	9
Table B3. Summary of Dilution Simulations for Buoyant Additional Scenarios30	С
Table C1. Further Modeled Discharge Scenarios32	2
Table C2. Summary of Dilution Simulations for Dense Scenarios33	3
Table C3. Summary of Dilution Simulations for Buoyant Further Scenarios35	5

#### 1. INTRODUCTION

It is proposed to dispose of the brine concentrate resulting from reverse osmosis (RO) seawater desalination into Monterey Bay, California. Discharge will be through an existing outfall and diffuser usually used for domestic wastewater disposal. Because of varying flow scenarios, the effluent and its composition vary from pure secondary effluent to pure brine. Sixteen scenarios, with flows ranging from 9.0 to 33.8 mgd (million gallons per day) and densities from 998.8 to 1045.2 kg/m³, were previously analyzed in Roberts (2016). The internal hydraulics of the outfall and diffuser were computed and dilutions predicted for flow scenarios resulting in buoyant and dense effluents. It was found that, for all dense discharge conditions, the salinity requirements in the new California Ocean Plan were met within the BMZ (Brine Mixing Zone).

Since that report was completed, new flow scenarios have been proposed that include higher volumes of brine and GWR effluent, the inclusion of hauled brine, and situations where the desalination plant is offline. It has been requested to analyze dilutions for many more flow combinations for typical and variant cases. And it is proposed to replace the opening in the diffuser's end gate, which allows some brine to be released at a low velocity and therefore low dilution, with a check valve that would increase the exit velocity and therefore increase dilution. The check valve would be angled upwards, further increasing dilution. Finally, it has been suggested to replace the horizontal 4-inch check valves along the diffuser with upwardly oriented valves that would increase the dilution of dense effluents.

The specific tasks addressed in this report are:

- Analyze internal hydraulics accounting for the effect of the new proposed end gate check valve;
- Compute dilutions for new scenarios with dense and buoyant flow effluents accounting for the effect of the valve;
- Assess the effects of currents on dense discharges;
- Compute the dilution of dense discharges from the end gate;
- Analyze the effect of varying the nozzle angle on the dilution of dense and buoyant effluents.

#### 2. MODELING SCENARIOS

#### 2.1 Introduction

To address the additional concerns and issues that have been raised, the revised dilution analyses will include the following:

- **End-Gate**: The outfall hydraulics will be revised assuming the end-gate has been replaced with one Tideflex valve. The assumed end-gate configuration may be modified depending on the California Ocean Plan (COP) compliance analysis results.
- **Effluent Water Quality:** The salinity and temperature of the secondary effluent and GWR effluent shall remain unchanged from prior analyses presented in the 2017 Draft EIR/EIS.
- Ocean Conditions: Dilution analyses shall incorporate conditions related to the ocean seasons consistent with previous analyses. Worstcase conditions shall be assessed and presented.
- **Mitigation:** Preliminary assessments of the impact of diffuser nozzle orientation on dilution of dense and buoyant effluents will be made.
- **Currents:** The effects of currents on the advection and dispersion of dense effluents will be assessed.

All revised discharge scenarios will incorporate consideration of a modified end-gate on outfall diffuser hydraulics and dilution.

Model analyses will be done for typical and high brine discharge scenarios with a range of secondary and GWR effluent flows. Modeling the highest RO concentrate flow expected follows the conservative approach previously used on COP compliance evaluations for this project. Also, scenarios involving high flows of secondary effluent will be assessed for typical operations of the Variant both with and without GWR effluent. In addition, it has been requested that discharge scenarios where brine is absent be included in dilution model analyses to cover times when the desalination plant is offline.

#### 2.2 Environmental and Discharge Conditions

In the previous report, Roberts (2016), oceanographic measurements obtained near the diffuser were discussed. Traditionally, three oceanic seasons have been defined in Monterey Bay: Upwelling (March-September), Oceanic (September-November), and Davidson (November-March). Density profiles were averaged by season to obtain representative profiles for the dilution simulations. The profiles are shown in Figure 1 and are tabulated in Appendix A. The salinities and temperatures near the depth of the diffuser were averaged seasonally as summarized in Table 1.

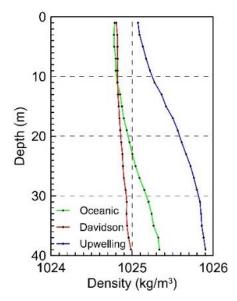


Figure 1. Seasonally averaged density profiles used for dilution simulations.

Table 1. Seasonally Averaged Properties at Diffuser Depth

Season	Temperature (°C)	Salinity (ppt)	Density (kg/m³)
Davidson	14.46	33.34	1024.8
Upwelling	11.48	33.89	1025.8
Oceanic	13.68	33.57	1025.1

The assumed constituent properties are summarized in Table 2.

Table 2. Assumed Properties of Effluent Constituents

Constituent	Temperature (°C)	Salinity (ppt)	Density (kg/m³)
Secondary effluent	20.0	0.80	998.8
Brine	9.9	58.23	1045.2
GWR	20.0	5.80	1002.6
Hauled brine	20.0	40.00	1028.6

#### 2.3 Discharge Scenarios

Following publication of the 2017 MPWSP Draft EIR/EIS, the MRWPCA commented on several concerns related to the impact analysis regarding Ocean Plan and NPDES compliance. Specifically, discharge scenarios involving higher volumes of desalination brine (following a shut down for repair or routine

maintenance) had not been assessed. Also, it was requested that higher resolution model analysis be conducted for scenarios involving low and moderate flows of secondary effluent for all project alternatives. Additionally, the MRWPCA requested that increased GWR effluent flows be assessed as part of planning for an increased capacity PWM project. Finally, it was requested that hauled brine be included in the dilution analysis for the Proposed Project.

It is proposed that revised model analysis be completed for typical and high brine discharge scenarios with secondary effluent flows ranging from 0 to 10 mgd and with the inclusion of hauled brine. Additionally, scenarios involving high flows of secondary effluent (15 and 19.78 mgd) will be assessed for typical operations. In addition, MPWPCA has requested that discharge scenarios where brine is absent be included in dilution model analyses to cover times when the desal plant is offline and to revise dilution model estimates based on the modified end-gate which may alter the outfall diffuser hydraulics.

Table 3 details the revised discharge scenarios for dilution model analysis of the Proposed Project (full size desalination facility and no implementation of GWR/PWM).

Table 4 details revised discharge scenarios for dilution model analysis of the Variant (MPWSP Alternative, reduced capacity desalination facility with PWM/GWR).

Table 3. Modeled Discharge Scenarios - Project (no GWR)

Case ID	Scenario	C	Constituent flo	ws (mgc	i)	Co	mbined eff	luent
		Brine	Secondary effluent	GWR	Hauled brine	Flow (mgd)	Salinity (ppt)	Density (kg/m³)
T1	SE Only	0.00	19.78	0	0.1	19.88	1.00	999.0
T2	Brine only	13.98	0.00	0	0.1	14.08	58.10	1045.1
Т3	Brine + Low SE	13.98	1.00	0	0.1	15.08	54.30	1042.0
T4	Brine + Low SE	13.98	2.00	0	0.1	16.08	50.97	1039.4
T5	Brine + Low SE	13.98	3.00	0	0.1	17.08	48.04	1037.0
T6	Brine + Low SE	13.98	4.00	0	0.1	18.08	45.42	1034.9
T7	Brine + Moderate SE	13.98	5.00	0	0.1	19.08	43.08	1033.0
T8	Brine + Moderate SE	13.98	6.00	0	0.1	20.08	40.98	1031.3
Т9	Brine + Moderate SE	13.98	7.00	0	0.1	21.08	39.07	1029.7
T10	Brine + Moderate SE	13.98	8.00	0	0.1	22.08	37.34	1028.3
T11	Brine + Moderate SE	13.98	9.00	0	0.1	23.08	35.76	1027.1
T12	Brine + High SE	13.98	10.00	0	0.1	24.08	34.30	1025.9
T13	Brine + High SE	13.98	15.00	0	0.1	29.08	28.54	1021.2
T14	Brine + High SE	13.98	19.78	0	0.1	33.86	24.63	1018.1
T15	High Brine only	16.31	0.00	0	0.1	16.41	58.12	1045.1
T16	High Brine + Low SE	16.31	1.00	0	0.1	17.41	54.83	1042.5
T17	High Brine + Low SE	16.31	2.00	0	0.1	18.41	51.89	1040.1
T18	High Brine + Low SE	16.31	3.00	0	0.1	19.41	49.26	1038.0
T19	High Brine + Low SE	16.31	4.00	0	0.1	20.41	46.89	1036.1
T20	High Brine + Moderate SE	16.31	5.00	0	0.1	21.41	44.73	1034.3

Table 4. Modeled Discharge Scenarios - Variant

Case ID	Scenario		Constituent Fl	ows (mg	d)	Co	Combined effluent			
		Brine	Secondary effluent	GWR	Hauled brine	Flow (mgd)	Salinity (ppt)	Density (kg/m³)		
V1	Brine only	8.99	0.00	0	0.0	8.99	58.23	1045.2		
V2	Brine + Low SE	8.99	1.00	0	0.0	9.99	52.48	1040.6		
V3	Brine + Low SE	8.99	2.00	0	0.0	10.99	47.78	1036.8		
V4	Brine + Low SE	8.99	3.00	0	0.0	11.99	43.86	1033.6		
V5	Brine + Low SE	8.99	4.00	0	0.0	12.99	40.55	1030.9		
V6	Brine + Moderate SE	8.99	5.00	0	0.0	13.99	37.70	1028.6		
V7	Brine + Moderate SE	8.99	5.80	0	0.0	14.79	35.71	1027.0		
V8	Brine + Moderate SE	8.99	7.00	0	0.0	15.99	33.09	1024.9		
V9	Brine + High SE	8.99	14.00	0	0.0	22.99	23.26	1017.0		
V10	Brine + High SE	8.99	19.78	0	0.0	28.77	18.75	1013.3		
V11	GWR Only	0.00	0.00	1.17	0.0	1.17	5.80	1002.6		
V12	Low SE + GWR	0.00	0.40	1.17	0.0	1.57	4.53	1001.6		
V13	Low SE + GWR	0.00	3.00	1.17	0.0	4.17	2.20	999.9		
V14	High SE + GWR	0.00	23.70	1.17	0.0	24.87	1.04	999.0		
V15	High SE + GWR	0.00	24.70	1.17	0.0	25.87	1.03	999.0		
V16	Brine + High GWR only	8.99	0.00	1.17	0.0	10.16	52.19	1040.3		
V17	Brine + High GWR + Low SE	8.99	1.00	1.17	0.0	11.16	47.59	1036.6		
V18	Brine + High GWR + Low SE	8.99	2.00	1.17	0.0	12.16	43.74	1033.5		
V19	Brine + High GWR + Low SE	8.99	3.00	1.17	0.0	13.16	40.48	1030.9		
V20	Brine + High GWR + Low SE	8.99	4.00	1.17	0.0	14.16	37.67	1028.6		
V21	Brine + High GWR + Moderate SE	8.99	5.00	1.17	0.0	15.16	35.24	1026.6		
V22	Brine + High GWR + Moderate SE	8.99	5.30	1.17	0.0	15.46	34.57	1026.1		
V23	Brine + High GWR + Moderate SE	8.99	6.00	1.17	0.0	16.16	33.11	1024.9		
V24	Brine + High GWR + Moderate SE	8.99	7.00	1.17	0.0	17.16	31.23	1023.4		
V25	Brine + High GWR + High SE	8.99	11.00	1.17	0.0	21.16	25.48	1018.7		
V26	Brine + High GWR + High SE	8.99	15.92	1.17	0.0	26.08	20.82	1015.0		
V27	Brine + Low GWR only	8.99	0.00	0.94	0.0	9.93	53.27	1041.2		
V28	Brine + Low GWR + Low SE	8.99	1.00	0.94	0.0	10.93	48.47	1037.3		
V29	Brine + Low GWR + Low SE	8.99	3.00	0.94	0.0	12.93	41.09	1031.4		
V30	Brine + Low GWR + Moderate SE	8.99	5.30	0.94	0.0	15.23	35.01	1026.4		
V31	Brine + Low GWR + High SE	8.99	15.92	0.94	0.0	25.85	20.95	1015.1		
V32	High Brine only	11.24	0.00	0.00	0.0	11.24	58.23	1045.2		
V33	High Brine + Low SE	11.24	0.50	0.00	0.0	11.74	55.78	1043.3		
V34	High Brine + Low SE	11.24	1.00	0.00	0.0	12.24	53.54	1041.4		
V35	High Brine + Low SE	11.24	2.00	0.00	0.0	13.24	49.55	1038.2		
V36	High Brine + Low SE	11.24	3.00	0.00	0.0	14.24	46.13	1035.5		
V37	High Brine + Low SE	11.24	4.00	0.00	0.0	15.24	43.16	1033.0		
V38	High Brine + Moderate (5) SE	11.24	5.00	0.00	0.0	16.24	40.55	1030.9		
V39	High Brine + GWR only	11.24	0.00	1.17	0.0	12.41	53.29	1041.2		
V40	High Brine + GWR + Low SE	11.24	0.50	1.17	0.0	12.91	51.25	1039.6		
V41	High Brine + GWR + Low SE	11.24	1.00	1.17	0.0	13.41	49.37	1038.0		
V42	High Brine + GWR + Low SE	11.24	2.00	1.17	0.0	14.41	46.00	1035.3		
V43	High Brine + GWR + Low SE	11.24	3.00	1.17	0.0	15.41	43.07	1033.0		
V44	High Brine + GWR + Low SE	11.24	4.00	1.17	0.0	16.41	40.49	1030.9		
V45	High Brine + GWR + Moderate SE	11.24	5.00	1.17	0.0	17.41	38.21	1029.0		

#### 3. OUTFALL HYDRAULICS

#### 3.1 Introduction

The outfall and diffuser is described in Roberts (2016) (see Figure 1 in that report) as follows:

The Monterey Regional Water Pollution Control Agency (MRWPCA) outfall at Marina conveys the effluent to the Pacific Ocean to a depth of about 100 ft below Mean Sea Level (MSL). The ocean segment extends a distance of 9,892 ft from the Beach Junction Structure (BJS). Beyond this there is a diffuser section 1,406 ft long. The outfall pipe consists of a 60-inch internal diameter (ID) reinforced concrete pipe (RCP), and the diffuser consists of 480 ft of 60-inch RCP with a single taper to 840 ft of 48-inch ID. The diffuser has 171 ports of two-inch diameter: 65 in the 60-inch section and 106 in the 48-inch section. The ports discharge horizontally alternately from both sides of the diffuser at a spacing of 16 ft on each side except for one port in the taper section that discharges vertically for air release. The 42 ports closest to shore are presently closed, so there are 129 open ports distributed over a length of approximately 1024 ft. The 129 open ports are fitted with four inch Tideflex "duckbill" check valves (the four inch refers to the flange size not the valve opening). The valves open as the flow through them increases so the cross-sectional area is variable. The end gate has an opening at the bottom about two inches high. The hydraulic characteristics of the four-inch valves and the procedure to compute the flow distribution in the diffuser with the end gate opening was detailed in Roberts (2016) Appendix A.

It is proposed to replace the end gate opening with a Tideflex check valve. A suitable valve is a 6 inch Tideflex check valve, Hydraulic Code 355. The hydraulic characteristics of this valve are shown in Figure 2.

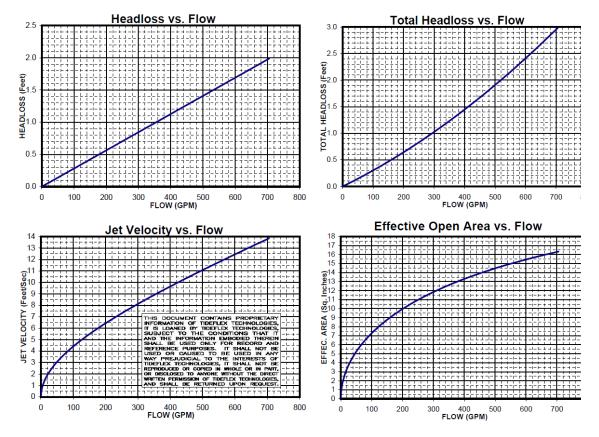


Figure 2. Characteristics of 6-inch TideFlex check valve Hydraulic Code 355.

The same methodology to compute the internal hydraulics as outlined in Roberts (2016) was used. For the purposes of the hydraulic computations, the relationship between the total head loss across the valve, E' and the flow Q of Figure 2 was approximated by:

$$Q = -28.24E'^2 + 319.8E' \tag{1}$$

The calculation procedure followed that in Roberts (2016) except that the open end gate relationship was replaced by Eq. 1.

Typical flow variations with and without the end gate valve are shown in Figure 3. This shows Case T1, mostly secondary effluent with a total flow of 19.88 mgd, density 999.0 kg/m³, and case T2, almost pure brine with a flow of 14.08 mgd, density 1045.1 kg/m³. The flow distributions with and without the Tideflex valve are virtually indistinguishable. The flow exiting from the end gate is reduced slightly from 4% to 3% of the total for T1 and from 5% to 4% for T2. The velocity from the end gate is increased significantly by the check valve, from 6.7 to 10.7 ft/s for T1 and from 6.1 to 9.7 ft/s for T2. The additional total head loss through the outfall due to the check valve is negligible, about 0.01 ft.

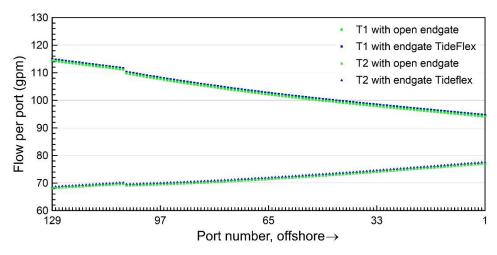


Figure 3. Typical port flow distributions with and without the endgate check valve for cases T1 and T2.

#### 3.2 Effect of End Gate Valve on Dilution

The end gate check valve decreases the flow from the end gate and increases the flow from the two-inch ports. The dilution calculations later in this report assume the check valve is in place. To assess the effect of the valve on dilution from the main diffuser, dilutions were calculated for cases T1 and T2.

For T1, the total flow through the two-inch ports increased from 19.1 to 19.2 mgd (0.5%) and the port diameter increased from 2.00 to 2.01 inches. This had no effect on dilution (when rounded to a whole number).

For T2, the total flow through the two-inch ports increased from 13.4 to 13.5 mgd (0.8%) and the port diameter was unchanged at 1.84 inches. This had no effect on dilution (when rounded to a whole number).

#### 4. Dense Discharge Dilution

#### 4.1 Introduction

The calculation procedure was similar to that in Roberts (2016), where dilutions were predicted by two methods. First was the semi-empirical equation due to Cederwall (1968) (Eq. 3 in Roberts, 2016):

$$\frac{S_i}{F_j} = 0.54 \left( 0.66 + 0.38 \frac{z}{dF_j} \right)^{5/3}$$
 (2)

where  $S_i$  is the impact dilution,  $F_j$  the jet densimetric Froude number, and z the height of the nozzle above the seabed. Second, the dilution and trajectories of the jets were predicted by UM3, a Lagrangian entrainment model in the mathematical modeling suite Visual Plumes (Frick et al. 2003, Frick 2004, and Frick and Roberts 2016).

First, the internal hydraulics program was run to determine the flow variation along the diffuser. Dilutions were then computed for the flow and equivalent nozzle diameter for the innermost and outermost nozzles and the lowest dilution chosen. Worst-case oceanic conditions were assumed, which corresponds to the lowest oceanic density, the "Davidson" condition (Table 1), i.e. salinity = 33.34 ppt, density = 1024.8 kg/m<sup>3</sup>.

#### 4.2 Results

The results for the Project scenarios (Table 3) are summarized in Table 5, and for the Variant (Table 4) in Table 6. For large density differences, the Cederwall equation gives the lowest dilutions but as the effluent density approaches the ambient density, UM3 gives lower dilutions. To be conservative, the lowest of the two model predictions was chosen, as shown in last columns of Tables 5 and 6. The increase in dilution from the impact point to the edge of the BMZ was assumed to be 20% as discussed in Roberts (2016).

All dense discharges meet the Ocean Plan requirement of a 2 ppt increment in salinity at the edge of the BMZ.

Table 5. Summary of Dilution Simulations for Dense Effluent Scenarios - Project (no GWR)

Case	Efflu	ent cond	litions		Port c	onditions					Prediction	าร		
ID								Cederwall	U	M3	At imp	act (ZID)	At	BMZ
	Flow (mgd)	Salinity (ppt)	Density (kg/m³)	Flow (gpm)	Diam. (inch)	Velocity (ft/s)	Froude no.	Dilution	Dilution	Distance (ft)	Dilution	Salinity increment (ppt)	Dilution	Salinity increment (ppt
T2	14.08	58.10	1045.1	77.8	1.88	9.0	28.5	15.4	16.2	10.2	15.4	1.61	18.5	1.34
Т3	15.08	54.30	1042.0	82.8	1.91	9.3	31.6	16.0	16.1	10.4	16.0	1.31	19.2	1.09
T4	16.08	50.97	1039.4	80.8	1.89	9.2	34.5	16.8	17.6	11.6	16.8	1.05	20.1	0.88
T5	17.08	48.04	1037.0	86.2	1.92	9.6	38.6	17.7	18.5	12.7	17.7	0.83	21.2	0.69
T6	18.08	45.42	1034.9	91.6	1.95	9.8	43.4	18.8	19.5	13.8	18.8	0.64	22.5	0.54
T7	19.08	43.08	1033.0	97.1	1.98	10.1	49.2	20.1	20.9	15.3	20.1	0.48	24.2	0.40
T8	20.08	40.98	1031.3	103.1	2.01	10.4	56.5	21.9	22.2	16.8	21.9	0.35	26.3	0.29
Т9	21.08	39.07	1029.7	108.7	2.02	10.9	67.4	24.8	24.9	19.2	24.8	0.23	29.7	0.19
T10	22.08	37.34	1028.3	114.2	2.05	11.1	80.6	28.2	27.5	21.9	27.5	0.15	33.0	0.12
T11	23.08	35.76	1027.1	119.8	2.07	11.4	103.3	34.2	27.7	22.3	27.7	0.09	33.2	0.07
T12	24.08	34.30	1025.9	125.3	2.10	11.6	150.4	46.7	39.2	33.0	39.2	0.02	47.0	0.02
T15	16.41	58.12	1045.1	82.4	1.90	9.3	29.3	15.5	16.3	10.5	15.5	1.60	18.6	1.33
T16	17.41	54.83	1042.5	87.8	1.93	9.6	32.3	16.1	16.9	11.3	16.1	1.34	19.3	1.11
T17	18.41	51.89	1040.1	93.3	1.96	9.9	35.4	16.7	17.5	12.1	16.7	1.11	20.1	0.92
T18	19.41	49.26	1038.0	98.7	1.99	10.2	38.9	17.5	18.4	13.1	17.5	0.91	21.0	0.76
T19	20.41	46.89	1036.1	104.8	2.01	10.6	43.6	18.6	19.3	14.2	18.6	0.73	22.3	0.61
T20	21.41	44.73	1034.3	110.3	2.04	10.8	48.1	19.6	20.4	15.4	19.6	0.58	23.6	0.48

**Table 6. Summary of Dilution Simulations for Dense Effluent Scenarios - Variant** 

Case	Efflu	uent cond	litions		Port c	onditions					Prediction	ıs		
ID								Cederwall	U	M3	At imp	act (ZID)	At	BMZ
	Flow (mgd)	Salinity (ppt)	Density (kg/m³)	Flow (gpm)	Diam. (inch)	Velocity (ft/s)	Froude no.	Dilution	Dilution	Distance (ft)	Dilution	Salinity increment (ppt)	Dilution	Salinity increment (ppt)
V1	9.0	58.23	1045.2	51.6	1.68	7.5	23.9	15.7	16.0	8.6	15.7	1.59	18.8	1.32
V2	10.0	52.48	1040.6	55.8	1.72	7.7	28.9	16.3	16.9	9.6	16.3	1.17	19.6	0.98
V3	11.0	47.78	1036.8	54.9	1.71	7.7	33.1	17.4	18.1	10.5	17.4	0.83	20.8	0.69
V4	12.0	43.86	1033.6	61.5	1.76	8.1	40.3	18.8	19.8	12.4	18.8	0.56	22.6	0.47
V5	13.0	40.55	1030.9	67.3	1.81	8.4	49.2	20.9	21.6	14.4	20.9	0.35	25.0	0.29
V6	14.0	37.70	1028.6	73.4	1.85	8.8	64.3	24.6	24.9	17.5	24.6	0.18	29.5	0.15
V7	14.8	35.71	1027.0	76.8	1.87	9.0	86.0	30.3	29.4	21.4	29.4	0.08	35.3	0.07
V8	16.0	33.09	1024.9	76.3	1.87	8.9	382.9	110.2	67.6	51.4	67.6	0.00	81.1	0.00
V16	10.2	52.19	1040.3	56.8	1.72	7.8	29.7	16.5	17.3	9.9	16.5	1.14	19.8	0.95
V17	11.2	47.59	1036.6	56.1	1.72	7.8	33.6	17.4	18.3	10.8	17.4	0.82	20.9	0.68
V18	12.2	43.74	1033.5	63.5	1.79	8.1	40.1	18.7	19.3	12.3	18.7	0.56	22.4	0.46
V19	13.2	40.48	1030.9	68.3	1.81	8.5	50.3	21.1	21.8	14.5	21.1	0.34	25.4	0.28
V20	14.2	37.67	1028.6	73.8	1.85	8.8	65.0	24.8	24.9	17.5	24.8	0.17	29.8	0.15
V21	15.2	35.24	1026.6	80.9	1.89	9.3	97.2	33.2	31.7	23.5	31.7	0.06	38.0	0.05
V22	15.5	34.57	1026.1	79.8	1.89	9.1	114.2	37.7	34.3	25.6	34.3	0.04	41.2	0.03
V23	16.2	33.11	1024.9	83.3	1.91	9.3	395.8	113.5	68.5	53.5	68.5	0.00	82.2	0.00
V27	9.9	53.27	1041.2	55.3	1.71	7.7	28.5	16.3	16.9	9.5	16.3	1.22	19.6	1.02

Table 6. Summary of Dilution Simulations for Dense Effluent Scenarios - Variant

Case	Efflu	uent cond	litions		Port c	onditions					Prediction	ıs		
ID								Cederwall	U	М3	At imp	act (ZID)	At	BMZ
	Flow (mgd)	Salinity (ppt)	Density (kg/m³)	Flow (gpm)	Diam. (inch)	Velocity (ft/s)	Froude no.	Dilution	Dilution	Distance (ft)	Dilution	Salinity increment (ppt)	Dilution	Salinity increment (ppt)
V28	10.9	48.47	1037.3	59.3	1.75	7.9	33.1	17.1	17.8	10.7	17.1	0.88	20.6	0.74
V29	12.9	41.09	1031.4	67.0	1.80	8.5	48.1	20.6	21.1	13.9	20.6	0.38	24.7	0.31
V30	15.2	35.01	1026.4	78.3	1.88	9.1	100.6	34.1	32.6	24.1	32.6	0.05	39.1	0.04
V32	11.2	58.23	1045.2	63.3	1.78	8.2	26.5	15.4	16.1	9.3	15.4	1.61	18.5	1.34
V33	11.7	55.78	1043.3	57.1	1.73	7.8	27.0	15.8	16.5	9.2	15.8	1.42	19.0	1.18
V34	12.2	53.54	1041.4	67.3	1.81	8.4	29.9	16.1	16.8	10.3	16.1	1.26	19.3	1.05
V35	13.2	49.55	1038.2	66.4	1.80	8.4	33.3	16.9	17.8	11.0	16.9	0.96	20.3	0.80
V36	14.2	46.13	1035.5	72.7	1.84	8.8	38.8	18.1	19.0	12.4	18.1	0.71	21.7	0.59
V37	15.2	43.16	1033.0	78.9	1.88	9.1	45.3	19.6	20.3	13.9	19.6	0.50	23.5	0.42
V38	16.2	40.55	1030.9	85.0	1.92	9.4	53.7	21.5	22.0	15.8	21.5	0.33	25.9	0.28
V39	12.4	53.29	1041.2	61.5	1.76	8.1	29.5	16.2	17.0	10.0	16.2	1.23	19.5	1.02
V40	12.9	51.25	1039.6	64.5	1.79	8.2	31.3	16.5	17.3	10.5	16.5	1.09	19.8	0.91
V41	13.4	49.37	1038.0	67.6	1.81	8.4	33.7	17.0	17.8	11.1	17.0	0.95	20.4	0.79
V42	14.4	46.00	1035.3	73.9	1.85	8.8	39.1	18.1	18.8	12.4	18.1	0.70	21.7	0.58
V43	15.4	43.07	1033.0	80.0	1.89	9.2	45.6	19.6	20.2	14.0	19.6	0.50	23.5	0.41
V44	16.4	40.49	1030.9	85.8	1.92	9.5	54.4	21.7	22.3	16.0	21.8	0.33	26.1	0.27
V45	17.4	38.21	1029.0	90.3	1.95	9.7	66.0	24.7	24.7	18.4	24.7	0.20	29.6	0.16

#### 4.3 Effect of Currents

The effect of currents on the dynamics of dense jets has been questioned. All simulations have been done with zero current speed, as this is usually the worst case that results in lowest dilutions. According to the Research Activity Panel of the Monterey Bay National Marine Sanctuary, currents in the vicinity of the diffuser are commonly 5 to 10 cm/s and can reach 20 cm/s.

The effect of currents on dense jets is determined by the dimensionless parameter  $u_rF_j$  (Gungor and Roberts 2009) where  $u_r=u_a/u$  is the ratio of the ambient current speed,  $u_a$ , to the jet velocity, u. If  $u_rF_j \ll 1$  the current does not significantly affect the jet; if  $u_rF_j \gg 1$  the jet will be significantly deflected by the current and dilution increases significantly. Gungor and Roberts (2009) investigated the effects of currents on vertical dense jets; experiments on multiport diffusers with 60° nozzles were reported by Abessi and Roberts (2017).

There are no known experiments on horizontal dense jets in flowing currents so we investigated the phenomenon using the UM3 model in Visual Plumes. We simulated the pure brine case, T2 (Table 3) at current speeds of zero, 5, 10, and 20 cm/s. Because of the orientation of the MRWPCA diffuser (see Figure 1 of Roberts 2016) the predominant current direction is expected to be perpendicular to the diffuser axis. The nozzles are perpendicular to the diffuser, so the current direction relative to the individual jets is either counter-flow (jets directly opposing the current), or co-flow (jets in the same direction as the currents.

UM3 was run for all cases. Screen shots of the jet trajectories for counter- and co-flowing jets are shown in Figure 4.

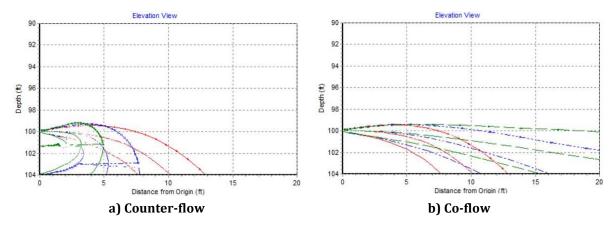


Figure 4. Screen shots of UM3 simulations of dense jet trajectories (Case T2) in counter- and co-flowing currents. Red: zero current; Blue: 10 cm/s; Green: 20 cm/s.

In counter flowing currents, the jets are bent backwards and impact the seabed closer to the diffuser. In co-flowing currents, the jets are advected downstream and impact the seabed farther from the diffuser. The numerical results are summarized in Table 7.

Table 7. UM3 Simulations of Case T2 with Current

Current	Count	er-flow	Co-flow				
Speed (cm/s)	Dilution	Impact distance (ft)	Dilution	Impact distance (ft)			
0	16.2	10	16.2	10			
5	17.3	8	22.6	13			
10	18.9	5	38.4	16			
20	32.6	0	78.0	27			

It can be seen that the effect of the currents is to increase dilution compared to the zero current case. The maximum impact distance from the diffuser occurs with co-flowing currents and increases as the current speed increases. In this case, the maximum impact distance (for  $u_a = 20 \text{ cm/s}$ ) is 27 ft (8.2 m). Clearly, this is much less than the distance to the edge of the BMZ (100 m) so we conclude that neglecting the effect of currents is indeed conservative, and the Ocean Plan regulations will be met for all anticipated currents.

#### 4.4 Dilution of End Gate Check Valve

As discussed in Section 3, it has been proposed to replace the opening in the end gate with a 6-inch Tideflex check valve. We simulated the dilution of this valve for various nozzle angles for the worst case of pure brine, T2 (Table 3). The flow distributions along the diffuser for this case were shown in Figure 3. The exit velocity from the end gate check valve is 9.7 ft/s and the equivalent round diameter is 4.1 inches, yielding a densimetric Froude number,  $F_i = 20.7$ .

The effect of nozzle angle on the dilution of dense jets is discussed in Section 6.2. Using Figure 6, the impact dilutions for various angles were calculated. The results are summarized in Table 8.

The corresponding dilution for the main diffuser nozzles is 15.4 (Table 5). It is therefore apparent that any nozzle angle greater than about 20° will result in dilutions greater than the main diffuser and will meet the BMZ requirements. Dilution is maximized for a 60° nozzle.

Table 8. Effect of Nozzle Angle on Impact Dilution for Flow from End Gate Check Valve for Case T2 (14.08 mgd, 1045.1 kg/m³).

Nozzle angle (Degrees)	Impact dilution
0	8.9
10	12.3
20	18.9
30	25.6
40	31.6
50	35.7
60	36.9

#### 5. BUOYANT DISCHARGE DILUTION

#### 5.1 Introduction

The same procedures and models discussed in Roberts (2016) were used except that all three seasonal profiles were used for each flow scenario to determine the worst-case condition. Inspection of Tables 3 and 4 show that there are 14 cases of buoyant discharges, i.e., the effluent density is less than the receiving water density. Three are for the Project and 11 for the Variant. Two models in the US EPA modeling suite Visual Plumes were used: NRFIELD and UM3. Zero current speed was assumed in all cases.

#### 5.2 Results

The following procedure was used: The internal hydraulics program was first run for each scenario and the average diameter and flow for each nozzle was obtained. UM3 and NRFIELD were then run for each oceanic season.

As was observed in Roberts (2016), for very buoyant cases, the average dilution predicted by UM3 is close to the minimum (centerline) dilution predicted by NRFIELD. They diverge as the effluent becomes only slightly buoyant (i.e. the effluent density approaches the ambient density), with UM3 dilutions being considerably higher.

NRFIELD is based on experiments conducted for parameters typical of domestic wastewater discharges into coastal waters and estuaries. For this situation, dilution and mixing are mainly dependent on the source buoyancy flux with momentum flux playing a minor role. As the effluent density approaches the background density, buoyancy becomes less important and the mixing becomes dominated by momentum. In that situation, NRFIELD continues to give predictions but issues a warning that "The results are extrapolated" when the parameters are outside the range of the original experiments. Table 9 summarizes the results; NRFIELD predictions are only given when they fall within the experimental range on which it is based.

The plume behavior depends strongly on the shape of the density profile (Figure 1) but dilutions are generally very high. The Upwelling profile always gives deepest submergence and lowest dilutions. The plumes are always submerged with the Upwelling and Oceanic profiles but some plumes surface with the weak Davidson stratification. Dilutions are very high for surfacing plumes, up to 842 (Case V12) when the flow is very low.

Table 9. Summary of Dilution Simulations for Buoyant Effluent Scenarios – Project and Variant

Case ID	Season	Efflu	uent cond	litions		Port c	onditions		UM3 si	mulations	NRFIELD simulations		
		Flow (mgd)	Salinity (ppt)	Density (kg/m³)	Flow (gpm)	Diam. (inch)	Velocity (ft/s)	Froude no.	Average dilution	Rise height (centerline) (ft)	Minimum dilution	Rise height (centerline) (ft)	Rise height (top) (ft)
T1	Upwelling	19.88	1.00	999.0	103.7	2.01	10.5	27.9	188	57	179	41	57
	Davidson								327	100	349	100	100
	Oceanic								239	80	238	50	72
T13	Upwelling	29.08	28.54	1021.2	151.6	2.18	13.0	80.6	93	28			
	Davidson								127	57			
	Oceanic								94	27			
T14	Upwelling	33.86	24.63	1018.1	176.4	2.25	14.2	66.7	99	36			
	Davidson								147	76			
	Oceanic								104	41			
V9	Upwelling	22.99	23.26	1017.0	119.6	2.10	11.1	50.3	110	37			
	Davidson								172	75			
	Oceanic								116	42			
V10	Upwelling	28.77	18.75	1013.3	149.9	2.18	12.9	48.3	118	44	100	39	41
	Davidson								202	96	215	97	100
	Oceanic								132	58	134	57	59
V11	Upwelling	1.17	5.80	1002.6	6.5	0.71	5.3	25.4	495	30			
	Davidson								974	48			
	Oceanic								549	35			
V12	Upwelling	1.57	4.53	1001.6	8.4	0.81	5.2	23.1	457	31	385	25	32
	Davidson								842	50	652	33	45
	Oceanic								520	37	460	28	36

**Table 9. Summary of Dilution Simulations for Buoyant Effluent Scenarios - Project and Variant** 

Case ID	Season	Season Effluent conditions				Port c	onditions		UM3 si	mulations	NRFIELD simulations		
		Flow (mgd)	Salinity (ppt)	Density (kg/m³)	Flow (gpm)	Diam. (inch)	Velocity (ft/s)	Froude no.	Average dilution	Rise height (centerline) (ft)	Minimum dilution	Rise height (centerline) (ft)	Rise height (top) (ft)
V13	Upwelling	4.17	2.20	999.9	21.7	1.24	5.8	19.9	324	39	301	30	40
	Davidson								547	66	687	51	74
	Oceanic								376	47	378	35	47
V14	Upwelling	24.87	1.04	999.0	129.6	2.11	11.9	30.9	174	60	165	56	59
	Davidson								290	100	301	67	100
	Oceanic								223	86	235	55	81
V15	Upwelling	25.87	1.03	999.0	134.8	2.13	12.1	31.4	172	60	163	57	59
	Davidson								281	100	293	67	100
	Oceanic								221	87	232	56	82
V24	Upwelling	17.16	31.23	1023.4	89.3	1.94	9.7	87.3	91	20			
	Davidson								131	46			
	Oceanic								91	18			
V25	Upwelling	21.16	25.48	1018.7	109.8	2.03	10.9	56.2	107	33			
	Davidson								159	65			
	Oceanic								111	37			
V26	Upwelling	26.08	20.82	1015.0	135.6	2.13	12.2	49.7	115	41			_
	Davidson								191	89			
	Oceanic								124	49			
V31	Upwelling	25.85	20.95	1015.1	134.4	2.13	12.1	49.5	115	41			
	Davidson								191	89			
	Oceanic								124	49			

#### 6. DILUTION MITIGATION – EFFECT OF NOZZLE ANGLE

#### 6.1 Introduction

Orienting the nozzles upwards from horizontal will increase the dilution of brine mixtures that are more dense than the receiving water. For buoyant effluents, it will decrease dilution slightly. In this section, we investigate the effect on dilution of varying nozzle orientations for dense and buoyant effluents.

#### 6.2 Dense Effluents

The effect of nozzle angle on dense jets has been recently investigated by Abessi and Roberts (2015). Figure 5 shows central plane tracer concentrations (inverse of dilution) obtained by laser-induced fluorescence for dense jets with angles ranging from 15° to 85°. For very shallow angles, e.g. 15°, the jet impacts the bed quickly, reducing dilution. For steep angles, e.g. 85°, the trajectory is also truncated and the jet falls back on itself, which also reduces dilution.

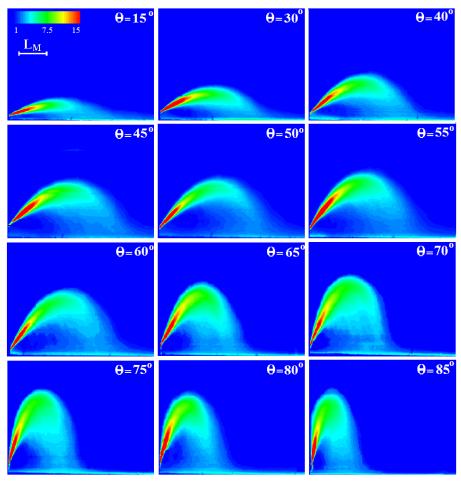


Figure 5. Central plane tracer concentrations for dense jets at various nozzle angles from 15° to 85°. After Abessi and Roberts (2015).

The optimum angle for dilution is  $60^{\circ}$ . This is illustrated by Figure 6, which shows the variation with nozzle angle on normalized impact dilution  $(S_i/F_j)$  and near field dilution  $(S_n/F_j)$  for single jets.

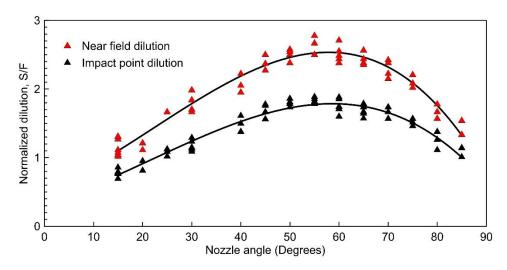


Figure 6. Effect of nozzle angle on normalized dilution of dense jets.

After Abessi and Roberts (2015).

Impact dilutions were computed for the "worst-case" of brine only (T2, for conditions, see Table 3) using Figure 6. The results are tabulated in Table 10 and plotted in Figure 7. The effect of the height of the nozzle above the seabed, z, is determined by the dimensionless parameter  $z/dF_j$ , where d is the nozzle diameter. For Monterey, the nozzles are four feet above the seabed, so for case T2 we have  $z/dF_j \approx 0.93$ . The experiments of Abessi and Roberts were done with nozzles closer to the bed, with  $h/dF_j$  ranging from 0.12 to 0.39, so actual dilutions are expected to be higher than predicted in Table 10.

Dilution calculations with UM3 are also shown for completeness with other simulations. However, it is known that UM3 considerably underestimates dilutions for inclined jets (Palomar et al. 2012), therefore only the Abessi and Roberts results are used.

Table 10. Effect of Nozzle Angle on Dense Jets Case T2. (for conditions, see Table 3)

		Di	Dilution predictions			At i	mpact	At BMZ	
Case ID	Nozzle angle	Cederwall	Abessi and Roberts (2015a)		UM3	Dilution	Salinity increment	Dilution	Salinity increment
	(deg)	Impact	Impact	Near field	Impact		(ppt)		(ppt)
T2	0	15.4	-	-	16.1	15.4	1.61	18.5	1.34
	10	-	16.9	25.2	18.7	16.9	1.47	20.3	1.22
	20	-	25.9	37.8	20.9	25.9	0.95	31.1	0.80
	30	-	35.3	50.8	22.8	35.3	0.70	42.3	0.59
	40	-	43.4	62.3	24.3	43.4	0.57	52.1	0.48
	50	-	49.0	70.0	24.5	49.0	0.50	58.9	0.42
	60	-	50.7	71.9	24.4	50.7	0.49	60.9	0.41

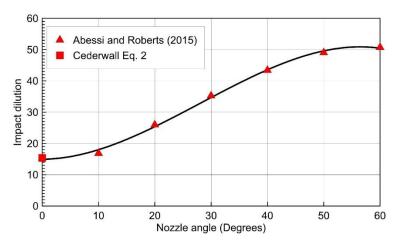


Figure 7. Effect of nozzle angle on dilution of dense jets, case T2.

Increasing the angle from horizontal (0°) to  $60^{\circ}$  increases dilution considerably, from 15 to 51. A  $30^{\circ}$  angle more than doubles the dilution compared to the horizontal jets.

The dilution at the BMZ is computed as 120% of the impact dilution. Note that in Table 10 the increase in dilution from the impact point to the end of the near field is more than 20%. This result, however, is for a single jet, and the increase for merged jets is less than this, and is conservatively assumed to be 20%, as explained in Roberts (2016).

#### 6.3 Buoyant Effluents

Diffusers for buoyant effluents are usually designed with horizontal nozzles to maximize the length of the jet trajectory up to the terminal rise height, and therefore maximize dilution. Inclining the nozzles upwards will usually reduce dilution, although for very buoyant discharges in deep water the effect may be minimal. This is because the dynamics are then buoyancy dominated and the effect of momentum flux and therefore nozzle orientation is unimportant.

For very buoyant discharges, NRFIELD is the preferred model. NRFIELD, however, assumes the nozzles to be horizontal, so UM3 was used to assess the effect of nozzle orientation.

Simulations were run with UM3 for selected cases to bracket the expected results. The chosen cases were for the project scenarios (Table 3): T1 (mainly pure secondary effluent) and T13 (brine plus high secondary effluent). The latter case is only slightly buoyant and resulted in the lowest dilution of the buoyant cases. The simulations were run only for the oceanic conditions that gave the highest dilutions (Upwelling) and lowest dilutions (Davidson).

The results are summarized in Table 11 and plotted in Figure 8.

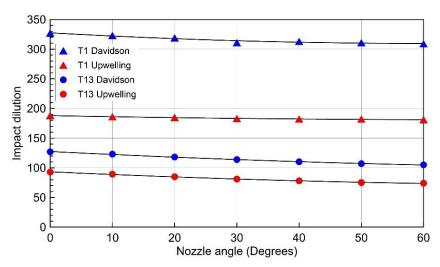


Figure 8. Effect of nozzle angle on dilution for selected buoyant discharge scenarios.

The results are insensitive to nozzle angle, especially for the very buoyant case of mainly pure secondary effluent (T1). Changing the nozzles from horizontal to 60° for the Davidson condition reduces dilution from 327 to 309, and for Upwelling condition from 188 to 181. For case T13 the corresponding reductions are from 127 to 105 and from 93 to 75. The percentage reductions for T13 are greater due to the increased effect of momentum flux, and therefore nozzle angle. More modest changes in orientation result in lesser effect; for a 30° nozzle the dilution reductions range from 3 to 13%.

 Table 11. Effect of nozzle Angle on Dilution for Selected Buoyant Effluent Scenarios

Case ID	Oceanic Season	Effluent conditions			Nozzle angle	UM3 s	imulations
		Flow (mgd)	Salinity (ppt)	Density	(deg)	Average dilution	Rise height (centerline) (ft)
T1	Upwelling	19.88	1.00	999.0	0	188	57
					10	186	58
					20	185	58
					30	183	59
					40	182	60
					50	182	61
					60	181	61
T1	Davidson	19.88	1.00	999.0	0	327	100
					10	323	100
					20	319	100
					30	311	100
					40	313	100
					50	311	100
					60	309	100
T13	Upwelling	29.08	28.54	1021.2	0	93	28
					10	89	29
					20	85	30
					30	81	31
					40	78	33
					50	75	35
					60	74	37
T13	Davidson	29.08	28.54	1021.2	0	127	57
					10	123	57
					20	118	57
					30	114	58
					40	110	60
					50	107	61
					60	105	63

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## **APPENDIX A. DENSITY PROFILES**

The seasonally averaged density profiles assumed for modeling purposes are summarized below.

Depth	De	ensity (kg/m³)	)
(m)	Upwelling	Davidson	Oceanic
1	1025.1	1024.8	1024.8
3	1025.1	1024.8	1024.8
5	1025.1	1024.8	1024.8
7	1025.2	1024.8	1024.8
9	1025.2	1024.8	1024.8
11	1025.3	1024.8	1024.8
13	1025.4	1024.8	1024.9
15	1025.4	1024.8	1024.9
17	1025.5	1024.8	1024.9
19	1025.6	1024.9	1024.9
21	1025.6	1024.9	1025.0
23	1025.7	1024.9	1025.0
25	1025.7	1024.9	1025.0
27	1025.8	1024.9	1025.1
29	1025.8	1024.9	1025.1
31	1025.8	1024.9	1025.2
33	1025.9	1024.9	1025.2
35	1025.9	1024.9	1025.3

#### **APPENDIX B. ADDITIONAL SCENARIOS**

In a memorandum from Trussell Technologies, Inc. dated July 21, 2017, dilution simulations for some additional scenarios were requested. They were contained in table 9 of that memo, which is reproduced below.

Table 9 – Proposed Flow Scenarios for Additional Modeling

RTP Secondary GWR Cocan									
No.	Effluent	Hauled Waste	Concentrate	Desal Brine	Condition <sup>1</sup>				
MPWSP with high Desal Brine flow									
1	6	0		16.31	All				
2	7	0		16.31	All				
3	8	0		16.31	All				
4	9	0		16.31	All				
5	10	0		16.31	All				
6	12	0		16.31	All				
7	14	0		16.31	All				
8	16	0	ì	16.31	All				
Varia	nt with Desal Off								
9	8	0	1.17	0	All				
Varia	nt with GWR Cond	centrate off and hi	igh Desal Brine flo	w					
10	6	0		11.24	All				
11	7	0	/	11.24	All				
12	8	0	7-	11.24	All				
13	9	0		11.24	All				
14	10	0		11.24	All				
15	12	0		11.24	All				
16	14	0		11.24	All				
17	16	0		11.24	All				
Varia	int with high Desa	l Brine flow							
18	6	0	1.17	11.24	All				
19	7	0	1.17	11.24	All				
20	8	0	1.17	11.24	All				
21	9	0	1.17	11.24	All				
22	10	0	1.17	11.24	All				
23	12	0	1.17	11.24	All				
24	14	0	1.17	11.24	All				
25	16	0	1.17	11.24	All				
1: All	ocean conditions	should be modeled	d when using the l	JM3 and NRFIELD r	models. For				

<sup>1:</sup> All ocean conditions should be modeled when using the UM3 and NRFIELD models. For dense plumes that are modeled with Cederwall and UM3, the worst-case ocean condition should be used.

The flow conditions for these additional scenarios are summarized in Table B1. Dilutions were simulated according to the same procedures as outlined in Sections 4 and 5. The results for dense discharges are summarized in Table B2 and for buoyant discharges in Table B3.

**Table B1. Additional Modeled Discharge Scenarios** 

Case ID	Scenario	Constituent flows (mgd)				Co	ombined e	ffluent
		Brine	Secondary effluent	GWR	Hauled brine	Flow (mgd)	Salinity (ppt)	Density (kg/m³)
AT1	MPWSP with high	16.31	6.00	0.00	0.0	22.31	42.78	1032.7
AT2	desal brine flow	16.31	7.00	0.00	0.0	23.31	40.98	1031.3
AT3		16.31	8.00	0.00	0.0	24.31	39.33	1030.0
AT4		16.31	9.00	0.00	0.0	25.31	37.81	1028.7
AT5		16.31	10.00	0.00	0.0	26.31	36.40	1027.6
AT6		16.31	12.00	0.00	0.0	28.31	33.89	1025.6
AT7		16.31	14.00	0.00	0.0	30.31	31.70	1023.8
AT8		16.31	16.00	0.00	0.0	32.31	29.79	1022.2
AV9	Variant with desal off	0.00	8.00	1.17	0.0	9.17	1.44	999.3
AV10	Variant with GWR	11.24	6.00	0.00	0.0	17.24	38.24	1029.1
AV11	concentrate off and	11.24	7.00	0.00	0.0	18.24	36.19	1027.4
AV12	high desal brine	11.24	8.00	0.00	0.0	19.24	34.35	1025.9
AV13	flow	11.24	9.00	0.00	0.0	20.24	32.69	1024.6
AV14		11.24	10.00	0.00	0.0	21.24	31.19	1023.4
AV15		11.24	12.00	0.00	0.0	23.24	28.58	1021.3
AV16		11.24	14.00	0.00	0.0	25.24	26.38	1019.5
AV17		11.24	16.00	0.00	0.0	27.24	24.50	1018.0
AV18	Variant with high	11.24	6.00	1.17	0.0	18.41	36.18	1027.4
AV19	desal brine flow	11.24	7.00	1.17	0.0	19.41	34.36	1025.9
AV20		11.24	8.00	1.17	0.0	20.41	32.71	1024.6
AV21		11.24	9.00	1.17	0.0	21.41	31.22	1023.4
AV22		11.24	10.00	1.17	0.0	22.41	29.87	1022.3
AV23		11.24	12.00	1.17	0.0	24.41	27.48	1020.4
AV24		11.24	14.00	1.17	0.0	26.41	25.46	1018.7
AV25		11.24	16.00	1.17	0.0	28.41	23.73	1017.3

**Table B2. Summary of Dilution Simulations for Dense Additional Scenarios** 

Case ID	Effluent conditions		litions	Port conditions				Prediction	ıs	At impact (ZID)		At BMZ		
	Flow (mgd)	Salinity (ppt)	Density (kg/m3)	Flow (gpm)	Diam. (inch)	Velocity (ft/s)	Froude no.	Dilution	Dilution	Impact distance (ft)	Dilution	Salinity increment (ppt)	Dilution	Salinity increment (ppt)
AT1	22.3	42.78	1032.7	116.0	2.06	11.2	57.9	22.1	21.4	16.6	21.4	0.42	25.7	0.35
AT2	23.3	40.98	1031.3	120.7	2.08	11.4	60.7	22.8	22.8	18.1	22.8	0.34	27.4	0.28
AT3	24.3	39.33	1030.0	125.5	2.10	11.6	69.2	25.0	24.5	19.8	24.5	0.24	29.4	0.20
AT4	25.3	37.81	1028.7	130.3	2.11	12.0	81.4	28.2	27.2	22.3	27.2	0.16	32.6	0.14
AT5	26.3	36.40	1027.6	135.1	2.13	12.2	97.8	32.5	30.2	25.3	30.2	0.10	36.2	0.08
AT6	28.3	33.89	1025.6	144.7	2.16	12.7	195.3	58.6	44.9	39.0	44.9	0.01	53.9	0.01
AV10	17.2	38.24	1029.1	89.4	1.94	9.7	66.0	24.7	24.6	18.2	24.6	0.20	29.5	0.17
AV11	18.2	36.19	1027.4	93.6	1.96	10.0	86.1	30.0	28.8	22.0	28.8	0.10	34.6	0.08
AV12	19.2	34.35	1025.9	98.4	1.99	10.2	133.0	42.4	37.4	29.7	37.4	0.03	44.9	0.02
AV18	18.4	36.18	1027.4	94.7	1.97	10.0	86.4	30.0	28.7	22.0	28.7	0.10	34.4	0.08
AV19	19.4	34.36	1025.9	99.5	1.99	10.3	135.0	42.9	37.6	29.8	37.6	0.03	45.1	0.02

 Table B3. Summary of Dilution Simulations for Buoyant Additional Scenarios

Case ID	Season	Efflu	uent cond	itions		Port o	onditions		UM3 simulations		NRFIE	LD simulati	ons
		Flow (mgd)	Salinity (ppt)	Density	Flow (gpm)		Velocity (ft/s)	Froude no.	Average dilution	Rise height centerline (ft)	Minimum dilution	Rise height centerline (ft)	Rise height top (ft)
AT7	Upwelling	30.31	31.70	1023.8	157.8	2.20	13.3	123.3	88	19			
	Davidson								120	45			
	Oceanic								90	17			
AT8	Upwelling	32.31	29.79	1022.2	179.2	2.26	14.3	98.6	90	26			
	Davidson								118	53			
	Oceanic								88	23			
AV9	Upwelling	9.17	1.44	999.3	55.9	1.72	7.7	22.4	244	48	234	35	48
	Davidson								467	100	584	67	100
	Oceanic								309	66	315	42	60
AV13	Upwelling	20.24	32.69	1024.6	108.9	2.03	10.8	133.6	91	17			
	Davidson								100	15			
	Oceanic								138	41			
AV14	Upwelling	21.24	31.19	1023.4	114.9	2.06	11.1	96.5	88	20			
	Davidson								124	47			
	Oceanic								88	18			
AV15	Upwelling	23.24	28.58	1021.3	126.9	2.08	12.0	76.2	96	28			
	Davidson								133	55			
	Oceanic								95	26			
AV16	Upwelling	25.24	26.38	1019.5	138.7	2.11	12.7	68.1	100	32			
	Davidson								144	64			
	Oceanic								104	35			
AV17	Upwelling	27.24	24.50	1018.0	151.1	2.15	13.4	63.6	103	36			
	Davidson								155	73			
	Oceanic								109	41			

 Table B3. Summary of Dilution Simulations for Buoyant Additional Scenarios

Case ID	Season	Efflo	uent cond	litions		Port o	onditions		UM3 sir	nulations	NRFIE	LD simulation	ons
		Flow (mgd)	Salinity (ppt)	Density	Flow (gpm)	Diam. (inch)	Velocity (ft/s)	Froude no.	Average dilution	Rise height centerline (ft)	Minimum dilution	Rise height centerline (ft)	Rise height top (ft)
AV20	Upwelling	20.41	32.71	1024.6	110.1	2.02	11.0	136.9	92	17			
	Davidson								139	41			
	Oceanic								101	15			
AV21	Upwelling	21.41	31.22	1023.4	116.1	2.02	11.6	102.6	91	20			
	Davidson								126	64			
	Oceanic								91	18			
AV22	Upwelling	22.41	29.87	1022.3	116.4	2.06	11.2	81.3	93	24			
	Davidson								128	51			
	Oceanic								90	21			
AV23	Upwelling	24.41	27.48	1020.4	134.0	2.10	12.4	71.8	98	30			
	Davidson								138	59			
	Oceanic								101	31			
AV24	Upwelling	26.41	25.46	1018.7	145.8	2.14	13.0	65.4	101	34			
	Davidson								149	68			
	Oceanic								106	38			
AV25	Upwelling	28.4	23.73	1017.3	157.6	2.17	13.7	62.3	105	37			
	Davidson								161	78			
	Oceanic								110	43			

### APPENDIX C. EFFECT OF NOZZLE ANGLE ON DILUTION

In order to further investigate the effect of nozzle angle on dilution for various scenarios, additional model runs were undertaken for horizontal and 60° nozzles. Most were previously analyzed cases, whose flow properties are given in Tables 3 and 4. Table C1 summarizes the properties of the new cases.

Dilutions were simulated according to the same procedures as outlined in Sections 4 and 5. Table C2 summarizes the results for dense discharges. For the buoyant cases, only Upwelling and Davidson conditions were run to bracket the expected results. Because NRFIELD only allows for horizontal nozzles, only results for UM3 are shown in Table C3.

**Table C1. Further Modeled Discharge Scenarios** 

Case ID	Scenario		Constituent flo	gd)	Com	bined et	fluent	
		Brine	Secondary effluent	GWR	Hauled brine	Flow (mgd)	Salinity (ppt)	Density (kg/m³)
1	GWR only	0.00	0.00	1.17	0.0	1.17	5.80	1002.6
5		0.00	0.40	1.17	0.0	1.57	4.53	1001.6
7		0.00	0.60	1.17	0.0	1.77	4.11	1001.3
12		0.00	2.00	1.17	0.0	3.17	2.65	1000.2
16		0.00	4.00	1.17	0.0	5.17	1.93	999.7
17		0.00	4.50	1.17	0.0	5.67	1.83	999.6
18		0.00	5.00	1.17	0.0	6.17	1.75	999.5
32		0.00	23.40	1.17	0.0	24.57	1.04	999.0
New	Variant with normal flows and GWR offline	8.99	10.00	0.00	0.0	18.99	27.99	1020.8
New2		8.99	6.50	1.17	0.0	16.66	32.14	1024.1
New3		8.99	7.00	1.17	0.0	17.16	31.23	1023.4

**Table C2. Summary of Dilution Simulations for Dense Scenarios** 

		Efflu	uent cond	ditions		Port c	onditions	;	Impact o	dilution prediction	ons	At impa	ct (ZID)	AT E	BMZ
Case ID	Nozzle angle (deg)	Flow (mgd)	Salinity (ppt)	Density (kg/m³)	Flow (gpm)	Diam. (in.)	Velocity (ft/s)	Froude no.	Cederwall	Abessi & Roberts 2015a	UM3	Dilution	Salinity incr- ement (ppt)	Dilution	Salinity incr- ement (ppt)
T5	0	17.08	48.04	1037.0	86.2	1.92	9.6	38.6	17.7	-	18.5	17.7	0.83	21.2	0.69
	60	17.08	48.04	1037.0	86.2	1.92	9.6	38.6	-	68.9	-	68.9	0.21	82.6	0.18
T10	0	22.08	37.34	1028.3	114.2	2.05	11.1	80.6	28.2	-	27.5	27.5	0.15	33.0	0.12
	60	22.08	37.34	1028.3	114.2	2.05	11.1	80.6	-	143.7	-	143.7	0.03	172.4	0.02
T20	0	21.41	44.73	1034.3	110.3	2.04	10.8	48.1	19.6	-	20.4	19.6	0.58	23.6	0.48
	60	21.41	44.73	1034.3	110.3	2.04	10.8	48.1	-	85.7	-	85.7	0.13	102.8	0.11
AT6	0	28.31	33.89	1025.6	144.7	2.16	12.7	194.0	58.3	-	44.9	44.9	0.01	53.9	0.01
	60	28.31	33.89	1025.6	144.7	2.16	12.7	194.0	-	345.6	-	345.6	0.00	414.8	0.00
V2	0	9.99	52.48	1040.6	55.8	1.72	7.7	28.9	16.3	-	16.9	16.3	1.17	19.6	0.98
	60	9.99	52.48	1040.6	55.8	1.72	7.7	28.9	-	51.5	-	51.5	0.37	61.9	0.31
V4	0	11.99	43.86	1033.6	61.5	1.76	8.1	40.3	18.8	-	19.8	18.8	0.56	22.6	0.47
	60	11.99	43.86	1033.6	61.5	1.76	8.1	40.3	-	71.8	-	71.8	0.15	86.1	0.12
V6	0	13.99	37.70	1028.6	73.4	1.85	8.8	64.3	24.6	-	24.9	24.6	0.18	29.5	0.15
	60	13.99	37.70	1028.6	73.4	1.85	8.8	64.3	-	114.6	-	114.6	0.04	137.5	0.03
V8	0	15.99	33.09	1024.9	76.3	1.87	8.9	382.9	110.2	-	67.6	67.6	0.00	81.1	0.00
	60	15.99	33.09	1024.9	76.3	1.87	8.9	382.9	-	682.3	-	682.3	0.00	818.8	0.00
V16	0	10.16	52.19	1040.3	56.8	1.72	7.8	29.7	16.5	-	17.3	16.5	1.14	19.8	0.95
	60	10.16	52.19	1040.3	56.8	1.72	7.8	29.7	-	52.9	-	52.9	0.36	63.5	0.30
V17	0	11.16	47.59	1036.6	56.1	1.72	7.8	33.6	17.4	-	18.3	17.4	0.82	20.9	0.68
	60	11.16	47.59	1036.6	56.1	1.72	7.8	33.6	-	59.9	-	59.9	0.24	71.9	0.20
V19	0	13.16	40.48	1030.9	68.3	1.81	8.5	50.3	21.1	-	21.8	21.1	0.34	25.4	0.28
	60	13.16	40.48	1030.9	68.3	1.81	8.5	50.3	-	89.6	-	89.6	0.08	107.6	0.07
V22	0	15.46	34.57	1026.1	79.8	1.89	9.1	114.2	37.7	-	34.3	34.3	0.04	41.2	0.03
	60	15.46	34.57	1026.1	79.8	1.89	9.1	114.2	-	203.5	-	203.5	0.01	244.2	0.01
V23	0	16.16	33.11	1024.9	83.3	1.91	9.3	395.8	113.5	-	68.5	68.5	0.00	82.2	0.00
	60	16.16	33.11	1024.9	83.3	1.91	9.3	395.8	-	705.4	-	705.4	0.00	846.5	0.00

**Table C2. Summary of Dilution Simulations for Dense Scenarios** 

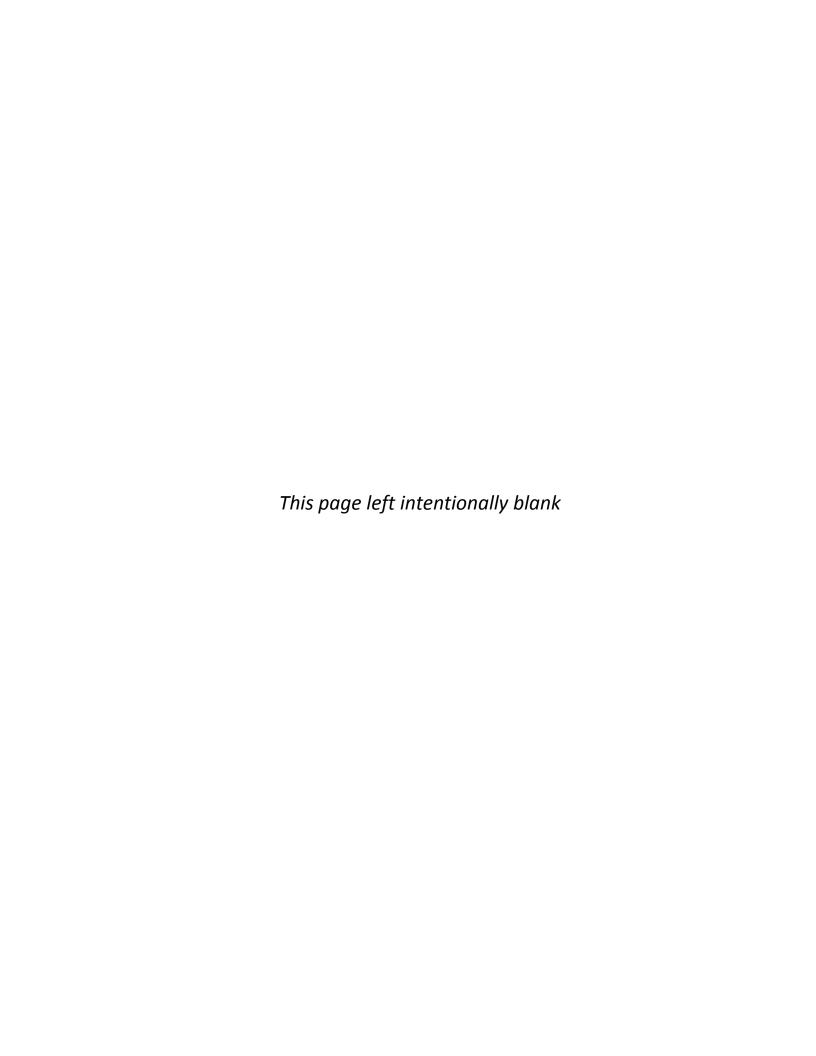
		Effluent conditions			Port o	onditions	3	Impact o	lilution prediction	ons	At impa	ct (ZID)	AT BMZ		
Case ID	Nozzle angle (deg)	Flow (mgd)	Salinity (ppt)	Density (kg/m³)	Flow (gpm)	Diam. (in.)	Velocity (ft/s)	Froude no.	Cederwall	Abessi & Roberts 2015a	UM3	Dilution	Salinity incr- ement (ppt)	Dilution	Salinity incr- ement (ppt)
V32	0	11.24	58.23	1045.2	63.3	1.78	8.2	26.5	15.4	-	16.1	15.4	1.61	18.5	1.34
	60	11.24	58.23	1045.2	63.3	1.78	8.2	26.5	-	47.2	-	47.2	0.53	56.6	0.44
V36	0	14.24	46.13	1035.5	72.7	1.84	8.8	38.8	18.1	-	19.0	18.1	0.71	21.7	0.59
	60	14.24	46.13	1035.5	72.7	1.84	8.8	38.8	-	69.1	-	69.1	0.19	82.9	0.15
AV10	0	17.24	38.24	1029.1	89.4	1.94	9.7	65.9	24.7	-	27.5	24.7	0.20	29.6	0.17
	60	17.24	38.24	1029.1	89.4	1.94	9.7	65.9	-	117.4	-	117.4	0.04	140.9	0.03
AV12	0	19.24	34.35	1025.9	98.4	1.99	10.2	132.4	42.2	-	37.4	37.4	0.03	44.9	0.02
	60	19.24	34.35	1025.9	98.4	1.99	10.2	132.4	-	235.9	-	235.9	0.00	283.1	0.00
V39	0	12.41	53.29	1041.2	61.5	1.76	8.1	29.5	16.2	-	17.0	16.2	1.23	19.5	1.02
	60	12.41	53.29	1041.2	61.5	1.76	8.1	29.5	-	52.6	-	52.6	0.38	63.1	0.32
V43	0	15.41	43.07	1033.0	80.0	1.89	9.2	45.6	19.6	-	20.2	19.6	0.50	23.5	0.41
	60	15.41	43.07	1033.0	80.0	1.89	9.2	45.6	-	81.2	-	81.2	0.12	97.5	0.10
V45	0	17.41	38.21	1029.0	90.3	1.95	9.7	66.0	24.7	-	18.4	18.4	0.26	22.1	0.22
	60	17.41	38.21	1029.0	90.3	1.95	9.7	66.0	-	117.7	-	117.7	0.04	141.2	0.03
AV19	0	19.41	34.36	1025.9	99.5	1.99	10.3	134.4	42.8	-	37.6	37.6	0.03	45.1	0.02
	60	19.41	34.36	1025.9	99.5	1.99	10.3	134.4	-	239.4	-	239.4	0.00	287.3	0.00

**Table C3. Summary of Dilution Simulations for Buoyant Further Scenarios** 

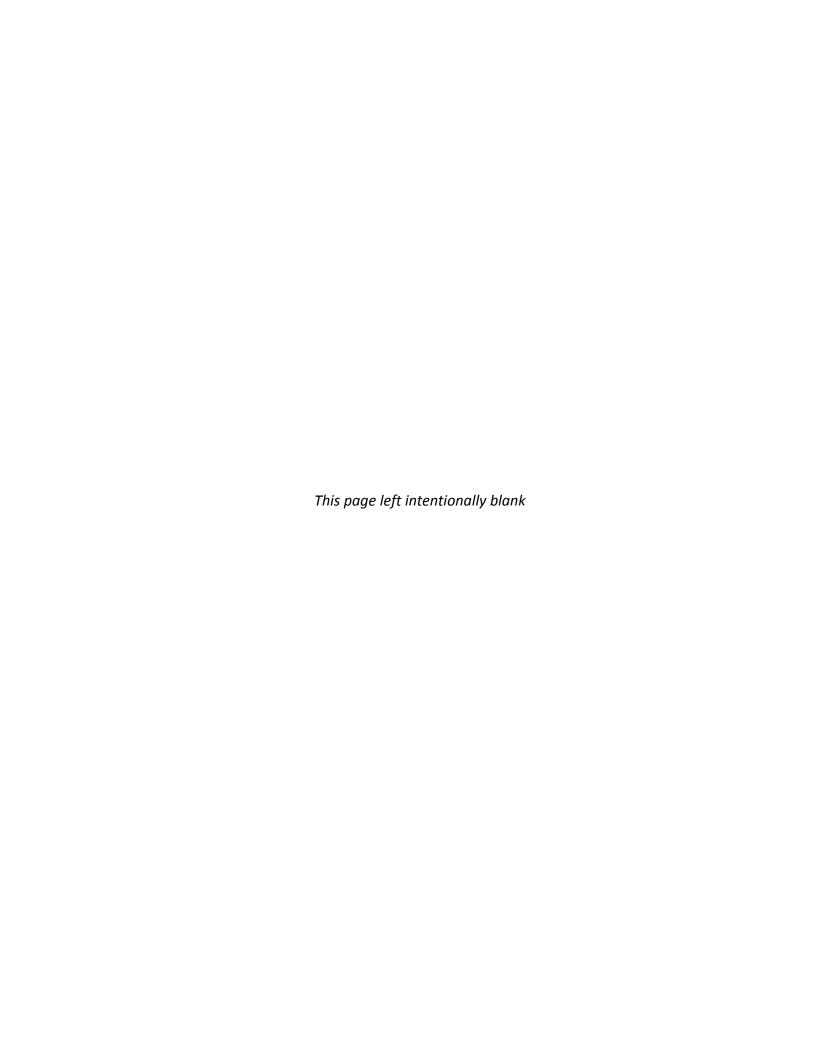
		Efflo	uent cond	litions		Po	rt cond	itions		UM3 si	imulations
Case ID	Season	Flow (mgd)	Salinity (ppt)	Density (kg/m³)	Nozzle angle (deg)	Flow (gpm)	Diam. (inch)	Velocity (ft/s	Froude no.	Average dilution	Rise height (centerline) (ft)
New	Upwelling	18.99	27.99	1020.8	0	98.5	1.99	10.2	62.8	101	28
					60					82	34
	Davidson				0					145	55
					60					123	58
V25	Upwelling	21.16	25.48	1018.7	0	109.8	2.03	10.9	56.2	107	33
					60					91	39
	Davidson				0					159	65
					60					141	70
AV14	Upwelling	21.24	31.19	1023.4	0	114.9	2.06	11.1	96.5	88	20
					60					66	28
	Davidson				0					124	47
					60					94	49
AV21	Upwelling	21.41	31.22	1023.4	0	116.1	2.02	11.6	102.6	91	20
					60					68	30
	Davidson				0					126	64
					60					96	49
1	Upwelling	1.17	5.80	1002.6	0	6.8	0.71	5.5	26.6	499	29
					60					488	30
	Davidson				0					987	S
					60					949	S
5	Upwelling	1.57	4.53	1001.6	0	8.1	0.79	5.3	23.7	461	31
					60					447	32
	Davidson				0					853	50
					60					817	50
7	Upwelling	1.77	4.11	1001.3	0	9.3	0.85	5.3	22.6	443	32
					60					428	33
	Davidson				0					800	S
					60					768	S

**Table C3. Summary of Dilution Simulations for Buoyant Further Scenarios** 

		Effl	uent cond	litions		Po	rt cond	itions		UM3 si	mulations
Case ID	Season	Flow (mgd)	Salinity (ppt)	Density (kg/m³)	Nozzle angle (deg)	Flow (gpm)	Diam. (inch)	Velocity (ft/s	Froude no.	Average dilution	Rise height (centerline) (ft)
12	Upwelling	3.17	2.65	1000.2	0	16.5	1.11	5.5	20.1	359	36
					60					347	37
	Davidson				0					609	59
					60					586	59
16	Upwelling	5.17	1.93	999.7	0	26.9	1.35	6.0	19.9	300	51
					60					291	41
	Davidson				0					517	S
					60					507	S
17	Upwelling	5.67	1.83	999.6	0	29.6	1.40	6.2	19.9	290	S
					60					282	S
	Davidson				0					509	S
					60					504	S
18	Upwelling	6.17	1.75	999.5	0	32.3	1.44	6.4	20.2	282	S
					60					274	S
	Davidson				0					506	S
					60					510	S
32	Upwelling	24.57	1.04	999.0	0	128.0	2.10	11.9	30.9	175	S
					60					168	S
	Davidson				0					291	S
					60					276	S
New2	Upwelling	16.66	32.14	1024.1	0	86.1	1.92	9.5	103.5	92	18
					60					65	26
	Davidson				0					131	43
					60					95	46
New3	Upwelling	17.16	31.23	1023.4	0	89.0	1.94	9.7	87.0	91	20
					60					69	29
	Davidson				0					131	46
					60					102	48



**Appendix D: Trussell Tech September 2017 Comparison of Dilution Results** 





### DRAFT COMMUNICATION

### **Comparison of Dilution Results**

**Draft Date:** September 12, 2017

**Final Date:** 

**Author:** Elaine Howe, P.E. (NM)

Brie Webber, P.E.

**To:** Denise Duffy (DDA)

Denise Conners (LWA)

**Subject:** Impact of larger RO concentrate discharge from 5 MGD AWPF on ocean

dilution

The following communication documents the changes to the estimated minimum probable dilution (D<sub>m</sub>) values determined during the various Ocean Plan compliance assessments that have been conducted for the GWR Project, MPWPSP, and Project Variant.

### **GWR Project**

The original analysis documented in the February 2015 Ocean Plan Compliance Assessment for the Pure Water Monterey Groundwater Replenishment Project (Appendix U1 of the Final Consolidated EIR, January 2016) assumed there were 120 ports open along the diffuser. This analysis also used ambient ocean (receiving water) profile data from the Monterey Bay Aquarium Research Institute (MBARI) at station C1, which is approximately five miles northwest of MRWPCA's existing ocean outfall. Data examined from this site was collected between 2002 – 2012, and a single representative profile for each ocean condition (Davidson, Oceanic, Upwelling) was selected. The D<sub>m</sub> values reported in Table 1 represent the lowest D<sub>m</sub> values calculated for each discharge flow scenario, with the ambient ocean condition varying depending on which condition produced the lowest D<sub>m</sub>. For additional information on modeling assumptions, refer to the FlowScience technical memoranda discussed in Appendix T of the Final Consolidated EIR for the Pure Water Monterey (PWM) Groundwater Replenishment Project (January 2016).

An addendum to the February 2015 Ocean Plan Compliance Assessment report was published in April 2015 (Appendix U2 of the PWM Final Consolidated EIR, January 2016) and included additional flow scenarios as well as modifications to the modeling assumptions. For the GWR Project, the model assumptions were updated to assume 130 open diffuser ports instead of 120 ports, which reflects current outfall conditions. This change increased the estimated minimum probable dilution. The most recent September 2017 modeling, done in relation to the larger 5

MGD AWPF, also considered 130 ports open (i.e., 129 existing ports plus the open end gate replaced with one diffuser port).

This most recent September 2017 GWR Project Ocean Plan Compliance Assessment for the larger, 5 MGD AWPF considered updated modeling assumptions as follows:

- The GWR RO concentrate flow was increased from 0.94 MGD to 1.17 MGD, in relation to increasing purified water production capacity from 4 MGD to 5 MGD.
- The open diffuser end gate was modeled with one 6-inch Tideflex valve
- The 0.1 MGD of blended hauled waste was not included in the analysis
- The ambient ocean profile data was updated using data collected between 2014 and 2017 in the vicinity of the outfall.

The original COP compliance analyses for the GWR Project (February 2015 and April 2015) modeled the end of the existing ocean outfall as an open pipe, which is the current configuration of the outfall. The September 2017 modeling work assumed that a 6-inch Tideflex valve was installed on the end of the outfall; this modification will occur prior to any discharge of RO concentrate.

The 0.1 MGD blended hauled waste, defined as up to 0.03 MGD of hauled waste mixed with secondary effluent (in a pond prior to discharge) for a maximum flow of 0.1 MGD, was not included in the updated analysis for simplicity. A sensitivity analysis was conducted to determine the impacts of hauled waste on the modeled  $D_m$  results. It was concluded that neither the flow nor TDS from the addition of hauled waste had a significant impact on the modeled  $D_m$  result, and was therefore excluded from the  $D_m$  calculation.

Starting in February of 2014, monthly conductivity-temperature-depth (CTD) water column profiles have been collected at four locations offshore of Marina, California adjacent to MRWPCA's ocean outfall. This work, funded by California American Water, has been done to establish a baseline ocean condition prior to changes in outfall discharge. Using this updated data, density profiles were averaged by season to obtain representative profiles for the dilution modeling included in the September 2017 COP compliance assessment report (Trussell Technologies, Inc.).

The previous dilution analysis conducted by FlowScience (November 2014 and April 2015) for the 2015 reports (included in the 2016 PWM Final Consolidated EIR) was performed using a semi-empirical model and the EPA's Visual Plumes method. The updated analysis (September 2017) used for the September 2017 report was performed by Dr. Philip Roberts (August 2017) using the same EPA Visual Plumes modeling suite.

Table 1 shows all of the modeled flow scenarios reported in all of the Ocean Plan compliance assessment technical memoranda. Footnotes document the relevant changes between each analysis effort.

**Table 1: GWR Project Dilution Modeling Results Summary** 

		Flows (MGD)		esuits Summary	D <sub>m</sub> Values for the GWR Pro	oject
No.	Secondary effluent	RO concentrate	Hauled Waste	COP Report February 2015 <sup>1</sup>	COP Addendum Report (4 MGD AWPF) April 2015 <sup>2</sup>	Larger GWR (5 MGD AWPF) September 2017 <sup>2,3,4</sup>
1	0	0.94	0.1	523	540	
	0	1.17	0			498
2	0.4	0.94	0.1	285	295	
	0.4	1.17	0			460
	0.6	1.17	0			442
3	0.8	1.17	0			
	2	1.17	0			358
4	3	0.94	0.1	201	208	
	4	1.17	0			299
	4.5	1.17	0			289
5	5	1.17	0			281
6	7	1.17	0			
7	8	0.94	0.1		228	
8	9	1.17	0			
9	21	1.17	0			
10	23.4	1.17	0			174
11	23.7	0.94	0.1	137	142	
12	24.7	0.94	0.1	150		

#### NOTES:

- 1 120 ports open
- 2 130 ports open
- 3 End gate closed, modeled with UM3 of Visual Plumes Suite
- 4 Updated ambient ocean data was used

The differences in  $D_m$  values between the 2015 4 MGD AWPF and the 2017 5 MGD AWPF is shown in Figure 1. Except for discharge scenarios with only RO concentrate (i.e., no Secondary Effluent), the larger AWPF allows for more dilution (i.e., higher  $D_m$  values). When there is no secondary effluent going to the outfall, less dilution in the ocean occurs for the larger GWR.

Differences in the  $D_m$  values are the result of both RO concentrate flow to the outfall and updated ambient ocean profiles.

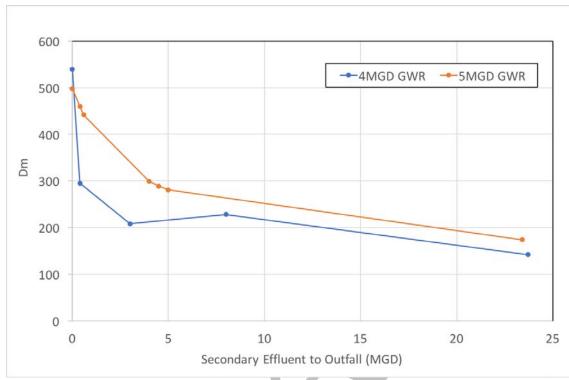


Figure 1. Comparison of D<sub>m</sub>s for two size GWR Advanced Water Purification Facilities

#### **MPWSP**

All of the changes for the GWR Project documented above also occurred for analysis of the MPWSP when comparing the (1) MPWSP and Variant COP assessment published in March 2015, (2) Addendum report published in April 2015, and (3) MPWSP and Variant Updates published in July 2016 and September 2017. The one exception to this statement is that hauled waste was included in the calculated D<sub>m</sub> values for the September 2017 analysis. (The March 2015 Ocean Plan Compliance Assessment report is included in Appendix V of the Final Consolidated EIR (January 2016), and the April 2015 Addendum report is included in Appendix U2 of the Final Consolidated EIR. The July 2016 Compliance Assessment report was included as Appendix D3 of CalAm's DEIR (January 2017).)

For the July 2016 and September 2017 reports, Dr. Phillip Roberts conducted the dilution modeling. Three methods were used when modeling ocean mixing: (1) the Cederwall formula (for neutral and negatively buoyant plumes only), (2) the mathematical model UM<sub>3</sub> in the United States Environmental Protection Agency's (EPA's) Visual Plume suite, and (3) the NRFIELD model (for positively buoyant plumes only) which is also from the EPA's Visual Plume suite. Table 2 shows all of the modeled flow scenarios reported in the Ocean Plan compliance assessment technical memoranda for the MPWSP.

**Table 2: MPWSP Dilution Modeling Results Summary** 

	Flows	(mgd)			D <sub>m</sub>	Values	
No.	Secondary effluent	Desal Brine	Hauled Waste	MPWSP & Variant March 2015 <sup>1</sup>	Addendum April 2015 <sup>2,3</sup>	MPWSP & Variant Update July 2016 <sup>2,5</sup>	MPWSP & Variant Update September 2017 <sup>2,4,5</sup>
1	0	13.98	0	16	17	14.6	14.4
2	1	13.98	0			15.2	
3	2	13.98	0	19		16.0	15.8
4	4	13.98	0.1				17.8
5	6	13.98	0.1				20.9
6	9	13.98	0		22	34.3	26.7
7	10	13.98	0.1				38.2
8	19.68	13.98	0	68			
9	19.78	13.98	0			153	98

#### NOTES:

- 1 120 ports open
- 2 130 ports open
- 3 Addendum scenarios included 0.1 mgd hauled waste
- 4 End gate closed, and 0.1 hauled waste was included
- 5 Updated ambient ocean data was used, Dr. Phillip Roberts provided dilution calculations using Cederwall, UM3, and NRFIELD models

None of these dilution modeling results were affected by the increased capacity of the GWR project's AWPF since none of these flow scenarios include RO concentrate from the AWPF. Note, though, that the large difference between  $D_{\rm m}$  values at the highest secondary effluent flow of 19.78 MGD is the result of different oceanic conditions—the 2016  $D_{\rm m}$  was for Davidson conditions while the 2017  $D_{\rm m}$  was for Upwelling conditions.

# **Variant Project**

All changes for the MPWSP documented above also occurred for the analysis of the Variant Project. Table 3 shows all modeled flow scenarios reported in the Ocean Plan compliance assessment technical memoranda. Footnotes document the relevant changes between each analysis effort.

**Table 3: Variant Project Dilution Modeling Results Summary** 

		Flows (mg		<b>-</b>			/alues	
No.	Secondary effluent	RO concentrate	Hauled Waste	Desal Brine	MPWSP & Variant March 2015 1,3	Addendum April 2015 <sup>2,3</sup>	MPWSP & Variant Update July 2016 <sup>5</sup>	MPWSP & Variant Update September 2017 <sup>4,5</sup>
1	0	0.94	0	8.99	17	18	15.6	15.3
2	0	1.17	0	8.99				15.5
3	1	0.94	0	8.99			16.4	16.1
4	1	1.17	0	8.99				16.4
5	2	1.17	0	8.99				17.7
6	3	0.94	0	8.99			20.3	19.6
7	3	1.17	0	8.99				20.1
8	4	1.17	0	8.99				23.8
9	5	1.17	0	8.99				30.7
10	5.3	0.94	0	8.99		24	54.4	31.6
11	5.3	1.17	0	8.99				33.3
12	6	1.17	0	8.99				67.5
13	7	1.17	0	8.99				90
14	11	1.17	0	8.99				106
15	15.92	0.94	0	8.99	82		194	114
16	15.92	1.17	0	8.99				114

#### NOTES:

- 1 120 ports open
- 2 130 ports open
- 3 Scenarios included 0.1 mgd hauled waste
- 4 End gate closed and 1.17 mgd RO concentrate
- 5 Updated ambient ocean data was used

Figure 2 shows a comparison of the modeled  $D_m$  values for the Variant Projects with the 4 MGD GWR and the 5 MGD GWR.  $D_m$  values for the two projects are similar until approximately 8 MGD of secondary effluent in the discharge. Beyond 8 MGD secondary effluent, greater dilution in the ocean (higher  $D_m$  values) is seen for the Variant Project with the smaller GWR.

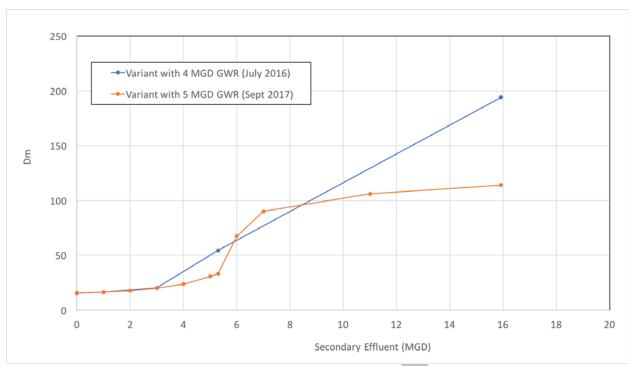


Figure 2. Comparison of  $D_m s$  for the Variant Project with two size GWR Advanced Water Purification Facilities

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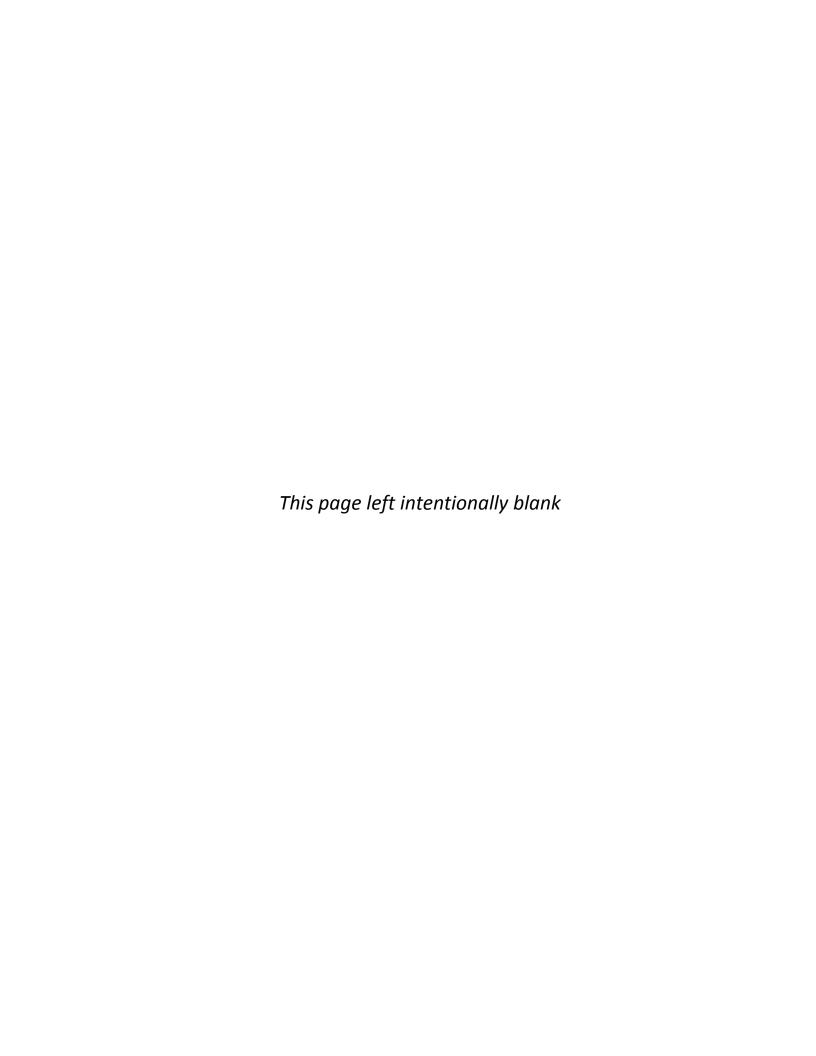
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Trussell Technologies, Inc. (Trussell Tech), 2017. "Ocean Plan Compliance Assessment for the Pure Water Monterey Groundwater Replenishment Project." Technical Memorandum prepared for MRWPCA and MPWMD. August.



# **Appendix E:**

Trussell Tech September 2017 Revised Ocean Plan Compliance Assessment for MPWSP and Project Variant



# Revised Ocean Plan Compliance Assessment for the Monterey Peninsula Water Supply Project and Project Variant

**Technical Memorandum** 

September 2017

# Prepared for:





# **Revised Ocean Plan Compliance Assessment for the Monterey Peninsula Water Supply Project and Project Variant**

## **Technical Memorandum**



Prepared By:

Trussell Technologies, Inc.

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# **Table of Contents**

1 Executive Summary	3
2 Introduction	7
2.1 Treatment through the Proposed CalAm Desalination Facility	
2.2 Treatment through the RTP and Proposed AWT Facilities	9
2.3 California Ocean Plan	10
2.4 Future Ocean Discharges	11
2.5 Objective of Technical Memorandum	14
3 Methodology for Ocean Plan Compliance Assessment	14
3.1 Methodology for Determination of Discharge Water Quality	14
3.1.1 Secondary Effluent	16
3.1.2 Desalination Brine	
3.1.3 Combined Ocean Discharge Concentrations	18
3.2 Ocean Modeling Methodology	18
3.2.1 Ocean Modeling Scenarios	19
3.2.2 Ocean Modeling Assumptions	22
4 Ocean Plan Compliance Results	22
4.1 Water Quality of Combined Discharge	22
4.2 Ocean Modeling Results	25
4.3 Ocean Plan Compliance Results	28
5 Conclusions	34
6 References	38
Appendix A	39
Appendix B	53
Appendix C	54
Appendix D	55

# **1 Executive Summary**

In response to State Water Resources Control Board (SWRCB) Water Rights Orders WR 95-10, WR 2009-0060, and WR 2016-0016, two proposed projects are in development on the Monterey Peninsula to provide potable water to offset pending reductions of Carmel River water diversions: (1) a seawater desalination project known as the **Monterey Peninsula Water Supply Project** (MPWSP), and (2) a groundwater replenishment project known as the **Pure Water Monterey Groundwater Replenishment Project** (GWR Project). The capacity of the MPWSP is dependent on the construction of the GWR Project.

If the GWR Project is not constructed, the MPWSP would entail California American Water ("CalAm") building a seawater desalination facility capable of producing 9.6 million gallons per day (mgd) of drinking water. In the variation of the MPWSP where the GWR Project is constructed, known as the **Monterey Peninsula Water Supply Project Variant** ("Variant"), CalAm would build a smaller desalination facility capable of producing 6.4 mgd of drinking water, and a partnership between the Monterey Peninsula Water Management District (MPWMD) and the Monterey Regional Water Pollution Control Agency (MRWPCA) would build an advanced water treatment facility ("AWPF") as part of the GWR Project. This AWPF would be able to produce up to 4,300 acre-feet per year (AFY) (annual average of 3.8 mgd) of highly purified recycled water to enable CalAm to extract 3,500 AFY (annual average of 3.1 mgd) from the Seaside Groundwater Basin for delivery to its customers.

Both the proposed desalination facility and the AWPF would employ reverse osmosis (RO) membranes to purify the waters, and as a result, both projects would produce RO concentrate waste streams that would be disposed through MRWPCA's existing ocean outfall: the brine concentrate from the desalination facility ("Desal Brine"), and the RO concentrate from the AWPF ("GWR Concentrate"). The goal of this technical memorandum (TM) is to analyze whether the discharges from the proposed projects through the existing ocean outfall would comply with the water quality objectives in the SWRCB 2015 Ocean Plan ("Ocean Plan") (SWRCB, 2015a).

The Ocean Plan sets forth numeric and narrative water quality objectives for the ocean with the intent of protecting the ocean's beneficial uses, which include recreation, aesthetics, navigation, fishing, mariculture, areas of special biological significance, rare and endangered species, habitat, fish migration, fish spawning, and shellfish harvesting. The Regional Water Quality Control Boards utilize these objectives to develop water quality-based effluent limitations for ocean dischargers that have a reasonable potential to exceed the water quality objectives.

When municipal wastewater flows are released from an outfall (typically using specially designed diffusers), the wastewater and ocean water undergo rapid mixing due to the momentum

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<sup>&</sup>lt;sup>1</sup> The AWPF would be capable of producing up to 5 mgd of highly purified recycled water on a daily basis, but production would fluctuate throughout the year, such that the average annual production would be 3.8 mgd (4,300 AFY) in a non-drought year, when adding to the drought reserve.



and buoyancy of the discharge.<sup>2</sup> The mixing that occurs in the rising plume is affected by the buoyancy and momentum of the discharge, a process referred to as initial dilution (NRC, 1993). For rising plumes, the Ocean Plan defines the initial dilution as complete when "the diluting wastewater ceases to rise in the water column and first begins to spread horizontally," (*i.e.*, when the momentum from the discharge has dissipated). For more saline discharges, a sinking plume forms when the discharge is denser than the ambient water (also known as a negatively buoyant plume). In the case of negatively buoyant plumes, the Ocean Plan defines the initial dilution as complete when "the momentum induced velocity of the discharge ceases to produce significant mixing of the waste, or the diluting plume reaches a fixed distance from the discharge to be specified by the Regional Board, whichever results in the lower estimate for initial dilution."

The numeric Ocean Plan objectives are to be met after the initial dilution of the discharge. The initial dilution occurs in an area known as the zone of initial dilution (ZID). The extent of dilution in the ZID is quantified and referred to as the minimum probable initial dilution ( $D_m$ ). The water quality objectives established in the Ocean Plan are adjusted by the  $D_m$  to derive effluent limitations in the National Pollutant Discharge Elimination System (NPDES) permit that are applied to a wastewater discharge prior to ocean dilution.

The purpose of this analysis was to assess the ability of the MPWSP and Variant to comply with the Ocean Plan objectives. Trussell Tech used a conservative approach to estimate the water qualities of the secondary effluent, GWR Concentrate, Desal Brine and hauled waste for these projects. Dr. Philip Roberts, a Professor in the School of Civil and Environmental Engineering at the Georgia Institute of Technology, conducted modeling of the ocean discharge and estimated D<sub>m</sub> values for scenarios involving different flow rates of the proposed projects and different ambient ocean conditions. These ocean modeling results were combined with projected discharge water quality to assess compliance with the Ocean Plan.

The estimates of minimum probable dilution ( $D_m$ ) developed by Dr. Roberts for the MPWSP range from 14.4 to 98, and from 14.4 to 114 for the Variant. These  $D_m$  values are substantially lower than what is currently specified in the MRWPCA NPDES permit (145) and those estimated for the GWR Project, which range from 174 to 498 (see Appendix B). As a result of the reduced dilution, some contaminants, which have not traditionally been of concern for discharge through MRWPCA's ocean outfall, are estimated to potentially exceed the Ocean Plan objectives at the edge of the ZID. A summary of the constituents that show potential to exceed the Ocean Plan objectives is provided in Table ES-1 for the MPWSP, and Table ES-2 for the Variant. These constituents can be divided into three categories:

• Category I - Insufficient analytical sensitivity to determine compliance: The constituent was not detected above the method reporting limit (MRL) in any of the source waters, but the MRL is not sensitive enough to demonstrate compliance with the Ocean Plan objective.

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<sup>&</sup>lt;sup>2</sup> Municipal wastewater effluent, being low in salinity, is less dense than seawater and thus rises (due to buoyancy) while it mixes with ocean water. GWR Concentrate, whether by itself or mixed with municipal wastewater effluent, is less dense than seawater and also rises (due to buoyancy) while it mixes with ocean water. Desal Brine, depending on the ratio of dilution with GWR Concentrate and municipal wastewater effluent, may be more or less dense than seawater.



- Category II Estimated to be close to exceeding the Ocean Plan objective: The constituent is estimated to be at a concentration between 80% and 100% of the Ocean Plan objective at the edge of the ZID.
- Category III Estimated to exceed the Ocean Plan objective: The constituent is estimated to be at a concentration higher than the Ocean Plan objective at the edge of the ZID.

Table ES-1: Summary of Compliance Conclusions for the MPWSP

	Category I <sup>a</sup>	Category II <sup>b</sup>	Category III c	Worst Case Exceedance	
Constituent	Compliance Determination Not Possible	Estimated to be Close to Exceeding Objective	Estimated to Exceed Objective	Flow Scenario <sup>f</sup>	Estimated Percentage of Objective at edge of ZID
Cyanide d			✓	4	140%
Ammonia			✓	5	102%
Chlorinated Phenolics	✓				
2,4-Dinitrophenol	✓				
Tributyltin	✓				
Acrylonitrile e	✓				
Aldrin	✓				
Benzidine	✓				
Beryllium <sup>e</sup>	✓				
Bis(2-chloroethyl)ether	✓				
3,3-Dichlorobenzidine	✓				
1,2-Diphenylhydrazine (azobenzene)	✓				
Heptachlor	✓				
TCDD Equivalents e	✓				
2,4,6-Trichlorophenol	✓				

#### Notes:

- **a:** ND in all sources, but MRL higher than Ocean Plan objective and therefore unable to demonstrate compliance. Exceptions are: MRL for 2,4-dinitrophenol was less than objective in secondary effluent and MRL for heptachlor was less than objective in slant well.
- **b:** Concentration of constituent at the edge of the ZID is estimated to be between 80% and 100% of the Ocean Plan objective for some scenarios
- c: Concentration of constituent is estimated to be > 100% of the Ocean Plan objective for some scenarios at the edge of the ZID
- d: Issues with approved analytical methods may have resulted in erroneously high cyanide quantification
- **e**: Only a best-case scenario could be evaluated, where a value of 0 was assumed when the constituent was ND and the MRL was larger than the Ocean Plan objective
- f: Flow scenarios are defined in Table 2 and Table 3



Table ES-2: Summary of Compliance Conclusions for the Variant

	Category I <sup>a</sup>	Category II b	Category III c	Worst Case Exceedance	
Constituent	Compliance Determination Not Possible	Estimated to be Close to Exceeding Objective	Estimated to Exceed Objective	Flow Scenario <sup>f</sup>	Estimated Percentage of Objective at edge of ZID
Cyanide d			✓	31	189%
Ammonia			✓	30	266%
Chlorinated Phenolics	✓				
2,4-Dinitrophenol	✓				
Tributyltin	✓				
Acrylonitrile <sup>e</sup>		✓		30	94%
Aldrin	✓				
Benzidine	✓				
Beryllium <sup>e</sup>	✓				
Bis(2-chloroethyl)ether	✓				
Bis(2-ethyl-hexyl)phthalate		✓		30	84%
Chlordane			✓	30	199%
3,3-Dichlorobenzidine	✓				
1,2-Diphenylhydrazine (azobenzene)	✓				
Heptachlor	✓				
PCBs			✓	30	169%
TCDD Equivalents <sup>e</sup>			✓	30	131%
Toxaphene			✓	30	126%
2,4,6-Trichlorophenol	✓				

#### Notes:

- a: ND in all sources, but MRL higher than Ocean Plan objective and therefore unable to demonstrate compliance. Exceptions are: MRL for 2,4-dinitrophenol was less than objective in secondary effluent and MRL for heptachlor was less than objective in slant well.
- b: Concentration of constituent at the edge of the ZID is estimated to be between 80% and 100% of the Ocean Plan objective for some scenarios
- c: Concentration of constituent is estimated to be > 100% of the Ocean Plan objective for some scenarios at the edge of the ZID
- d: Issues with approved analytical methods may have resulted in erroneously high cyanide quantification
- **e**: Only a best-case scenario could be evaluated, where a value of 0 was assumed when the constituent was ND and the MRL was larger than the Ocean Plan objective
- f: Flow scenarios are defined in Table 2 and Table 3

Based on the data, assumptions, modeling, and analytical methodology presented in this TM, the MPWSP and Variant show a potential to exceed certain Ocean Plan objectives under specific discharge scenarios (see Tables ES-1 and ES-2). In particular, potential issues were identified for the MPWSP and Variant discharge scenarios involving low to moderate secondary effluent flows with Desal Brine: discharges are estimated to exceed or come close to exceeding multiple Ocean Plan objectives, specifically those for cyanide and ammonia for the MPWSP, and cyanide,



ammonia, chlordane, PCBs, TCDD equivalents, and toxaphene for the Variant. Ammonia clearly exceeds the Ocean Plan objective and must be resolved for the MPWSP and Variant. When considering a best-case analysis for the Variant, acrylonitrile is estimated to come close to exceeding the Ocean Plan objective, and TCDD equivalents show a potential to exceed the objective. Additional analytical investigation regarding cyanide analysis is recommended to determine if the potential exceedances are representative of actual water quality conditions. Chlordane, PCBs and toxaphene, which were estimated to exceed the objectives for the Variant flow scenarios, were detected at concentrations that are orders of magnitude below detection limits of methods currently used for discharge compliance.

### 2 Introduction

In response to State Water Resources Control Board (SWRCB) Water Rights Orders WR 95-10, WR 2009-0060, and WR 2016-0016, two proposed projects are in development on the Monterey Peninsula to provide potable water to offset pending reductions of Carmel River water diversions: (1) a seawater desalination project known as the **Monterey Peninsula Water Supply Project** (MPWSP), and (2) a groundwater replenishment project known as the **Pure Water Monterey Groundwater Replenishment Project** (GWR Project). The capacity of the MPWSP is dependent on the construction of the GWR Project.<sup>3</sup>

If the GWR Project is constructed, the MPWSP would entail California American Water ("CalAm") building a seawater desalination facility capable of producing 9.6 million gallons per day (mgd) of drinking water. In the variation of the MPWSP where the GWR Project is constructed, known as the **Monterey Peninsula Water Supply Project Variant** ("Variant"), CalAm would build a smaller desalination facility capable of producing 6.4 mgd of drinking water, and a partnership between the Monterey Peninsula Water Management District (MPWMD) and the Monterey Regional Water Pollution Control Agency (MRWPCA) would build an advanced water treatment facility ("AWPF") as part of the GWR Project. This AWPF would be able to produce up to 4,300 acre-feet per year (AFY) (annual average of 3.8 mgd)<sup>4</sup> of highly purified recycled water to enable CalAm to extract 3,500 AFY (annual average of 3.1 mgd) from the Seaside Groundwater Basin for delivery to its customers.

The GWR Project involves treating secondary-treated wastewater (*i.e.*, secondary effluent) from MRWPCA's Regional Treatment Plant (RTP) through the proposed Advanced Water Purification Facility (AWPF) and then injecting up to 3,700 AFY of this highly purified recycled water into the Seaside Groundwater Basin, with subsequent withdrawal for use as a municipal water supply, and providing up to 600 AFY to Marina Coast Water District for urban landscape irrigation. The GWR Project will also provide additional tertiary recycled water for agricultural irrigation in the northern Salinas Valley as part of the Castroville Seawater Intrusion Project (CSIP). Both the proposed desalination facility and the AWPF would employ reverse osmosis (RO) membranes to purify the waters, and as a result, both projects would produce RO concentrate waste streams that would be disposed through MRWPCA's existing ocean outfall:

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<sup>&</sup>lt;sup>3</sup> Construction of the GWR Project is expected to begin in September 2018.

<sup>&</sup>lt;sup>4</sup> The AWPF would be capable of producing up to 5 mgd of highly purified recycled water on a daily basis, but production would fluctuate throughout the year, such that the average annual production would be 3.8 mgd (4,300 AFY) in a non-drought year, when adding to the drought reserve.



the brine concentrate from the desalination facility ("Desal Brine"), and the RO concentrate from the AWPF ("GWR Concentrate").

The goal of this TM is to analyze whether the discharges from the proposed projects through the existing ocean outfall would comply with the numeric water quality objectives in the SWRCB 2015 Ocean Plan ("Ocean Plan") (SWRCB, 2015). A similar assessment of the GWR Project on its own was previously performed (Trussell Tech, 2017, see Appendix B), and so this document provides complementary information focused on the MPWSP and Variant projects.

The original version of this document (Trussell Tech, 2015a) and an addendum report to that document (Trussell Tech, 2015b) were included in both the GWR Project Consolidated Final Environmental Impact Report (CFEIR) and the MPWSP draft Environmental Impact Report (EIR). A second version of this document was updated to include new water quality data and flow scenarios for the MPWSP and Variant to address data gaps noted in the original analyses, and was included in the 2017 MPWSP draft EIR (Trussell Tech, 2016, see Appendix C). The following TM incorporates updates to the 2016 version, including additional water quality data and flow scenarios, and these revisions are discussed in more detail in the following sections.

### 2.1 Treatment through the Proposed CalAm Desalination Facility

This section describes the proposed treatment train for the MPWSP and Variant desalination facility. Seawater from the Monterey Bay would be extracted through subsurface slant wells beneath the ocean floor and piped to a new CalAm-owned desalination facility. This facility would consist of granular media pressure filters, cartridge filters, a two-pass RO membrane system, RO product-water stabilization (for corrosion control), and disinfection – (Figure 1). The RO process is expected to recover 42 percent of the influent seawater flow as product water, while the remainder of the concentrated influent water becomes the Desal Brine. The MPWSP and Variant product water (desalinated water) would be used for municipal drinking water, while the Desal Brine would be blended with (1) available RTP secondary effluent, (2) brine that is trucked and stored at the RTP, and (3) GWR Concentrate (for the Variant only), and discharged to the ocean through the existing MRWPCA ocean outfall. The volume of Desal Brine is dependent on the project size: 13.98 and 8.99 mgd for the MPWSP and Variant, respectively.

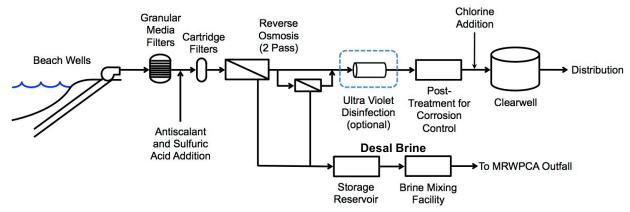


Figure 1 – Schematic of CalAm desalination facilities

### 2.2 Treatment through the RTP and Proposed AWT Facilities

The existing MRWPCA RTP treatment process includes screening, primary sedimentation, secondary biological treatment through trickling filters, followed by a solids contactor (*i.e.*, bioflocculation), and clarification (Figure 2). Much of the secondary effluent undergoes tertiary treatment (coagulation, flocculation, granular media filtration, and disinfection) to produce recycled water used for agricultural irrigation. The unused secondary effluent is discharged to the Monterey Bay through the MRWPCA outfall. MRWPCA also accepts trucked brine waste for ocean disposal ("hauled waste"), which is stored in a pond and mixed with secondary effluent prior to being discharged.

The AWPF will include several advanced treatment technologies for purifying the secondary effluent: ozone (O<sub>3</sub>), membrane filtration (MF), reverse osmosis (RO), an advanced oxidation process (AOP) using ultraviolet light (UV) and hydrogen peroxide, and finished water stabilization. The Project Partners conducted a pilot-scale study of the planned AWPF ozone, MF, and RO processes from December 2013 through July 2014, successfully demonstrating the ability of the various treatment processes to produce highly-purified recycled water that complies with the California Water Recycling Criteria for Indirect Potable Reuse: Groundwater Replenishment – Subsurface Application (Groundwater Replenishment Regulations) (SWRCB, 2015b) and Central Coast Water Quality Control Plan (Basin Plan) standards, objectives and guidelines for groundwater (CCRWQCB, 2011). After the pilot-scale study, an advanced water purification demonstration facility was built to gain additional experience operating ozone, MF, and RO processes. The new facility also included a UV/hydrogen peroxide AOP and stabilization treatment. The demonstration facility is operated and maintained by MRWPCA.

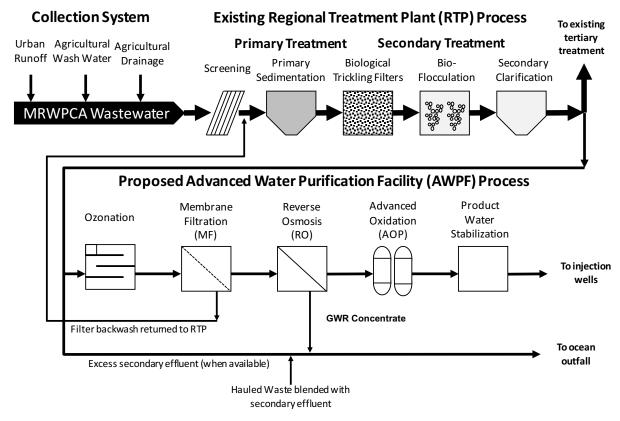


Figure 2 - Schematic of existing MRWPCA RTP and proposed AWPF treatment

### 2.3 California Ocean Plan

The Ocean Plan sets forth numeric and narrative water quality objectives for the ocean waters with the intent of protecting the ocean's beneficial uses, which include recreation, aesthetics, navigation, fishing, mariculture, areas of special biological significance, rare and endangered species, habitat, fish migration, fish spawning, and shellfish harvesting (SWRCB, 2015a). The Regional Water Quality Control Boards utilize these objectives to develop water quality-based effluent limitations for ocean dischargers that have a reasonable potential to exceed the water quality objectives.

When municipal wastewater flows are released from an outfall (typically using specially designed diffusers), the wastewater and ocean water undergo rapid mixing due to the momentum and buoyancy of the discharge.<sup>5</sup> The mixing that occurs in the rising plume is affected by the buoyancy and momentum of the discharge, a process referred to as initial dilution (NRC, 1993). For rising plumes, the Ocean Plan defines the initial dilution as complete when "the diluting wastewater ceases to rise in the water column and first begins to spread horizontally," (*i.e.*, when the momentum from the discharge has dissipated). For more saline discharges, a sinking plume forms when the discharge is denser than the ambient water (also known as a negatively buoyant

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seawater.

<sup>&</sup>lt;sup>5</sup> Municipal wastewater effluent, being low in salinity, is less dense than seawater and thus rises (due to buoyancy) while it mixes with ocean water. GWR Concentrate, whether by itself or mixed with municipal wastewater effluent, is less dense than seawater and also rises (due to buoyancy) while it mixes with ocean water. Desal Brine, depending on the ratio of dilution with GWR Concentrate and municipal wastewater effluent, may be more or less dense than



plume). In the case of negatively buoyant plumes, the Ocean Plan defines the initial dilution as complete when "the momentum induced velocity of the discharge ceases to produce significant mixing of the waste, or the diluting plume reaches a fixed distance from the discharge to be specified by the Regional Board, whichever results in the lower estimate for initial dilution."

The numeric Ocean Plan objectives are to be met after the initial dilution of the discharge. The initial dilution occurs in an area known as the zone of initial dilution (ZID). The extent of dilution in the ZID is quantified and referred to as the minimum probable initial dilution ( $D_m$ ). The water quality objectives established in the Ocean Plan are adjusted by the  $D_m$  to derive National Pollutant Discharge Elimination System (NPDES) permit limits that are applied to a wastewater discharge prior to ocean dilution.

The current MRWPCA wastewater discharge is governed by NPDES Permit No. CA0048551 (currently implemented as Order No. R3-2014-0013) issued by the Central Coast Regional Water Quality Control Board ("RWQCB") (CCRWQCB, 2014). Because the existing NPDES permit for the MRWPCA ocean outfall must be amended to discharge Desal Brine, comparing future discharge concentrations to the current NPDES permit limits (that will likely change when the permit is amended) would not be an appropriate metric or threshold for determining whether the proposed projects would have a significant impact on marine water quality. Instead, compliance with the Ocean Plan objectives was selected as an appropriate threshold for determining whether the proposed projects would result in a significant impact requiring mitigation.

Dr. Philip Roberts, a Professor in the School of Civil and Environmental Engineering at the Georgia Institute of Technology, conducted dilution modeling of the ocean discharge and estimated  $D_m$  values for scenarios involving different flow rates of the proposed projects and different ambient ocean conditions. These ocean modeling results were combined with projected discharge water quality to assess compliance with the Ocean Plan. Dr. Roberts' report is included as Appendix D.

# 2.4 Future Ocean Discharges

A summary schematic of the MPWSP and Variant is presented in Figure 3. For the MPWSP, 23.58 mgd of ocean water (design capacity) would be treated in the desalination facility; an RO recovery of 42% would lead to an MPWSP Desal Brine flow of 13.98 mgd that would be discharged through the outfall. Following periods of plant shutdown, the facility may produce 16.31 mgd of Desal Brine to temporarily boost plant production. Secondary effluent from the RTP would also be discharged through the outfall, although the flow would be variable depending on both the raw wastewater flow and the proportion being processed through the tertiary treatment system at the Salinas Valley Reclamation Plant (SVRP) to produce recycled water for agricultural irrigation. The third and final discharge component is hauled waste that is trucked to the RTP and blended with secondary effluent prior to discharge. The maximum anticipated flow of the hauled waste is 0.03 mgd, and is blended with secondary effluent for a total flow of 0.1 mgd. These three discharge components (Desal Brine, secondary effluent, and hauled waste) would be mixed at the proposed Brine Mixing Facility prior to ocean discharge.

For the Variant, 15.93 mgd of ocean water (design capacity) would be pumped to the desalination facility, and an RO recovery of 42% would result in a Variant Desal Brine flow of



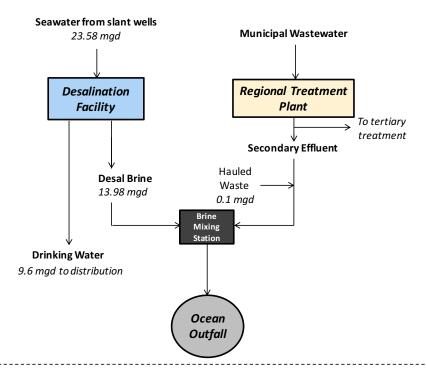
8.99 mgd. Similar to the larger desalination facility, the plant may produce 11.24 mgd of Desal Brine for a short period of time to boost plant production. The Variant would include the GWR Project, which involves the addition of new source waters to the RTP that would alter the water quality of the secondary effluent produced by the RTP. The secondary effluent in the Variant is referred to as "Variant secondary effluent," and would be different in quality from the MPWSP secondary effluent. Under the GWR Project, a portion of the secondary effluent would be fed to the AWPF, and the resultant GWR Concentrate (maximum 1.17 mgd) would be discharged through the outfall. The hauled waste received at the RTP would continue to be mixed with secondary effluent prior to discharge, and so the quality of the blended brine and secondary effluent will change as a result of the change in secondary effluent quality. The hauled waste for the Variant is referred to as "Variant hauled waste." The discharge components for the MPWSP and Variant are summarized in Table 1.

Table 1 – Discharge waters Included in each analysis

Project	Desal Brine	Secondary Effluent	Variant Secondary Effluent	Hauled Waste	Variant Hauled Waste <sup>a</sup>	GWR Concentrate
MPWSP	(13.98 mgd, 16.31 mgd periodically)	✓ (flow varies)		<b>√</b> (0.1 mgd)		
Variant	(8.99 mgd, 11.24 mgd periodically)		✓ (flow varies)		<b>√</b> (0.1 mgd)	<b>√</b> (1.17 mgd)

<sup>&</sup>lt;sup>a</sup> This is placed in a separate category because it contains Variant secondary effluent.

### **MPWSP**



## MPWSP Variant ("Variant")

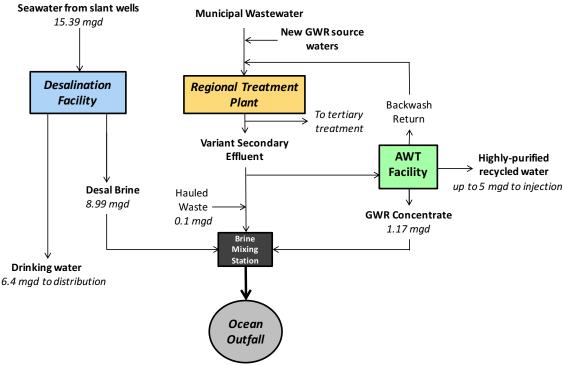


Figure 3 – Flow schematics for the MPWSP and Variant projects (specified flow rates are at design capacity)

# 2.5 Objective of Technical Memorandum

Trussell Technologies, Inc. ("Trussell Tech") estimated worst-case in-pipe water quality for the various ocean discharge scenarios (*i.e.*, prior to dilution through ocean mixing) for the proposed projects. Dr. Roberts' ocean discharge modeling and the results of the water quality analysis were then used to provide an assessment of whether the proposed projects would consistently meet Ocean Plan water quality objectives. The objective of this TM is to summarize the assumptions, methodology, results and conclusions of the Ocean Plan compliance assessment for the MPWSP and Variant.

# 3 Methodology for Ocean Plan Compliance Assessment

Water quality data from various sources for the different treatment process influent and waste streams were compiled. Trussell Tech combined these data for different flow scenarios and used ocean modeling results (i.e.,  $D_m$  values) to assess compliance of different discharge scenarios with the Ocean Plan objectives. This section documents the data sources and provides further detail on the methodology used to perform this analysis. A summary of the methodology is presented in Figure 4.

# 3.1 Methodology for Determination of Discharge Water Quality

The amounts and combinations of various wastewaters that would be disposed through the MRWPCA outfall will vary depending on the capacity, seasonal and daily flow characteristics, and extent and timing of implementation of the proposed projects.

Detailed discussions about the methods used to determine the discharge water qualities related to the GWR Project were previously discussed and can be found in Appendix B. This previous analysis included water quality estimates of the secondary effluent, Variant secondary effluent, hauled waste, Variant hauled waste, and the GWR Concentrate (*i.e.*, all of the discharges except for the Desal Brine). In the previous analysis, Trussell Tech assumed that the highest observed values for the various Ocean Plan constituents within each type of water flowing to and treated at the RTP, including the AWPF as applicable, to be the worst-case water quality. These same data and assumptions were used in the analysis described in this memorandum. Use of these worst-case water quality concentrations ensures that the analysis in this memorandum is conservative related to the Ocean Plan compliance assessment (and thus, the impact analysis for the MPWSP environmental review processes).

To determine the impact of the MPWSP and Variant, the worst-case water quality of the Desal Brine was estimated using available data from CalAm's temporary test subsurface slant well on the CEMEX mine property in Marina, California. Long-term pumping and water quality sampling from this well began in April 2015. As in the previous Ocean Plan compliance

<sup>&</sup>lt;sup>6</sup> Except for copper, where instead the median was calculated from the data for each new source water because the maximum values detected seemed to be outliers, and the Ocean Plan objective for copper considered in this assessment is the 6-month median concentration.

<sup>&</sup>lt;sup>7</sup> The well was shut down on June 5, 2015 to assess regional trends in aquifer water levels and resumed pumping October 27, 2015. The well was shut down again between March 4, 2016 and May 2, 2016 for discharge line repairs. No water quality data were collected during shutdown periods.



assessments, the highest observed concentrations in the slant well were used for this Ocean Plan compliance assessment.<sup>8</sup>

The methodology for determining the water quality of the Desal Brine and secondary effluent is further described in this section (the methodology for all other discharge waters can be found in Appendix B). A summary of which discharge waters are considered for both the MPWSP and Variant, and which data sources were used in the determination of the water quality for each discharge stream is shown in Figure 4.

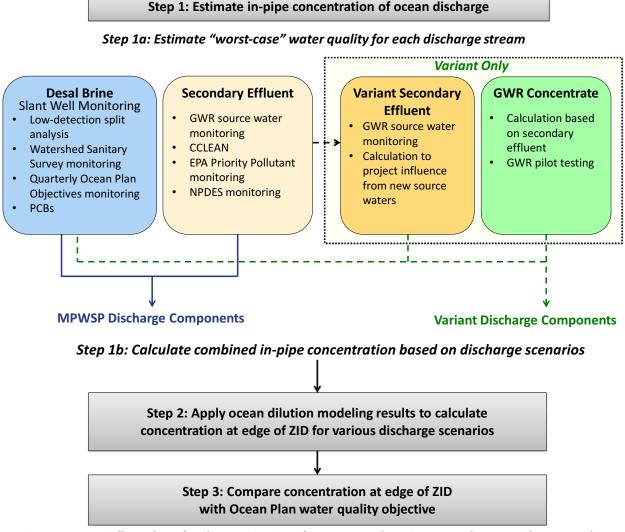


Figure 4 – Logic flow chart for determination of MPWSP and Variant compliance with Ocean Plan objectives.

<sup>&</sup>lt;sup>8</sup> Except for copper, where instead the median was calculated from data from the test slant well because the maximum values detected seemed to be outliers, and the Ocean Plan objective for copper considered in this assessment is the 6-month median concentration.



## 3.1.1 Secondary Effluent

For the MPWSP, the discharged secondary effluent would not be impacted by additional source waters that would be brought in for the Variant; therefore, the historical secondary effluent quality was used in the analysis. The following sources of data were considered for selecting a secondary effluent concentration for each constituent in the analysis:

- Secondary effluent water quality monitoring conducted for the GWR Project from July 2013 through June 2014.
- MRWPCA RTP historical NPDES compliance water quality data collected semi-annually by MRWPCA (2005- Spring 2017).
- Historical NPDES RTP Priority Pollutant data collected annually by MRWPCA (2004-2016).
- Water quality data collected semi-annually by the Central Coast Long-Term Environmental Assessment Network (CCLEAN) (2008-2016) (CCLEAN, 2014).

The secondary effluent concentration for each constituent selected for the analysis was the maximum reported value from the above sources. In some cases, constituents were not detected (ND); in these cases, the values are reported as ND (<MRL). In cases where the analysis of a constituent was detected but not quantified, the result is also reported as less than the Method Reporting Limit ND (<MRL). Because the actual concentration could be any value equal to or less than the MRL, the conservative approach is to use the value of the MRL for the compliance analysis. For some ND constituents, the MRL exceeds the Ocean Plan objective, and thus no compliance determination can be made. A detailed discussion of the cases where a constituent was reported as less than the MRL is included in the GWR Project TM in Appendix B (Trussell Technologies, 2017).

Cyanide has been detected in the RTP effluent at relatively high levels compared to the discharge requirements. The maximum detected value in the RTP effluent was 81 µg/L.

Several investigations have been conducted into the accuracy of sampling, preservation, and analytical methods for cyanide. These have shown that sample holding time and preservation have a significant impact on measured cyanide concentrations. Pandit et al. (2006) demonstrated that when sodium hydroxide was added to adjust the pH higher than 12, as specified in accepted methods for cyanide measurement in order to preserve the sample, the measured cyanide concentrations were consistently higher than those for samples preserved at pH 10 to 11. They also showed that cyanide levels increased within the recommended holding times of the approved cyanide methods (at pH 12).

<sup>&</sup>lt;sup>9</sup> The lowest amount of an analyte in a sample that can be quantitatively determined with stated, acceptable precision and accuracy under stated analytical conditions (i.e., the lower limit of quantitation). Therefore, acceptable quality control and quality assurance procedures are calibrated to the MRL, or lower. To take into account day-to-day fluctuations in instrument sensitivity, analyst performance, and other factors, the MRL is established at three times the Method Detection Limit (or greater). The Method Detection Limit is the minimum concentration of a substance that can be measured and reported with 99% confidence that the analyte concentration is greater than zero. (40 Code of Federal Regulations Section 136 Appendix B).

<sup>&</sup>lt;sup>10</sup> This phenomenon is common in the implementation of the Ocean Plan where for some constituents, suitable analytical methods are not capable of measuring low enough to quantify the minimum toxicologically relevant concentrations. For these constituents, a discharge is considered compliant if the monitoring results are less than the MRL.

In addition, the 2015 California Ocean Plan specifies the following:

If a discharger can demonstrate to the satisfaction of the Regional Water Board (subject to EPA approval) that an analytical method is available to reliably distinguish between strongly and weakly complexed cyanide, effluent limitations for cyanide may be met by the combined measurement of free cyanide, simple alkali metal cyanides, and weakly complexed organometallic cyanide complexes. In order for the analytical method to be acceptable, the recovery of free cyanide from metal complexes must be comparable to that achieved by the approved method in 40 CFR PART 136, as revised May 14, 1999.

Based on the above information, it is recommended that additional cyanide sampling be conducted using different methods (*e.g.*, analysis within 15 minutes with no preservation) to determine if the laboratory method leads to inaccurately high cyanide values. It is also recommended to determine if a method can be performed that distinguishes between weakly and strongly complexed cyanide. Until this is completed, all cyanide concentrations presently available are used in this Ocean Plan compliance assessment.

### 3.1.2 Desalination Brine

Trussell Tech used the following four sources of data for the Desal Brine water quality assessment:

- A one-time 7-day composite sample from the test slant well with separate analysis of particulate and dissolved phase fractions of constituents using low-detection CCLEAN analysis techniques (February 18-25, 2016). The maximum total concentration was used in this analysis (*i.e.* the sum of the concentration in the particulate and dissolved phase fractions). Of the constituents analyzed with this split phase method, all were detected 100% in the dissolved phase, except PCBs, which were detected 99% in the dissolved phase.
- CalAm Watershed Sanitary Survey monitoring program monthly test slant well sampling water quality results (May 2015 April 2017). 13
- Quarterly sampling of the test slant well for constituents specified in the Ocean Plan (November 2015, February, June, and September 2016).
- Test slant well sampling by Geoscience Support Services, Inc. ("Geoscience") every other month for polychlorinated biphenyls (PCBs) (May 2015 February 2016). 11

The maximum value observed in any of the data sources was assumed to be the "worst-case" water quality for the raw seawater feeding the desalination facility. If a constituent was ND in all samples, and multiple analysis methods were used with varying MRL values, the highest MRL

<sup>&</sup>lt;sup>11</sup> Only method detection limits were provided for these results. When a constituent was ND in this dataset, the method detection limit was used for analysis.

<sup>&</sup>lt;sup>12</sup> Hexachlorobutadiene, hexachlorobenzene, HCH, heptachlor, aldrin, chlordane, DDT, heptachlor epoxide, dieldrin, Endrin, endosulfans, toxaphene, PCBs

<sup>&</sup>lt;sup>13</sup> The well was shut down on June 5, 2015 to assess regional trends in aquifer water levels and resumed pumping October 27, 2015. The well was shut down again between March 4, 2016 and May 2, 2016 for discharge line repairs. No water quality data were collected during shutdown periods.



was assumed for compliance analysis; the exception to this statement is when data were available from the low detection limit 7-day composite sample. For these constituents, <sup>14</sup> the detected value from the low detection analysis was used, even if it was lower than the MRL provided by the standard analysis methods. If the sample results of a constituent reported the concentration as less than the MRL, the MRL was assumed for compliance analysis and the concentration is reported as ND ( $\leq$ MRL) in this TM. Equation 1 was used to calculate a conservative estimate of the Desal Brine concentration ( $C_{Brine}$ ) for each constituent by using a concentration factor of 1.73, which was calculated assuming complete rejection of the constituent in the feed water ( $C_{Feed}$ ) and a 42% recovery (%R) through the seawater RO membranes.

$$C_{Brine} = \frac{C_{Feed}}{1 - \%_{R}} \tag{1}$$

## 3.1.3 Combined Ocean Discharge Concentrations

Having estimated the worst-case concentrations for each of the discharge components, the combined concentration prior to discharge was determined as a flow-weighted average of the contributions of each of the discharge components appropriate for the MPWSP and Variant.

# 3.2 Ocean Modeling Methodology

In order to determine Ocean Plan compliance, Trussell Tech used the following information: (1) the in-pipe (*i.e.*, pre-ocean dilution) concentration of a constituent ( $C_{\text{in-pipe}}$ ) that was developed as discussed in the previous section, (2) the minimum probable dilution for the ocean mixing ( $D_{\text{m}}$ ) for the discharge flow scenarios that were modeled by Dr. Roberts<sup>15</sup> (Roberts, P. J. W, 2017), and (3) the background concentration of the constituent in the ocean ( $C_{\text{Background}}$ ) that is specified in Table 3 of the Ocean Plan (SWRCB, 2015b). With this information, the concentration at the edge of the zone of initial dilution ( $C_{\text{ZID}}$ ) was calculated using the following equation:

$$C_{ZID} = \frac{C_{In-pipe} + D_m * C_{Background}}{1 + D_m}$$
 (2)

The C<sub>ZID</sub> was then compared to the Ocean Plan water quality objectives<sup>16</sup> in Table 1 of the Ocean Plan (SWRCB, 2015). In this table, there are three categories of objectives: (1)

<sup>&</sup>lt;sup>14</sup> Endrin, hexachlorocyclohexane, chlordane, DDT, dieldrin, heptachlor, heptachlor epoxide, hexachlorobutadiene, PCBs, toxaphene.

<sup>&</sup>lt;sup>15</sup> The Ocean Plan defines dilution differently than Dr. Roberts. Dr. Roberts provided results defined as  $S = [total volume of a sample]/[volume of effluent contained in the sample]. The <math>D_m$  referenced in Equation 1 of the California Ocean Plan is defined as  $D_m = S - 1$ . A value of 1 was subtracted from the dilution estimates provided by Dr. Roberts prior to using Ocean Plan Equation 1.

<sup>&</sup>lt;sup>16</sup> Note that the Ocean Plan also defines effluent limitations for oil and grease, suspended solids, settleable solids, turbidity, and pH (see Ocean Plan Table 2). These parameters were not evaluated in this assessment. It is assumed that, if necessary, the pH of the water would be adjusted to be within acceptable limits prior to discharge. Oil and grease, suspended solids, settable solids, and turbidity in the GWR Concentrate and Desal Brine would be significantly lower than the secondary effluent. Prior to the AWPF RO treatment process, the process flow would be treated by MF, which will reduce these parameters, and the waste stream from the MF will be returned to RTP



Objectives for Protection of Marine Aquatic Life, (2) Objectives for Protection of Human Health – Non-Carcinogens, and (3) Objectives for Protection of Human Health – Carcinogens. There are also three objectives for each constituent included in the first category (for marine aquatic life): six-month median, daily maximum and instantaneous maximum concentration. For the other two categories, there is one objective: 30-day average concentration. When a constituent had three objectives, the lowest objective, the six-month median, was used to estimate compliance. This approach was taken because the discharge scenarios, discussed in further detail below, could be experienced for six months, and therefore the 6-month median objective would need to be met. For the ammonia objectives (specifically, the total ammonia concentration calculated as the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>), expressed in µg/L as N) the daily maximum and 6-month median objectives were evaluated.

For each discharge scenario, if the  $C_{\rm ZID}$  was below the Ocean Plan objective, then it was assumed that the discharge would comply with the Ocean Plan. However, if the  $C_{\rm ZID}$  exceeds the Ocean Plan objective, then it was concluded that the discharge scenario could violate the Ocean Plan objective. Note that this approach could not be applied for some constituents, viz., acute toxicity, chronic toxicity, and radioactivity. Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based on the nature of the constituents. These constituents were measured individually for the secondary effluent and GWR Concentrate, and these individual concentrations would comply with the Ocean Plan objectives. Toxicity testing on the seawater was not included in the analysis for this TM; it will be evaluated by another method not discussed in this TM.

Dr. Roberts performed modeling of various discharge scenarios for the MPWSP and Variant that include combinations of Desal Brine, secondary effluent, GWR Concentrate, and hauled waste (Roberts, P. J. W, 2017). Forty-seven scenarios resulting in the worst-case dilution conditions will be presented in this TM. These scenarios assume the maximum flow rates for the GWR Concentrate, Desal Brine and hauled waste, which is a conservative assumption in terms of constituent loading and minimum dilution. Additional flow scenarios were modeled by Dr. Roberts, and can be found in his report (Appendix D).

## 3.2.1 Ocean Modeling Scenarios

The modeled scenarios are summarized in Tables 2 and 3 for the MPWSP and the Variant, respectively. The Variant discharge scenarios that have no Desal Brine (*i.e.*, Scenarios 21 through 29) have already been analyzed and found to comply with the Ocean Plan (Trussell Tech 2017, see Appendix B); these scenarios are shown in Table 3 for completeness, but for simplicity, the analysis of these scenarios is not repeated in Section 4.

The MPWSP flow scenarios included in this analysis cover the range of potential future discharge compositions, with various secondary effluent flows and Desal Brine flows included. The amount of secondary effluent being discharged is dependent on the demand for recycled water (highest demand, and lowest secondary effluent discharge is experienced during the

headworks. Prior to the Desalination Facility RO treatment process, the process flow would be treated by granular media filters and cartridge filters, which reduce these parameters. The waste stream from the granular media filter would be further treated in gravity thickening basins prior to any discharge of the decant through the ocean outfall. The cartridge filters will be disposed off-site and the solids will not be returned to the process.



summer months), and whether the SVRP is operational. Modeling the minimum secondary effluent flows (*i.e.*, no secondary effluent discharged) provides conditions where the influence of Desal Brine on the ocean discharge water quality is maximized and the discharge plumes are negatively buoyant. The moderate secondary effluent flow scenarios create conditions where the Desal Brine and the secondary effluent have similar levels of influence on the water quality of the ocean discharge, as well as neutrally buoyant discharge plumes. The high secondary effluent flow scenarios provide analysis of the highest expected flows that may be discharged, where the discharge is buoyant.

Table 2 - Modele	ed flow scenari	ios for the	IVIPVVSP

Flow Scenario		Discharge Flows (mgd)	
No.	Secondary Effluent <sup>a</sup>	Desal Brine	Hauled Waste
MPWSP w	ith Normal Desal Brine Flow		
1	0	13.98	0.1
2	2	13.98	0.1
3	4	13.98	0.1
4	6	13.98	0.1
5	9	13.98	0.1
6	10	13.98	0.1
7	19.78	13.98	0.1
MPWSP w	ith High Desal Brine Flow	·	
8	0	16.31	0.1
9	2	16.31	0.1
10	7	16.31	0.1
11	8	16.31	0.1
12	10	16.31	0.1
13	12	16.31	0.1
14	16	16.31	0.1

<sup>&</sup>lt;sup>a</sup> Note that RTP wastewater flows have been declining in recent years as a result of water conservation; while 19.78 mgd is higher than current RTP wastewater flows, this is expected to be a conservative scenario with respect to ocean modeling, compared to using the current wastewater flows of 16 to 18 mgd.

Similar to the flow scenarios for the MPWSP, Variant flow scenarios were selected to cover the complete range of potential future discharge compositions. These scenarios encompass periods when the AWPF is offline, and/or the desalination plant is offline. They also cover short-term operations with higher Desal Brine discharges when the desalination plant is catching up on production after periods of being offline. All these potential operating conditions were considered with varying amounts of secondary effluent flow, as it is possible that any of these conditions may be experienced during future operations.

Table 3 – Modeled flow scenarios for the Variant

Flow		Discharge	Flows (mgd)	
Scenario No.	Secondary Effluent <sup>a</sup>	Desal Brine	GWR Concentrate	Hauled Waste <sup>b</sup>
Variant with	h AWPF Offline			
15	0	8.99	0	0
16	2	8.99	0	0
17	4	8.99	0	0
18	5.8	8.99	0	0
19	14	8.99	0	0
20	19.78	8.99	0	0
Variant with	h Desalination Plant Offline			
21	0	0	1.17	0
22	0.4	0	1.17	0
23	0.8	0	1.17	0
24	3	0	1.17	0
25	5	0	1.17	0
26	7	0	1.17	0
27	9	0	1.17	0
28	21	0	1.17	0
29	23.4	0	1.17	0
Variant with	h Normal Flows			
30	0	8.99	1.17	0
31	2	8.99	1.17	0
32	4	8.99	1.17	0
33	6	8.99	1.17	0
34	11	8.99	1.17	0
35	15.92	8.99	1.17	0
Variant with	h High Desal Brine Flows a	nd AWPF Offline	1	
36	0	11.24	0	0
37	3	11.24	0	0
38	5	11.24	0	0
39	9	11.24	0	0
40	12	11.24	0	0
41	16	11.24	0	0
Variant with	h High Desal Brine Flows		•	
42	0	11.24	1.17	0
43	1	11.24	1.17	0
44	4	11.24	1.17	0
45	9	11.24	1.17	0
46	12	11.24	1.17	0
47	16	11.24	1.17	0

## 3.2.2 Ocean Modeling Assumptions

Dr. Roberts documented the modeling assumptions and results in a TM (Roberts, P. J. W., 2017, Appendix D). Changes incorporated into this modeling work compared to the work produced in 2016 included (a) modification to the outfall end gate to include one 6-inch Tideflex valve instead of an open end, (b) analysis of all worst-case ocean conditions, and (c) additional flow scenarios incorporating higher brine discharge flows. The modeling assumptions were specific to ambient ocean conditions: Davidson (November to March), Upwelling (April to August), and Oceanic (September to October). In order to conservatively demonstrate Ocean Plan compliance, the lowest D<sub>m</sub> from the applicable ocean conditions was used for each flow scenario. For all scenarios, the ocean modeling was performed assuming all 129 operational diffuser ports were open.

Three methods were used when modeling the ocean mixing: (1) the Cederwall formula (for neutral and negatively buoyant plumes only), (2) the mathematical model UM $_3$  in the United States Environmental Protection Agency's (EPA's) Visual Plume suite, and (3) the NRFIELD model (for positively buoyant plumes only), also from the EPA's Visual Plume suite (Roberts, P. J. W., 2017). When results were provided from both Cederwall and UM $_3$ , the minimum estimated  $D_m$  value was used in this analysis; when results were provided from both UM $_3$  and NRFIELD, the  $D_m$  value estimated with the UM $_3$  model was selected for consistency, such that all dilution results for buoyant discharges used for this analysis were determined using the same model.

# 4 Ocean Plan Compliance Results

# 4.1 Water Quality of Combined Discharge

As described above, the first step in the Ocean Plan compliance analysis was to estimate the worst-case water quality for the future wastewater discharge components (*viz.*, Desal Brine, secondary effluent, hauled waste and GWR Concentrate). The estimated water quality for each type of discharge is provided in Table 4. Specific assumptions and data sources for each constituent are documented in the Table 4 footnotes.

Constituent	Units	Desal	Seconda	ry Effluent	Hauled	l Waste	GWR	Footnotes
Constituent	Ullits	Brine	MPWSP	Variant	MPWSP	Variant	Concentrate	roothotes
Ocean Plan water quality of	jective	s for prote	ction of m	arine aquati	c life			
Arsenic	μg/L	17.2	45	45	45	45	12	2,6,16,21
Cadmium	μg/L	5.0	1	1.2	1	1.2	6.5	1,7,15,21
Chromium (Hexavalent)	μq/L	ND(<0.03)	ND(<2)	2.5	130	130	13	3,7,15,21

Table 4 – Estimated worst-case water quality for the various discharge waters

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<sup>&</sup>lt;sup>a</sup> Note that RTP wastewater flows have been declining in recent years as a result of conservation; while 24.7 mgd is higher than current RTP wastewater flows, this is expected to be a conservative scenario with respect to ocean modeling, compared to using the current wastewater flows of 16 to 18 mgd.

 $<sup>^{</sup>b}$  A sensitivity analysis was conducted to determine the impacts of hauled waste on the modeled  $D_{m}$  results. It was concluded that neither the flow nor TDS from the addition of hauled waste had a significant impact on the modeled  $D_{m}$  result, and was therefore excluded from the  $D_{m}$  calculation.

<sup>&</sup>lt;sup>17</sup> Note that these ranges assign the transitional months to the ocean condition that is typically more restrictive at relevant discharge flows.



0	11.24	Desal	Seconda	ry Effluent	Hauled	l Waste	GWR	Footootee
Constituent	Units	Brine	MPWSP	Variant	MPWSP	Variant	Concentrate	Footnotes
Copper	μg/L	0.5	11	11	39	39	58	1,7,15,21,28
Lead	μg/L	ND(<0.5)	0.11	2.69	0.76	2.69	14.2	1,7,15,21
Mercury	μg/L	0.414	0.019	0.085	0.044	0.085	0.510	1,10,16,21
Nickel	μg/L	11.0	5.2	12.2	5.2	12.2	64	1,7,15,21
Selenium	μg/L	8.4	4	6.4	75	75	34	1,7,15,21
Silver	μg/L	0.50	0.14	0.77	0.14	0.77	4.05	1,10,15,21
Zinc	μg/L	9.5	20	57.5	170	170	303	1,7,15,21
Cyanide	μg/L	ND(<8.6)	81	89.7	81	89.7	143	1,7,16,17,21
Total Chlorine Residual	μg/L		ND(<200)	ND(<200)	ND(<200)	ND(<200)	ND(<200)	5
Ammonia (as N) 6-mo	μg/L	143.1	42,900	42,900	42,900	42,900	225,789	1,6,15,21,27
median					<i>'</i>			
Ammonia (as N) daily max	μg/L	143.1	49,000	49,000	49,000	49,000	257,895	1,6,15,21,27
Acute Toxicity Chronic Toxicity	TUa TUc		2.3 40	2.3 40	2.3 80	2.3 40	0.77 100	1,12,16,17,24
Phenolic Compounds								1,12,16,17,24
(non-chlorinated)	μg/L	ND(<86.2)	69	69	69	69	363	1,6,14,15,23,2526
Chlorinated Phenolics	μg/L	ND(<34.5)	ND(<20)	ND(<20)	ND(<20)	ND(<20)	ND(<20)	3,9,18,23,25,26
Endosulfan	μg/L	ND(<3.4E-6)	0.015	0.046	0.015	0.046	0.24	1,10,14,15,22,25
Endrin	μg/L	ND(<1.6E-6)	0.000112	0.000112	0.000112	0.000112	0.00059	4,8,15,22
HCH (Hexachlorocyclohexane)	μg/L	0.000043	0.036	0.059	0.036	0.059	0.312	1,10,14,15,22,
Radioactivity (Gross Beta)	pCi/L	ND(<5.17)	32	32	307	307	34.8	25 1,6,12,16,17,23
Radioactivity (Gross Alpha)	pCi/L	22.4	18	18	457	457	14.4	1,6,12,16,17,23
Objectives for protection of					401	401	17.7	1,0,12,10,17,20
Acrolein	μg/L	ND(<3.4)	ND(<5)	8.3	ND(<5)	8.3	44	3,7,15,23
Antimony	μg/L μg/L	0.21	0.65	0.78	0.65	0.78	4.1	1,7,15,21
Bis (2-chloroethoxy) methane	μg/L μg/L	ND(<16.7)	ND(<0.5)	ND(<4.0)	ND(<0.5)	ND(<4.0)	ND(<1)	3,9,18,23
Bis (2-chloroisopropyl) ether	μg/L	ND(<16.7)		ND(<4.0)	ND(<0.5)	ND(<4.0)	ND(<1)	3,9,18,23
Chlorobenzene	μg/L	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
Chromium (III)	μg/L	17	3.0	6.9	87	87	36	2,7,15,21
Di-n-butyl phthalate	μg/L	ND(<16.7)	ND(<5)	ND(<7)	ND(<5)	ND(<7)	ND(<1)	3,9,18,23
Dichlorobenzenes	μg/L	ND(<0.9)	1.6	1.6	1.6	1.6	8.4	1,10,15,21
Diethyl phthalate	μg/L	ND(<0.9)	ND(<5)	ND(<5)	ND(<5)	ND(<5)	ND(<1)	3,9,18,23
Dimethyl phthalate	μg/L	ND(<0.9)	ND(<2)	ND(<2)	ND(<2)	ND(<2)	ND(<0.5)	3,9,18,23
4,6-dinitro-2-methylphenol	μg/L	ND(<84.5)	ND(<0.5)	ND(<19)	ND(<0.5)	ND(<19)	ND(<5)	3,9,18,23
2,4-dinitrophenol	μg/L	ND(<86.2)	ND(<0.5)	ND(<9)	ND(<0.5)	ND(<9)	ND(<5)	3,9,18,23
Ethylbenzene	μg/L	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
Fluoranthene	μg/L	ND(<0.2)	0.00684	0.00684	0.00684	0.00684	0.0360	4,8,15,23
Hexachlorocyclopentadiene	μg/L	ND(<0.09)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.05)	3,9,18,23
Nitrobenzene	μg/L	ND(<41.4)		ND(<2.1)	ND(<0.5)	ND(<2.1)	ND(<1)	3,9,18,23
Thallium	μg/L	ND(<0.1)	ND(<0.5)	0.68	ND(<0.5)	0.68	3.6	3,7,15,21
Toluene	μg/L	ND(<0.9)	0.47	0.48	0.47 ND(<0.05)	0.48	2.5	1,10,15,21
Tributyltin	μg/L		ND(<0.05)	ND(<0.05)		ND(<0.05)	ND(<0.02)	3,13,18,23 3,9,18,21
1,1,1-trichloroethane  Objectives for protection of hu	man has	Ith carein	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,10,21
Acrylonitrile		ND(<3.4)	ND(<2)	2.5	ND(<2)	2.5	13	2 7 15 22
Acrylonitrile	μg/L μg/L			2.5 ND(<0.007)	ND(<2)	2.5 ND(<0.007)	ND(<0.01)	3,7,15,23 3,9,18,23
Benzene	μg/L μg/L	ND(<0.9)	ND(<0.005)	ND(<0.007)	ND(<0.005)	ND(<0.007)	ND(<0.01)	3,9,18,21
Benzidine	μg/L	ND(<86.2)		ND(<18.6)	ND(<0.5)	ND(<18.6)	ND(<0.05)	3,9,18,23
Beryllium	μg/L	ND(<0.9)	ND(<0.5)	ND(<0.68)	0.0052	0.0052	ND(<0.03)	3,9,17,18,21
Bis(2-chloroethyl)ether	μg/L	ND(<41.4)		ND(<4.0)	ND(<0.5)	ND(<4.0)	ND(<1)	3,9,18,23
Bis(2-ethyl-hexyl)phthalate	μg/L	ND(<1.0)	78	78	78	78	411	2,6,15,23
Carbon tetrachloride	μg/L	ND(<0.9)	ND(<0.5)	0.50	ND(<0.5)	0.50	2.66	3,7,15,21
Chlordane	μg/L	1.45E-5	0.00122	0.00122	0.00122	0.00122	0.0064	4,8,14,15,22,25
Chlorodibromomethane	μg/L	ND(<0.9)	ND(<0.5)	2.2	ND(<0.5)	2.2	12	3,7,15,21
Chloroform	μg/L	ND(<0.9)	2	34	2	34	180	2,7,15,21
DDT	μg/L	1.7E-6	0.001	0.001	0.001	0.001	0.0003	4,7,14,19,22,25
1,4-dichlorobenzene	μg/L	ND(<0.9)	1.6	1.6	1.6	1.6	8.4	1,6,15,21
3,3-dichlorobenzidine	μg/L	ND(<86)	ND(<0.03)	ND(<18)	ND(<0.03)	ND(<18)	ND(<2)	3,9,18,23
1,2-dichloroethane	μg/L	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
1,1-dichloroethylene	μg/L	ND(<0.9)	ND(<0.5)	ND(<0.5)	0.5	0.5	ND(<0.5)	3,9,18,21
Dichlorobromomethane Dichloromethane	μg/L	ND(<0.9)	ND(<0.5)	2.4	ND(<0.5)	2.4	12	3,7,15,21
	μg/L	ND(<0.9)	0.88	0.88	0.88	0.88	4.6 3.0	1,7,15,21
1,3-dichloropropene	μg/L	ND(<0.9)	ND(<0.5)	0.56	ND(<0.5)	0.56	ა.0	3,7,15,21



Canatituant	Heite	Desal	Seconda	ry Effluent	Hauled	l Waste	GWR	Factorias
Constituent	Units	Brine	MPWSP	Variant	MPWSP	Variant	Concentrate	Footnotes
Dieldrin	μg/L	4.7E-5	0.0007	0.0015	0.0007	0.0015	0.0001	4,7,19,22
2,4-dinitrotoluene	μg/L	ND(<0.2)	ND(<2)	ND(<2)	ND(<2)	ND(<2)	ND(<0.1)	3,9,18,23
1,2-diphenylhydrazine	μg/L	ND(<16.7)	ND(<0.5)	ND(<4)	ND(<0.5)	ND(<4)	ND(<1)	3,9,18,23
Halomethanes	μg/L	ND(<0.9)	0.54	1.3	0.73	1.3	6.9	2,7,14,15,21
Heptachlor	μg/L	ND(<6.9E-7)	ND(<0.01)	ND(<0.01)	ND(<0.01)	ND(<0.01)	ND(<0.01)	2,9,18,22
Heptachlor epoxide	μg/L	ND(<1.6E-6)	0.000088	0.000088	0.000088	0.000088	0.000463	4,8,15,22
Hexachlorobenzene	μg/L	ND (<6.5E-5)	0.000078	0.000078	0.000078	0.000078	0.000411	4,8,15,22
Hexachlorobutadiene	μg/L	ND(<3.4E-7)	0.000009	0.000009	0.000009	0.000009	0.000047	4,8,15,22
Hexachloroethane	μg/L	ND(<16.7)	ND(<0.5)	ND(<2.1)	ND(<0.5)	ND(<2.1)	ND(<0.5)	3,9,18,23
Isophorone	μg/L	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,23
N-Nitrosodimethylamine	μg/L	ND(<0.003)	0.017	0.086	0.017	0.086	0.150	2,7,16,17,23
N-Nitrosodi-N-Propylamine	μg/L	ND(<0.003)	0.076	0.076	0.076	0.076	0.019	2,6,16,17,23
N-Nitrosodiphenylamine	μg/L	ND(<16.7)	ND(<0.5)	ND(<2.1)	ND(<0.5)	ND(<2.1)	ND(<1)	3,9,18,23
PAHs	μg/L	2.2E-3	0.04	0.04	0.04	0.04	0.21	4,7,14,15,22,25
PCBs	μg/L	0.00013	0.00068	0.00068	0.00068	0.00068	0.00357	4,8,14,15,22,25
TCDD Equivalents	μg/L	ND (<2.5E-5)	1.37E-7	1.39E-7	1.37E-7	1.39E-7	7.29E-7	4,7,13,14,15,23, 25
1,1,2,2-tetrachloroethane	μg/L	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
Tetrachloroethylene	μg/L	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
Toxaphene	μg/L	3.97E-5	0.0071	0.0071	0.0071	0.0071	0.0373	4,8,15,22
Trichloroethylene	μg/L	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
1,1,2-trichloroethane	μg/L	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
2,4,6-trichlorophenol	μg/L	ND(<16.7)	ND(<0.5)	ND(<2.1)	ND(<0.5)	ND(<2.1)	ND(<1)	3,9,18,23
Vinyl chloride	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21

#### **Table 4 Footnotes:**

#### MPWSP Secondary Effluent and Hauled Waste

<sup>1</sup> The value reported is based on MRWPCA historical data.

### Total Chlorine Residual

<sup>5</sup> For all waters, it is assumed that dechlorination will be provided such that the total chlorine residual will be below detection.

### Variant Secondary Effluent and Hauled Waste

- <sup>6</sup> Existing RTP effluent exceeds concentrations observed in other proposed source waters; the value reported is the existing secondary effluent value.
- <sup>7</sup> The proposed new source waters may increase the secondary effluent concentration; the value reported is based on estimated source water blends.
- <sup>8</sup> RTP effluent value is based on CCLEAN data; no other source waters were considered due to MRL differences.
- <sup>9</sup> MRL provided represents the maximum flow-weighted MRL based on the blend of source waters.
- <sup>10</sup> The only water with a detected concentration was the RTP effluent, however the flow-weighted concentration increases due to higher MRLs for the proposed new source waters.
- <sup>11</sup> Additional source water data are not available; the reported value is for RTP effluent.
- <sup>12</sup> Calculation of the flow-weighted concentration was not feasible due to constituent. The maximum observed value is reported.
- <sup>13</sup> Agricultural Wash Water data are based on an aerated sample, instead of a raw water sample.

<sup>&</sup>lt;sup>2</sup> The value reported is based on secondary effluent data collected during the GWR Project source water monitoring programs (not impacted by the proposed new source waters), and are representative of future water quality under the MPWSP scenario.

<sup>&</sup>lt;sup>3</sup> The MRL provided represents the Maximum Reported Value in Table F-3 of MRWPCA's current NPDES permit. There are two exceptions to this statement: (1) the maximum reported value for hexavalent chromium was disregarded as it was the concentration measured in the hauled waste, not the secondary effluent (2) chlorinated phenolics was not included in Table F-3, and so the MRL provided is the reported value from MRWPCA's priority pollutant monitoring.



<sup>14</sup> This value in the Ocean Plan is an aggregate of several congeners or compounds. Per the approach described in the Ocean Plan, for cases where the individual congeners/compounds were less than the MRL, a value of 0 is assumed in calculating the aggregate value.

#### **GWR** Concentrate Data

- <sup>15</sup> The value presented represents a calculated value assuming no removal prior to RO, complete rejection through RO membrane, and an 81% RO recovery.
- <sup>16</sup> The value represents the maximum value observed during the pilot testing study.
- <sup>17</sup> The calculated value for the AWPF data (described in note 15) was not used in the analysis because it was not considered representative. It is expected that the value would increase as a result of treatment through the AWPF (*e.g.* formation of N-Nitrosodimethylamine as a disinfection by-product), or that it will not concentrate linearly through the RO (*e.g.* toxicity and radioactivity).
- <sup>18</sup> The MRL provided represents the limit from the source water and pilot testing monitoring programs.
- <sup>19</sup> The value presented represents a calculated value assuming 93% and 84% removal through primary and secondary treatment for DDT and dieldrin, respectively, 36% and 44% removal through ozone for DDT and dieldrin, respectively, 92% and 97% removal through MF for DDT and dieldrin, respectively, recycling of the MF backwash to the RTP, complete rejection through the RO membrane, and an 81% RO recovery. The assumed removals are based on results from ozone bench-scale testing of Blanco Drain water blended with secondary effluent and low detection sampling through the RTP.
- <sup>20</sup> Footnote not used

#### Desal Brine Data

- <sup>21</sup> The value reported is based on test slant well data collected through the Watershed Sanitary Survey.
- <sup>22</sup> The value reported is based on data from the one-time 7-day composite sample from the test slant well. If ND, the method detection limit was used for the analysis instead of the MRL. MRLs were not available for this data set.

  <sup>23</sup> The value reported is based on data from the test slant well collected through the quarterly Ocean Plan constituents monitoring.
- <sup>24</sup> Acute and chronic toxicity have not been measured or estimated
- <sup>25</sup> This value in the Ocean Plan is an aggregate of several congeners or compounds. Per the approach described in the Ocean Plan, for cases where the individual congeners/compounds were less than the MRL, a value of 0 is assumed in calculating the aggregate value.
- <sup>26</sup> Chlorinated phenolic compounds is the sum of the following: 4-chloro-3-methylphenol, 2-chlorophenol, pentachlorophenol, 2,4,5-trichlorophenol, and 2,4,6-trichlorophenol. Non-chlorinated phenolic compounds is the sum of the following: 2,4-dimethylphenol, 4,6-Dinitro-2-methylphenol, 2,4-dinitrophenol, 2-methylphenol, 4-methylphenol, 2-nitrophenol, 4-nitrophenol, and phenol.

## General

- <sup>27</sup> Ammonia (as N) represents the total ammonia concentration, *i.e.* the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>).
- <sup>28</sup> The value reported for the Variant secondary effluent was calculated using the median of the data collected for the new source waters and is an estimate of the potential increase in concentration of the secondary effluent based on estimated source water blends. The value reported for the Desal Brine was calculated with the median of the data collected from the test slant well and assuming a 42% recovery through the RO. The median values were used because the maximum values detected in both sources appear to be outliers, and because the Ocean Plan objective is a 6-month median concentration, it is reasonable to use the median value detected from these source waters.

# 4.2 Ocean Modeling Results

The resulting estimates of minimum probable dilution  $(D_m)$  for each discharge scenario are presented in Tables 5 and 6 (Roberts, P. J. W., 2017). For discharge scenarios that were modeled with more than one modeling method, the lowest  $D_m$  (*i.e.*, most conservative) is reported in the tables below. For the MPWSP, the flow scenarios in which little or no secondary effluent was discharged (Scenarios 1, 2, 8, and 9) resulted in the lowest  $D_m$  values as a result of the discharge plume being negatively buoyant. At higher secondary effluent flows, the discharge plume would



be positively buoyant, resulting in an increased D<sub>m</sub>, as evidenced in Scenarios 7 and 14. The same trend was observed for Variant scenarios.

The estimates of minimum probable dilution ( $D_m$ ) for the MPWSP range from 14.4 to 98, and 14.4 to 114 for the Variant. These  $D_m$  values are substantially lower than what is currently specified in the MRWPCA NPDES permit (145) and those estimated for the GWR Project, which range from 174 to 498 (see Appendix B). As a result of the reduced dilution, some contaminants, which have not traditionally been of concern for discharge through MRWPCA's ocean outfall, are estimated to potentially exceed the Ocean Plan objectives at the edge of the ZID.

Table 5 – Flow scenarios and modeled D<sub>m</sub> values used for Ocean Plan compliance analysis for MPWSP

Flow		Disc	charge flows (mg	ıd)	
Scenario No.	Ocean Condition	Secondary Effluent <sup>a</sup>	Desal Brine	Hauled Waste	D <sub>m</sub> b
MPWSP wi	th Normal Desal Brine Flow				
1	Davidson	0	13.98	0.1	14.4
2	Davidson	2	13.98	0.1	15.8
3	Davidson	4	13.98	0.1	17.8
4	Davidson	6	13.98	0.1	20.9
5	Davidson	9	13.98	0.1	26.7
6	Upwelling	10	13.98	0.1	38.2
7	Upwelling	19.78	13.98	0.1	98
MPWSP wi	th High Desal Brine Flow				
8	Davidson	0	16.31	0.1	14.5
9	Davidson	2	16.31	0.1	15.7
10	Davidson	7	16.31	0.1	21.8
11	Davidson	8	16.31	0.1	23.5
12	Davidson	10	16.31	0.1	29.2
13	Davidson	12	16.31	0.1	43.9
14	Oceanic	16	16.31	0.1	87

<sup>&</sup>lt;sup>a</sup> Note that RTP wastewater flows have been declining in recent years as a result of conservation; while 19.68 mgd is higher than current RTP wastewater flows, this is expected to be a conservative scenario with respect to ocean modeling, compared to using the current wastewater flows of 16 to 18 mgd.

<sup>&</sup>lt;sup>b</sup> Several models were used to estimate the minimal probable dilution value (UM<sub>3</sub>, Cederwall for neutral and negatively buoyant plumes, and NRFIELD for buoyant plumes). Values included here are the model results ( $D_m$  values) that resulted in the lowest  $D_m$ . The Ocean Plan defines dilution differently than Dr. Roberts. Dr. Roberts provided results defined as S = [total volume of a sample]/[volume of effluent contained in the sample]. The  $D_m$  referenced in Equation 1 of the California Ocean Plan is defined as  $D_m = S - 1$ . A value of 1 was subtracted from the dilution estimates provided by Dr. Roberts prior to using Equation 1.



Table 6 - Flow scenarios and modeled D<sub>m</sub> values used for Ocean Plan compliance analysis for Variant

Flow			Discharge flo	ows (mgd)	_	
Scenario No.	Ocean Condition	Secondary Effluent <sup>a</sup>	Desal Brine	GWR Concentrate	Hauled Waste <sup>b</sup>	D <sub>m</sub> <sup>c</sup>
Variant with	h AWPF Offline		•			
15	Davidson	0	8.99	0	0	15.7
16	Davidson	2	8.99	0	0	16.4
17	Davidson	4	8.99	0	0	19.9
18	Davidson	5.8	8.99	0	0	28.4
19	Upwelling	14	8.99	0	0	109.0
20	Upwelling	19.78	8.99	0	0	117.0
Variant with	n Normal Flows		•			
30	Davidson	0	8.99	1.17	0	15.5
31	Davidson	2	8.99	1.17	0	17.7
32	Davidson	4	8.99	1.17	0	23.8
33	Davidson	6	8.99	1.17	0	67.5
34	Upwelling	11	8.99	1.17	0	106.0
35	Upwelling	15.92	8.99	1.17	0	114.0
Variant with	h High Desal Brine Flows and AV	/PF Offline				
36	Davidson	0	11.24	0	0	14.4
37	Davidson	3	11.24	0	0	17.1
38	Davidson	5	11.24	0	0	20.5
39	Upwelling	9	11.24	0	0	90.0
40	Oceanic	12	11.24	0	0	94.0
41	Upwelling	16	11.24	0	0	102.0
Variant with	h High Desal Brine Flows					
42	Davidson	0	11.24	1.17	0	15.2
43	Davidson	1	11.24	1.17	0	16.0
44	Davidson	4	11.24	1.17	0	20.8
45	Upwelling	9	11.24	1.17	0	90.0
46	Upwelling	12	11.24	1.17	0	97.0
47	Upwelling	16	11.24	1.17	0	104

<sup>&</sup>lt;sup>a</sup> Note that RTP wastewater flows have been declining in recent years as a result of conservation; while 19.68 mgd is higher than current RTP wastewater flows, this is expected to be a conservative scenario with respect to ocean modeling, compared to using the current wastewater flows of 16 to 18 mgd.

 $<sup>^{\</sup>rm b}$  Hauled waste was not included in the modeling of MPWSP flow scenarios; however, the change in both flow and TDS from the addition of hauled waste is less than 1% and thus is expected to have a negligible impact on the modeled  $D_{\rm m}$ .

<sup>&</sup>lt;sup>c</sup> Several models were used to estimate the minimal probable dilution value (UM<sub>3</sub>, Cederwall for neutral and negatively buoyant plumes, and NRFIELD for buoyant plumes). Values included here are the model results ( $D_m$  values) that resulted in the lowest  $D_m$ . The Ocean Plan defines dilution differently than Dr. Roberts. Dr. Roberts provided results defined as S = [total volume of a sample]/[volume of effluent contained in the sample]. The  $D_m$  referenced in Equation 1 of the California Ocean Plan is defined as  $D_m = S - 1$ . A value of 1 was subtracted from the dilution estimates provided by Dr. Roberts prior to using Equation 1.

# 4.3 Ocean Plan Compliance Results

The flow-weighted in-pipe concentration for each constituent was calculated for each modeled discharge scenario using the water quality presented in Table 4 and the discharge flows presented in Tables 2 and 3. The in-pipe concentration was then used to calculate the concentration at the edge of the ZID using the D<sub>m</sub> values presented in Tables 5 and 6. The resulting concentrations for each constituent in each scenario were compared to the Ocean Plan objectives to assess compliance. The estimated concentrations for the 47 flow scenarios (14 for the MPWSP and 33 for the Variant) for all constituents are presented as concentrations at the edge of the ZID (Appendix A, Table A1 and A3) and as a percentage of the Ocean Plan objective (Appendix A, Table A2 and A4).

Some constituents were estimated to potentially exceed or come close to exceeding the Ocean Plan water quality objectives for the MPWSP and Variant; however, some of these constituents were never detected above the MRL in any of the source waters, but the MRLs are higher than the Ocean Plan objective. Due to this insufficient analytical sensitivity, no compliance conclusion can be drawn for these constituents. This is a common occurrence for ocean discharges since the MRL of the approved compliance analysis method is higher than the Ocean Plan objective for certain constituents.

Of the constituents detected in the source waters, two (cyanide and ammonia) were identified as having potential to exceed the Ocean Plan objective in the MPWSP, and eight (cyanide, ammonia, acrylonitrile, beryllium, chlordane, PCBs, TCDD equivalents, and toxaphene) were identified as having potential to exceed the Ocean Plan objective in the Variant. Within this Variant subset of eight constituents, acrylonitrile, beryllium and TCDD equivalents were detected in some of the source waters, but not in the others. For these analyses, the MRLs themselves were above the Ocean Plan objective. To assess the blended concentrations for these constituents, a value of zero was assumed for any sources when the concentration was below the MRL. 18 This approach is a "best-case" scenario because it assumes the lowest possible concentration—namely, a value of zero—for any constituent below the reporting limit. This approach is still useful, however, to bracket the analysis and assess the potential for Ocean Plan compliance issues under best-case conditions. Through this method, TCDD equivalents continues to show potential to exceed the Ocean Plan objective for the Variant. The estimated concentration of acrylonitrile<sup>19</sup> and beryllium at the edge of the ZID is less than the Ocean Plan objective and therefore did not show exceedances through this "best-case" analysis. However, because this is only a partial analysis (a special case), it is not possible to draw conclusions on whether acrylonitrile and beryllium will comply with the Ocean Plan during actual conditions.

The constituents that may exceed the Ocean Plan objective, or come close to exceeding the objective, are shown at their estimated concentration at the edge of the ZID in Table 7 for the MPWSP and Table 8 for the Variant, and as the concentration at the edge of the ZID as a

<sup>&</sup>lt;sup>18</sup> Additionally, the Ocean Plan states that for constituents that are made up of an aggregate of constituents, a concentration of 0 can be assumed for the individual constituents that are not detected above the MRL, such as TCDD equivalents.

<sup>&</sup>lt;sup>19</sup> Acrylonitrile was only detected in one potential source water for the Variant. It was not detected in any potential source waters for the MPWSP Project; therefore, a compliance determination cannot be made for the MPWSP Project and only partial determination can be made for the Variant.



percentage of the Ocean Plan objective in Table 9 and 10 for the MPWSP and Variant, respectively. The "best-case" scenario compliance assessment results for acrylonitrile and TCDD equivalents are also included in these tables.

Table 7 – Estimated concentrations at the edge of the ZID for Ocean Plan constituents of concern in the MPWSP a

		Ocean Plan					Estimate	ed Concen	tration at E	dge of ZID	by Flow S	Scenario				
Constituent	Units	Ocean Plan Objective				MPWSP					MP	WSP with	High Desa	Brine Flo	ws	
		0.0,000.10	1	2	3	4	5	6	7	8	9	10	11	12	13	14
Objectives for protection of marine	aquatic life	- 6-month me	edian limit													
Cyanide	μg/L	1	0.6	1.1	1.3	1.4	1.3	1.0	0.5	0.6	1.0	1.3	1.3	1.2	0.9	0.5
Ammonia (as N) – 6-mo median <sup>b</sup>	μg/L	600	29	341	523	600	614	461	255	26	301	575	585	546	409	243
Objectives for protection of human	health - ca	rcinogens - 30	)-day aver	age limit <sup>c (</sup>	i											
Acrylonitrile <sup>c d</sup>	μg/L	0.1					1				1			-		
Bis(2-ethyl-hexyl)phthalate	μg/L	4	0.1	0.7	1.0	1.1	1.1	0.8	0.5	0.1	0.6	1.1	1.1	1.0	0.8	0.4
Chlordane	μg/L	2.3E-05	1.5E-06	1.0E-05	1.5E-05	1.7E-05	1.8E-05	1.3E-05	7.3E-06	1.4E-06	9.1E-06	1.7E-05	1.7E-05	1.6E-05	1.2E-05	6.9E-06
PCBs	μg/L	1.9E-05	8.9E-06	1.2E-05	1.4E-05	1.4E-05	1.3E-05	9.2E-06	4.6E-06	8.8E-06	1.2E-05	1.3E-05	1.3E-05	1.1E-05	8.1E-06	4.6E-06
TCDD Equivalents <sup>d</sup>	μg/L	3.9E-09	6.3E-11	1.1E-09	1.7E-09	1.9E-09	1.9E-09	1.5E-09	8.1E-10	5.4E-11	9.4E-10	1.8E-09	1.9E-09	1.7E-09	1.3E-09	7.7E-10
Toxaphene e	μg/L	2.1E-04	5.8E-06	5.7E-05	8.7E-05	1.0E-04	1.0E-04	7.6E-05	4.2E-05	5.3E-06	5.1E-05	9.6E-05	9.7E-05	9.1E-05	6.8E-05	4.0E-05

a: Shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.

b: Ammonia (as N) represents the total ammonia concentration, *i.e.* the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>).

c: Acrylonitrile was only detected in one potential source water for the Variant Project. It was not detected in any potential source waters for the MPWSP Project; therefore, a compliance determination cannot be made for the MPWSP Project and only partial determination can be made for the Variant Project.

d: Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time. When only the detected values were considered, beryllium did not exceed the Ocean Plan objective and therefore was not included in Tables 7 through 10.

e: Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.

Table 8 – Estimated concentrations at the edge of the ZID for Ocean Plan constituents of concern in the Variant <sup>a</sup>

										Е	stimate	ed Con	centrat	ion at E	Edge of	ZID by	Flow S	cenari	0							
Constituent	Units	Ocean Plan Objective		Varia	nt with	GWR C	offline			Varian	t with I	Normal	Flows		Varia	nt with	High Do	esal Br Offline		ws and	Vari	iant wit	h High	Desal I	Brine F	lows
		Objective	15	16	17	18	19	20	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47
Objectives for pro	otection	of marine ac	quatic li	fe - 6-	month	mediar	limit																			
Cyanide	μg/L	1	0.6	1.4	1.6	1.4	0.5	0.5	1.5	1.9	1.7	0.7	0.5	0.6	0.6	1.4	1.6	0.5	0.5	0.5	1.3	1.6	1.8	0.6	0.6	0.6
Ammonia (as N)  – 6-mo median b	μg/L	600	39	474	648	581	239	251	1593	1551	1248	473	326	316	34	519	627	212	235	246	1333	1363	1227	335	327	320
Objectives for pro	otection	of human he	ealth - c	arcino	gens -	30-day	averaç	je limit	c d																	
Acrylonitrile cd	μg/L	0.1	0.002	0.03	0.04	0.03	0.01	0.01	0.1	0.1	0.1	0.03	0.02	0.02	0.001	0.03	0.04	0.01	0.01	0.01	0.1	0.1	0.1	0.02	0.02	0.02
Bis(2-ethyl- hexyl)phthalate	μg/L	4	0.1	0.9	1.2	1.1	0.4	0.5	2.9	2.9	2.3	0.9	0.6	0.6	0.1	1.0	1.2	0.4	0.4	0.5	2.5	2.5	2.3	0.6	0.6	0.6
Chlordane	μg/L	2.3E-05	2E-06	1E-05	2E-05	2E-05	7E-06	7E-06	5E-05	4E-05	4E-05	1E-05	9E-06	9E-06	2E-06	2E-05	2E-05	6E-06	7E-06	7E-06	4E-05	4E-05	4E-05	1E-05	9E-06	9E-06
PCBs	μg/L	1.9E-05	9E-06	1E-05	1E-05	1E-05	4E-06	4E-06	3E-05	3E-05	2E-05	9E-06	6E-06	5E-06	9E-06	1E-05	1E-05	4E-06	4E-06	4E-06	3E-05	3E-05	2E-05	6E-06	6E-06	6E-06
TCDD Equivalents <sup>d</sup>	μg/L	3.9E-09	1E-10	2E-09	2E-09	2E-09	8E-10	8E-10	5E-09	5E-09	4E-09	2E-09	1E-09	1E-09	8E-11	2E-09	2E-09	7E-10	8E-10	8E-10	4E-09	4E-09	4E-09	1E-09	1E-09	1E-09
Toxaphene e	μg/L	2.1E-04	7E-06	8E-05	1E-04	1E-04	4E-05	4E-05	3E-04	3E-04	2E-04	8E-05	5E-05	5E-05	7E-06	9E-05	1E-04	4E-05	4E-05	4E-05	2E-04	2E-04	2E-04	6E-05	5E-05	5E-05

a: Shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.

b: Ammonia (as N) represents the total ammonia concentration, i.e. the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>).

c: Acrylonitrile was only detected in one potential source water for the Variant Project. It was not detected in any potential source waters for the MPWSP Project; therefore, a compliance determination cannot be made for the MPWSP Project and only partial determination can be made for the Variant Project.

d: Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time. When only the detected values were considered, beryllium did not exceed the Ocean Plan objective and therefore was not included in Tables 7 through 10.

e: Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.

Table 9 – Estimated concentrations at the edge of the ZID expressed as percentage of Ocean Plan Objective for constituents of in the MPWSP a

		Ocean Plan				Est. F	Percentage	of Ocean	Plan objec	ective at Edge of ZID by Flow Scenario								
Constituent	Units	Ocean Plan Objective				MPWSP					MP	WSP with	High Desa	l Brine Flo	ws			
			1	2	3	4	5	6	7	8	9	10	11	12	13	14		
Objectives for protection of marine a	aquatic life	- 6-month m	edian limit															
Cyanide	μg/L	1	59%	108%	133%	140%	134%	99%	52%	58%	101%	134%	133%	120%	88%	51%		
Ammonia (as N) – 6-mo median b	μg/L	600	5%	57%	87%	100%	102%	77%	43%	4%	50%	96%	97%	91%	68%	40%		
Objectives for protection of human l	health - ca	rcinogens - 30	-day average limit <sup>c d</sup>															
Acrylonitrile <sup>c d</sup>	μg/L	0.1		-									-		-			
Bis(2-ethyl-hexyl)phthalate	μg/L	4	3%	19%	28%	32%	32%	24%	13%	3%	17%	31%	31%	29%	22%	13%		
Chlordane	μg/L	2.3E-05	6%	44%	66%	75%	77%	57%	32%	6%	39%	72%	73%	68%	51%	30%		
PCBs	μg/L	1.9E-05	47%	64%	72%	72%	66%	49%	24%	46%	61%	69%	67%	60%	43%	24%		
TCDD Equivalents d	μg/L	3.9E-09	2%	27%	42%	49%	50%	38%	21%	1%	24%	47%	48%	44%	33%	20%		
Toxaphene e	μg/L	2.1E-04	3%	27%	42%	47%	48%	36%	20%	3%	24%	45%	46%	43%	32%	19%		

a: Shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.

b: Ammonia (as N) represents the total ammonia concentration, i.e. the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>).

c: Acrylonitrile was only detected in one potential source water for the Variant Project. It was not detected in any potential source waters for the MPWSP Project; therefore, a compliance determination cannot be made for the MPWSP Project and only partial determination can be made for the Variant Project.

d: Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time. When only the detected values were considered, beryllium did not exceed the Ocean Plan objective and therefore was not included in Tables 7 through 10.

e: Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.

Table 10 – Estimated concentrations at the edge of the ZID expressed as percentage of Ocean Plan Objective for constituents of in the Variant <sup>a</sup>

				Est. Percentage of Ocean Plan objective at Edge of ZID by Flow Scenario																						
Constituent	Units	Ocean Plan Objective		Varia	nt with	GWR O	ffline			Variar	nt with N	Normal	Flows		Varia	nt with		esal Brii Offline	ne Flow	s and	Var	iant wit	h High	Desal B	rine Flo	ws
		Objective	15	16	17	18	19	20	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47
Objectives for pr	otection	of marine	aquatic	life - 6	-month	mediar	ı limit																			
Cyanide	μg/L	1	61%	138%	163%	139%	53%	55%	150%	189%	173%	71%	55%	56%	61%	144%	158%	49%	53%	55%	135%	158%	176%	55%	56%	57%
Ammonia (as N) – 6-mo median <sup>b</sup>	μg/L	600	7%	79%	108%	97%	40%	42%	266%	258%	208%	79%	54%	53%	6%	86%	105%	35%	39%	41%	222%	227%	205%	56%	54%	53%
Objectives for pr	otection	n of human i	health -	carcino	ogens	- 30-day	averaç	ge limit	c d																	
Acrylonitrile cd	μg/L	0.1	2%	28%	38%	34%	14%	14%	94%	92%	74%	28%	19%	19%	1%	30%	37%	13%	14%	15%	79%	81%	73%	20%	19%	19%
Bis(2-ethyl- hexyl)phthalate	μg/L	4	3%	26%	34%	31%	12%	13%	84%	81%	65%	25%	17%	17%	3%	28%	33%	11%	12%	13%	70%	72%	64%	18%	17%	17%
Chlordane	μg/L	2.3E-05	8%	60%	81%	72%	30%	31%	199%	193%	155%	59%	40%	39%	7%	66%	79%	26%	29%	30%	167%	170%	153%	42%	40%	40%
PCBs	μg/L	1.9E-05	47%	71%	77%	63%	22%	23%	169%	156%	121%	45%	30%	28%	47%	73%	74%	22%	23%	23%	149%	147%	124%	32%	30%	29%
TCDD Equivalents <sup>d</sup>	μg/L	3.9E-09	2%	39%	53%	48%	20%	21%	131%	128%	103%	39%	27%	26%	2%	42%	52%	17%	19%	20%	110%	112%	101%	28%	27%	26%
Toxaphene e	μg/L	2.1E-04	4%	38%	51%	46%	19%	20%	126%	122%	98%	37%	26%	25%	3%	41%	50%	17%	19%	19%	105%	108%	97%	26%	26%	25%

a: Shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.

b: Ammonia (as N) represents the total ammonia concentration, i.e. the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>).

c: Acrylonitrile was only detected in one potential source water for the Variant Project. It was not detected in any potential source waters for the MPWSP Project; therefore, a compliance determination cannot be made for the MPWSP Project and only partial determination can be made for the Variant Project.

d: Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time. When only the detected values were considered, beryllium did not exceed the Ocean Plan objective and therefore was not included in Tables 7 through 10.

e: Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.

Potential issues for cyanide and ammonia compliance were identified to occur when there is no, or relatively low secondary effluent flow mixed with hauled waste and Desal Brine, as in MPWSP Scenarios 2-6 and 9-13. Potential issues were also identified to occur when there is little or no secondary effluent flow discharged for the Variant Project, as in Variant Scenarios 16-18, 30-32, 37, 38, and 42-44. The constituents of interest related to these scenarios are cyanide, ammonia, acrylonitrile, bis(2-ethyl-hexyl)phthalate, chlordane, PCBs, TCDD equivalents, and toxaphene. Ammonia is expected to be the constituent with the highest exceedance, being 2.66 times the Ocean Plan objective in flow scenario 30 (0 mgd secondary effluent with hauled waste, 1.17 mgd GWR Concentrate and 8.99 mgd Desal Brine). This scenario is problematic because constituents that have relatively high loadings in the secondary effluent are concentrated in the GWR Concentrate. This scenario assumes the GWR Concentrate flow is much smaller than the Desal Brine flow, such that the resulting discharge plume is negatively buoyant and achieves poor ocean dilution.

Chlordane, PCBs, and toxaphene were only detected when analyzed with low-detection methods, which have far greater sensitivity than standard methods. These results were used to investigate potential to exceed Ocean Plan objectives because these objectives are orders of magnitude below detection limits of methods currently used for discharge compliance.

# 5 Conclusions

The purpose of this analysis was to assess the ability of the MPWSP and Variant to comply with the Ocean Plan objectives. Trussell Tech used a conservative approach to estimate the water qualities of the secondary effluent, GWR Concentrate, Desal Brine and hauled waste for these projects. These water quality data were then combined for various discharge scenarios, and a concentration at the edge of the ZID was calculated for each constituent and scenario. A summary of the constituents that show potential to exceed the Ocean Plan objectives is provided in Table 11 for the MPWSP and Table 12 for the Variant. These constituents can be divided into three categories:

- Category I Insufficient analytical sensitivity to determine compliance: The constituent was not detected above the MRL in any of the source waters, but the MRL is not sensitive enough to demonstrate compliance with the Ocean Plan objective.
- Category II Estimated to be close to exceeding the Ocean Plan objective: The constituent is estimated to be at a concentration between 80% and 100% of the Ocean Plan objective at the edge of the ZID.
- Category III Estimated to exceed the Ocean Plan objective: The constituent is estimated to be at a concentration higher than the Ocean Plan objective at the edge of the ZID.

	Category I <sup>a</sup>	Category II <sup>b</sup>	Category III <sup>c</sup>		st Case edance
Constituent	Compliance Determination Not Possible	Estimated to be Close to Exceeding Objective	Estimated to Exceed Objective	Flow Scenario	Estimated Percentage of Objective at edge of ZID
Cyanide <sup>d</sup>			✓	4	140%
Ammonia			✓	5	102%
Chlorinated Phenolics	✓				
2,4-Dinitrophenol	✓				
Tributyltin	✓				
Acrylonitrile e	✓				
Aldrin	✓				
Benzidine	✓				
Beryllium <sup>e</sup>	✓				
Bis(2-chloroethyl)ether	✓				
3,3-Dichlorobenzidine	✓				
1,2-Diphenylhydrazine (azobenzene)	✓				
Heptachlor	✓				
TCDD Equivalents e	✓				
2,4,6-Trichlorophenol	<b>√</b>				

Table 11: Summary of Compliance Conclusions for the MPWSP

## Notes:

- **a:** ND in all sources, but MRL higher than Ocean Plan objective and therefore unable to demonstrate compliance. Exceptions are: MRL for 2,4-dinitrophenol was less than objective in secondary effluent and MRL for heptachlor was less than objective in slant well.
- **b**: Concentration of constituent at the edge of the ZID is estimated to be between 80% and 100% of the Ocean Plan objective for some scenarios
- c: Concentration of constituent is estimated to be > 100% of the Ocean Plan objective for some scenarios at the edge of the ZID
- d: Issues with approved analytical methods may have resulted in erroneously high cyanide quantification
- **e**: Only a best-case scenario could be evaluated, where a value of 0 was assumed when the constituent was ND and the MRL was larger than the Ocean Plan objective

<b>Constituent</b> Det	ompliance termination	Estimated to			
Ammonia Chlorinated Phenolics	t Possible	be Close to Exceeding Objective	Estimated to Exceed Objective	Flow Scenario	Estimated Percentage of Objective at edge of ZID
Chlorinated Phenolics			✓	31	189%
			✓	30	266%
2,4-Dinitrophenol	✓				
	✓				
Tributyltin	✓				
Acrylonitrile e		✓		30	94%
Aldrin	✓				
Benzidine	✓				
Beryllium <sup>e</sup>	✓				
Bis(2-chloroethyl)ether	✓				
Bis(2-ethyl-hexyl)phthalate		✓		30	84%
Chlordane			<b>✓</b>	30	199%
3,3-Dichlorobenzidine	✓				
1,2-Diphenylhydrazine (azobenzene)	✓				
Heptachlor	<b>-</b> ✓				
PCBs			✓	30	169%
TCDD Equivalents <sup>e</sup>			✓	30	131%
Toxaphene			✓	30	126%
2,4,6-Trichlorophenol	1				

Table 12: Summary of Compliance Conclusions for the Variant

#### Notes:

- **a:** ND in all sources, but MRL higher than Ocean Plan objective and therefore unable to demonstrate compliance. Exceptions are: MRL for 2,4-dinitrophenol was less than objective in secondary effluent and MRL for heptachlor was less than objective in slant well.
- **b**: Concentration of constituent at the edge of the ZID is estimated to be between 80% and 100% of the Ocean Plan objective for some scenarios
- c: Concentration of constituent is estimated to be > 100% of the Ocean Plan objective for some scenarios at the edge of the ZID
- d: Issues with approved analytical methods may have resulted in erroneously high cyanide quantification
- **e:** Only a best-case scenario could be evaluated, where a value of 0 was assumed when the constituent was ND and the MRL was larger than the Ocean Plan objective

Based on the data, assumptions, modeling, and analytical methodology presented in this TM, the MPWSP and Variant show a potential to exceed certain Ocean Plan objectives under specific discharge scenarios (see Tables 11 and 12). In particular, potential issues were identified for the MPWSP and Variant flow scenarios involving low to moderate secondary effluent flows with Desal Brine. Under these conditions, discharges are estimated to exceed or come close to exceeding multiple Ocean Plan objectives, specifically those for cyanide and ammonia for the MPWSP, and cyanide, ammonia, chlordane, PCBs, TCDD equivalents, and toxaphene for the

Variant. Ammonia clearly exceeds the Ocean Plan objective and must be resolved for the MPWSP and Variant. When considering a best-case analysis for the Variant, acrylonitrile comes close to exceeding the Ocean Plan objective, and TCDD equivalents show a potential to exceed the objective. Additional analytical investigation regarding cyanide analysis is recommended to determine if the potential exceedances are representative of actual water quality conditions. Chlordane, PCBs and toxaphene, which were estimated to exceed the objectives for Variant flow scenarios, were detected at concentrations that are orders of magnitude below detection limits of methods currently used for discharge compliance.



# 6 References

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# **Appendix A**

Table A1 – Complete list of estimated concentrations of Ocean Plan constituents at the edge of the ZID for the MPWSP

							Estimate	ed Concen	tration at E	Edge of ZID	by Flow S	Scenario				
Constituent	Units	Ocean Plan				MPWSP					MP	WSP with	High Desa	l Brine Flo	ws	
		Objective	1	2	3	1	5	6	7	8	q	10	11	12	13	14
Objectives for protection of marine a	acuatia life	6 month ma	dian limit		<b>J</b>		<b>J</b>	· ·	_ '	•	3	10		12	10	17
Arsenic	ug/L	8 8	3.9	4.1	4.1	4.0	3.9	3.7	3.3	3.9	4.0	4.0	4.0	3.8	3.6	3.3
Cadmium	μg/L	1	0.3	0.3	0.2	0.2	0.1	0.1	0.03	0.3	0.3	0.2	0.1	0.1	0.1	0.03
Chromium (Hexavalent)	ug/L	2	0.06	0.06	0.06	0.06	0.05	0.04	0.02	0.05	0.06	0.05	0.05	0.04	0.03	0.03
Copper	µg/L	3	1.9	2.0	2.1	2.1	2.1	2.1	2.0	1.9	2.0	2.1	2.1	2.1	2.1	2.0
Lead	µg/L	2	0.03	0.03	0.02	0.02	0.01	0.01	0.003	0.03	0.03	0.02	0.02	0.01	0.01	0.004
Mercury	µg/L	0.04	0.03	0.02	0.02	0.01	0.01	0.01	0.002	0.03	0.02	0.01	0.01	0.01	0.01	0.003
Nickel	µg/L	5	0.7	0.6	0.5	0.4	0.3	0.2	0.1	0.7	0.6	0.4	0.4	0.3	0.2	0.1
Selenium	µg/L	15	0.6	0.5	0.4	0.3	0.3	0.2	0.1	0.6	0.5	0.3	0.3	0.2	0.2	0.1
Silver	μg/L	0.7	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
Zinc	μg/L	20	8.2	8.2	8.2	8.2	8.2	8.2	8.1	8.2	8.2	8.2	8.2	8.2	8.1	8.1
Cyanide	µg/L	1	0.6	1.1	1.3	1.4	1.3	1.0	0.5	0.6	1.0	1.3	1.3	1.2	0.9	0.5
Total Chlorine Residual	µg/L	2														
Ammonia (as N) - 6-mo median	μg/L	600	29	341	523	600	614	461	255	26	301	575	585	546	409	243
Ammonia (as N) - Daily Max	μg/L	2,400	32	388	597	684	701	526	291	28	342	656	668	623	467	277
Acute Toxicity a	TUa	0.3														
Chronic Toxicity a	TUc	1														
Phenolic Compounds (non-chlorinated)	μg/L	30	5.6	5.0	4.4	3.7	2.9	2.0	0.8	5.6	5.0	3.6	3.3	2.6	1.8	0.9
Chlorinated Phenolics <sup>b</sup>	μg/L	1	<2.2	<1.9	<1.7	<1.4	<1.0	<0.7	<0.3	<2.2	<2.0	<1.3	<1.2	<1.0	<0.6	<0.3
Endosulfan	μg/L	0.009	7E-06	1E-04	2E-04	2E-04	2E-04	2E-04	9E-05	6E-06	1E-04	2E-04	2E-04	2E-04	1E-04	8E-05
Endrin	μg/L	0.002	2E-07	1E-06	1E-06	2E-06	2E-06	1E-06	7E-07	1E-07	8E-07	2E-06	2E-06	1E-06	1E-06	6E-07
HCH (Hexachlorocyclohexane)	μg/L	0.004	2E-05	3E-04	4E-04	5E-04	5E-04	4E-04	2E-04	2E-05	2E-04	5E-04	5E-04	5E-04	3E-04	2E-04
Radioactivity (Gross Beta) a	pCi/L	0.0														
Radioactivity (Gross Alpha) a	pCi/L	0.0														
Objectives for protection of human	health – no															
Acrolein	μg/L	220	<0.2	<0.2	<0.2	<0.2	<0.1	<0.1	<0.04	<0.2	<0.2	<0.2	<0.2	<0.1	<0.1	<0.05
Antimony	μg/L	1200	0.01	0.02	0.02	0.02	0.01	0.01	0.005	0.01	0.02	0.01	0.01	0.01	0.01	0.005
Bis (2-chloroethoxy) methane	μg/L	4.4	<1.1	<0.9	<0.7	<0.5	<0.4	<0.3	<0.1	<1.1	<0.9	<0.5	<0.5	<0.3	<0.2	<0.1
Bis (2-chloroisopropyl) ether	μg/L	1200	<1.1	<0.9	<0.7	<0.5	<0.4	<0.3	<0.1	<1.1	<0.9	<0.5	<0.5	<0.3	<0.2	<0.1
Chlorobenzene	μg/L	570	<0.06	<0.05	<0.04	<0.03	<0.03	<0.02	<0.01	<0.06	<0.05	< 0.03	<0.03	<0.02	<0.02	<0.01
Chromium (III)	μg/L	190000	1.2	0.9	0.8	0.6	0.4	0.3	0.1	1.1	1.0	0.6	0.5	0.4	0.3	0.1
Di-n-butyl phthalate	μg/L	3500	<1.1	<0.9	<0.7	<0.6	<0.4	<0.3	<0.1	<1.1	<0.9	<0.6	<0.5	<0.4	<0.3	<0.1
Dichlorobenzenes	μg/L	5100	0.1	0.1	0.1	0.05	0.04	0.03	0.01	0.1	0.1	0.05	0.05	0.04	0.03	0.01
Diethyl phthalate	μg/L	33000	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.03	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.03
Dimethyl phthalate	μg/L	820000	<0.1	<0.1	<0.1	<0.1	<0.05	<0.03	<0.02	<0.1	<0.1	<0.1	<0.1	<0.04	<0.03	<0.02
4,6-dinitro-2-methylphenol	μg/L	220	<5.4	<4.4	<3.5	<2.7	<1.9	<1.3	<0.4	<5.4	<4.5	<2.6	<2.3	<1.7	<1.1	<0.5

# MPWSP AND VARIANT OCEAN PLAN COMPLIANCE

							Estimate	ed Concen	tration at E	dge of ZID	by Flow S	Scenario				
Constituent	Units	Ocean Plan				MPWSP					MP	WSP with	High Desa	I Brine Flo	ws	
		Objective	1	2	3	4	5	6	7	8	9	10	11	12	13	14
2,4-Dinitrophenol b	μg/L	4.0	<5.6	<4.5	<3.6	<2.7	<1.9	<1.3	<0.4	<5.5	<4.6	<2.6	<2.4	<1.8	<1.1	<0.5
Ethylbenzene	μg/L	4100	<0.06	< 0.05	<0.04	< 0.03	< 0.03	<0.02	<0.01	<0.06	<0.05	< 0.03	< 0.03	<0.02	< 0.02	<0.01
Fluoranthene	μg/L	15	0.01	0.01	0.01	0.01	0.004	0.003	0.001	0.01	0.01	0.01	0.005	0.004	0.002	0.001
Hexachlorocyclopentadiene	μg/L	58	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.003	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.003
Nitrobenzene	μg/L	4.9	<2.7	<2.1	<1.7	<1.3	<0.9	<0.6	<0.2	<2.7	<2.2	<1.3	<1.1	<0.9	<0.5	<0.2
Thallium	μg/L	2	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.003	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.003
Toluene	μg/L	85000	0.06	0.05	0.04	0.03	0.03	0.02	0.01	0.06	0.05	0.03	0.03	0.02	0.02	0.01
Tributyltin <sup>b</sup>	μg/L	0.0014	<0.005	<0.005	< 0.004	<0.003	<0.003	<0.002	<0.001	<0.005	<0.005	<0.003	<0.003	<0.002	<0.002	<0.001
1,1,1-Trichloroethane	μg/L	540000	<0.06	<0.05	<0.04	< 0.03	< 0.03	<0.02	<0.01	<0.06	<0.05	< 0.03	< 0.03	<0.02	<0.02	<0.01
Objectives for protection of human	health - ca	rcinogens - 30	0-day aver	age limit <sup>c</sup>	d											
Acrylonitrile cd	μg/L	0.10														
Aldrin b	μg/L	0.000022	<7E-06	<4E-05	<6E-05	<7E-05	<7E-05	<5E-05	<3E-05	<6E-06	<4E-05	<7E-05	<7E-05	<6E-05	<5E-05	<3E-05
Benzene	μg/L	5.9	<0.06	< 0.05	<0.04	< 0.03	< 0.03	<0.02	<0.01	<0.06	<0.05	< 0.03	< 0.03	<0.02	<0.02	<0.01
Benzidine b	μg/L	0.000069	<5.6	<4.5	<3.6	<2.7	<1.9	<1.3	<0.4	<5.5	<4.6	<2.6	<2.4	<1.8	<1.1	<0.5
Beryllium <sup>d</sup>	μg/L	0.033	2E-06	2E-06	2E-06	1E-06	8E-07	6E-07	2E-07	2E-06	2E-06	1E-06	9E-07	7E-07	4E-07	2E-07
Bis(2-chloroethyl)ether b	μg/L	0.045	<2.7	<2.1	<1.7	<1.3	<0.9	<0.6	<0.2	<2.7	<2.2	<1.3	<1.1	<0.9	<0.5	<0.2
Bis(2-ethyl-hexyl)phthalate	μg/L	3.5	0.1	0.7	1.0	1.1	1.1	0.8	0.5	0.1	0.6	1.1	1.1	1.0	0.8	0.4
Carbon tetrachloride	μg/L	0.90	<0.06	< 0.05	<0.04	< 0.03	< 0.03	<0.02	<0.01	<0.06	<0.05	< 0.03	< 0.03	<0.02	<0.02	<0.01
Chlordane	μg/L	0.000023	1E-06	1E-05	2E-05	2E-05	2E-05	1E-05	7E-06	1E-06	9E-06	2E-05	2E-05	2E-05	1E-05	7E-06
Chlorodibromomethane	μg/L	8.6	<0.1	< 0.05	<0.04	< 0.03	< 0.03	<0.02	<0.01	<0.1	<0.05	< 0.03	< 0.03	<0.02	<0.02	<0.01
Chloroform	μg/L	130	0.1	0.1	0.1	0.1	0.05	0.03	0.02	0.1	0.1	0.1	0.1	0.04	0.03	0.02
DDT	μg/L	0.00017	6E-07	8E-06	1E-05	1E-05	1E-05	1E-05	6E-06	5E-07	7E-06	1E-05	1E-05	1E-05	1E-05	6E-06
1,4-Dichlorobenzene	μg/L	18	0.1	0.1	0.1	0.05	0.04	0.03	0.01	0.1	0.1	0.05	0.05	0.04	0.03	0.01
3,3-Dichlorobenzidine <sup>b</sup>	μg/L	0.0081	<5.6	<4.5	<3.5	<2.7	<1.9	<1.3	<0.4	<5.5	<4.6	<2.6	<2.4	<1.8	<1.1	<0.5
1,2-Dichloroethane	μg/L	28	<0.06	< 0.05	<0.04	< 0.03	< 0.03	< 0.02	<0.01	<0.06	< 0.05	< 0.03	< 0.03	<0.02	< 0.02	<0.01
1,1-Dichloroethylene	μg/L	0.9	0.06	0.05	0.04	0.03	0.03	0.02	0.01	0.06	0.05	0.03	0.03	0.02	0.02	0.01
Dichlorobromomethane	μg/L	6.2	<0.1	<0.05	<0.04	< 0.03	<0.03	<0.02	<0.01	<0.1	<0.05	<0.03	< 0.03	<0.02	<0.02	<0.01
Dichloromethane	μg/L	450	0.06	0.05	0.05	0.04	0.03	0.02	0.01	0.06	0.05	0.04	0.04	0.03	0.02	0.01
1,3-dichloropropene	μg/L	8.9	<0.06	<0.05	<0.04	< 0.03	<0.03	<0.02	<0.01	<0.06	<0.05	<0.03	<0.03	<0.02	<0.02	<0.01
Dieldrin	μg/L	0.00004	3E-06	8E-06	1E-05	1E-05	1E-05	8E-06	4E-06	3E-06	7E-06	1E-05	1E-05	9E-06	7E-06	4E-06
2,4-Dinitrotoluene	μg/L	2.6	<0.01	<0.02	< 0.03	< 0.03	<0.03	<0.02	<0.01	<0.01	<0.02	<0.03	< 0.03	< 0.03	<0.02	<0.01
1,2-Diphenylhydrazine b	μg/L	0.16	<1.1	<0.9	<0.7	<0.5	<0.4	<0.3	<0.1	<1.1	<0.9	<0.5	<0.5	< 0.3	<0.2	<0.1
Halomethanes	μg/L	130	0.1	0.05	0.04	0.03	0.03	0.02	0.01	0.1	0.05	0.03	0.03	0.02	0.02	0.01
Heptachlor <sup>b</sup>	μg/L	0.00005	<5E-06	<8E-05	<1E-04	<1E-04	<1E-04	<1E-04	<6E-05	<4E-06	<7E-05	<1E-04	<1E-04	<1E-04	<9E-05	<6E-05
Heptachlor Epoxide	μg/L	0.00002	1E-07	8E-07	1E-06	1E-06	1E-06	1E-06	5E-07	1E-07	7E-07	1E-06	1E-06	1E-06	9E-07	5E-07
Hexachlorobenzene	μg/L	0.00021	4E-06	4E-06	4E-06	3E-06	3E-06	2E-06	7E-07	4E-06	4E-06	3E-06	3E-06	2E-06	2E-06	8E-07
Hexachlorobutadiene	μg/L	14	3E-08	9E-08	1E-07	1E-07	1E-07	1E-07	5E-08	3E-08	8E-08	1E-07	1E-07	1E-07	9E-08	5E-08
Hexachloroethane	μg/L	2.5	<1.1	<0.9	<0.7	<0.5	<0.4	<0.3	<0.1	<1.1	<0.9	<0.5	<0.5	<0.3	<0.2	<0.1
Isophorone	μg/L	730	<0.06	<0.05	<0.04	<0.03	<0.03	<0.02	<0.01	<0.06	<0.05	<0.03	< 0.03	<0.02	<0.02	<0.01
N-Nitrosodimethylamine	μg/L	7.3	0.0002	0.0003	0.0003	0.0003	0.0003	0.0002	0.0001	0.0002	0.0003	0.0003	0.0003	0.0003	0.0002	0.0001
N-Nitrosodi-N-Propylamine	μg/L	0.38	0.0003	0.001	0.001	0.001	0.001	0.001	0.0005	0.0003	0.001	0.001	0.001	0.001	0.001	0.0004



							Estimate	ed Concen	tration at E	Edge of ZIE	by Flow S	Scenario				
Constituent	Units	Ocean Plan Objective				MPWSP					MP	WSP with	High Desa	Brine Flo	ws	
			1	2	3	4	5	6	7	8	9	10	11	12	13	14
N-Nitrosodiphenylamine	μg/L	2.5	<1.1	<0.9	<0.7	<0.5	<0.4	<0.3	<0.1	<1.1	<0.9	<0.5	<0.5	<0.3	<0.2	<0.1
PAHs	μg/L	0.0088	0.0002	0.0004	0.0005	0.0006	0.0005	0.0004	0.0002	0.0002	0.0004	0.0005	0.0005	0.0005	0.0004	0.0002
PCBs	μg/L	0.000019	9E-06	1E-05	1E-05	1E-05	1E-05	9E-06	5E-06	9E-06	1E-05	1E-05	1E-05	1E-05	8E-06	5E-06
TCDD Equivalents d	μg/L	3.9E-09	6E-11	1E-09	2E-09	2E-09	2E-09	1E-09	8E-10	5E-11	9E-10	2E-09	2E-09	2E-09	1E-09	8E-10
1,1,2,2-Tetrachloroethane	μg/L	2.3	<0.06	< 0.05	<0.04	< 0.03	< 0.03	<0.02	<0.01	<0.06	<0.05	< 0.03	< 0.03	<0.02	<0.02	<0.01
Tetrachloroethylene	μg/L	2.0	<0.1	< 0.05	<0.04	< 0.03	< 0.03	<0.02	<0.01	<0.1	<0.05	< 0.03	< 0.03	<0.02	<0.02	<0.01
Toxaphene e	μg/L	2.1E-04	6E-06	6E-05	9E-05	1E-04	1E-04	8E-05	4E-05	5E-06	5E-05	1E-04	1E-04	9E-05	7E-05	4E-05
Trichloroethylene	μg/L	27	<0.1	<0.05	<0.04	< 0.03	< 0.03	<0.02	<0.01	<0.1	<0.05	< 0.03	< 0.03	<0.02	<0.02	<0.01
1,1,2-Trichloroethane	μg/L	9.4	<0.1	<0.05	<0.04	< 0.03	< 0.03	<0.02	<0.01	<0.1	<0.05	< 0.03	< 0.03	<0.02	<0.02	<0.01
2,4,6-Trichlorophenol <sup>b</sup>	μg/L	0.29	<1.1	<0.9	<0.7	<0.5	<0.4	< 0.3	<0.1	<1.1	<0.9	<0.5	<0.5	<0.3	<0.2	<0.1
Vinyl chloride	μg/L	36	< 0.03	< 0.03	< 0.03	< 0.02	< 0.02	<0.01	<0.01	< 0.03	<0.03	<0.02	<0.02	<0.02	<0.01	<0.01

- a: Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based the nature of the constituent.
- b: All observed values from some data sources were below the MRL, and the flow-weighted average of the MRLs is higher than the Ocean Plan objective. No compliance conclusions can be drawn for these constituents.
- c: Acrylonitrile was only detected in one potential source water for the Variant Project. It was not detected in any potential source waters for the MPWSP Project; therefore, a compliance determination cannot be made for the MPWSP Project and only partial determination can be made for the Variant Project.
- d: Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time.
- e: Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.

Table A2 – Complete list of estimated concentrations at the edge of the ZID expressed as a percentage of Ocean Plan<sup>a</sup>

		piete iist oi									e of ZID by		nario			
Constituent	Units	Ocean Plan				MPWSP					MP'	WSP with	High Desa	I Brine Flo	ws	
		Objective	1	2	3	4	5	6	7	8	9	10	11	12	13	14
Objectives for protection of marine a	aquatic life	e - 6-month me	dian limit													
Arsenic	μg/L	8	49%	51%	51%	50%	49%	46%	41%	49%	51%	50%	49%	48%	45%	41%
Cadmium	μg/L	1	32%	27%	22%	17%	12%	8%	3%	32%	27%	17%	15%	11%	7%	3%
Chromium (Hexavalent)	μg/L	2	3%	3%	3%	3%	2%	2%	1%	3%	3%	3%	2%	2%	1%	1%
Copper	μg/L	3	64%	67%	69%	69%	70%	69%	68%	64%	66%	69%	70%	70%	69%	68%
Lead	μg/L	2	2%	1%	1%	1%	1%	0.4%	0.1%	2%	1%	1%	1%	1%	0.4%	0.2%
Mercury	μg/L	0.04	68%	55%	44%	35%	25%	17%	6%	68%	56%	33%	30%	23%	15%	7%
Nickel	μg/L	5	14%	12%	10%	8%	6%	4%	2%	14%	12%	8%	7%	6%	4%	2%
Selenium	μg/L	15	4%	3%	3%	2%	2%	1%	0.4%	4%	3%	2%	2%	2%	1%	0.5%
Silver	μg/L	0.7	26%	25%	25%	24%	24%	24%	23%	26%	25%	24%	24%	24%	23%	23%
Zinc	μg/L	20	41%	41%	41%	41%	41%	41%	40%	41%	41%	41%	41%	41%	41%	40%
Cyanide	μg/L	1	59%	108%	133%	140%	134%	99%	52%	58%	101%	134%	133%	120%	88%	51%
Total Chlorine Residual	μg/L	2														
Ammonia (as N) - 6-mo median	μg/L	600	5%	57%	87%	100%	102%	77%	43%	4%	50%	96%	97%	91%	68%	40%
Ammonia (as N) - Daily Max	μg/L	2,400	1%	16%	25%	29%	29%	22%	12%	1%	14%	27%	28%	26%	19%	12%
Acute Toxicity a	TUa	0.3														
Chronic Toxicity a	TUc	1														
Phenolic Compounds (non-chlorinated)	μg/L	30	19%	17%	15%	12%	10%	7%	3%	19%	17%	12%	11%	9%	6%	3%
Chlorinated Phenolics b	μg/L	1						<72%	<26%						<63%	<31%
Endosulfan	μg/L	0.009	0.1%	1%	2%	2%	2%	2%	1%	0.1%	1%	2%	2%	2%	2%	1%
Endrin	μg/L	0.002	0.01%	0.05%	0.1%	0.1%	0.1%	0.1%	0.03%	0.01%	0.04%	0.08%	0.08%	0.07%	0.05%	0.03%
HCH (Hexachlorocyclohexane)	μg/L	0.004	0.5%	7%	11%	13%	13%	10%	5%	0.4%	6%	12%	12%	11%	9%	5%
Radioactivity (Gross Beta) a	pCi/L	0.0														
Radioactivity (Gross Alpha) a	pCi/L	0.0														
Objectives for protection of human I	nealth – no	on carcinogens	- 30-day	average lin	nit											
Acrolein	μg/L	220	<0.1%	<0.1%	<0.1%	<0.1%	<0.1%	<0.05%	<0.02%	<0.1%	<0.1%	<0.1%	<0.1%	<0.1%	<0.04%	<0.02%
Antimony	μg/L	1200	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Bis (2-chloroethoxy) methane	μg/L	4.4	<25%	<20%	<16%	<12%	<8%	<6%	<2%	<24%	<20%	<12%	<11%	<8%	<5%	<2%
Bis (2-chloroisopropyl) ether	μg/L	1200	<0.1%	<0.1%	<0.1%	<0.04%	<0.03%	<0.02%	<0.01%	<0.1%	<0.1%	<0.04%	<0.04%	<0.03%	<0.02%	<0.01%
Chlorobenzene	μg/L	570	0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Chromium (III)	μg/L	190000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Di-n-butyl phthalate	μg/L	3500	<0.03%	<0.03%	<0.02%	<0.02%	<0.01%	<0.01%	<0.01%	<0.03%	<0.03%	<0.02%	<0.01%	<0.01%	<0.01%	<0.01%
Dichlorobenzenes	μg/L	5100	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Diethyl phthalate	μg/L	33000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Dimethyl phthalate	μg/L	820000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
4,6-dinitro-2-methylphenol	μg/L	220	<2%	<2%	<2%	<1%	<1%	<1%	<0.2%	<2%	<2%	<1%	<1%	<1%	<0.5%	<0.2%
2,4-Dinitrophenol b	μg/L	4.0		-		<69%	<47%	<32%	<9%			<66%	<59%	<44%	<28%	<12%
Ethylbenzene	μg/L	4100	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Fluoranthene	μg/L	15	0.1%	0.1%	0.05%	0.04%	0.03%	0.02%	0.01%	0.1%	0.1%	0.04%	0.03%	0.02%	0.02%	0.01%

# MPWSP AND VARIANT OCEAN PLAN COMPLIANCE

		Occar Diam				Est. F	ercentage	of Ocean	Plan objec	tive at Edg	je of ZID b	y Flow Sce	nario			
Constituent	Units	Ocean Plan Objective				MPWSP					MP	WSP with	High Desa	l Brine Flo	ws	
		Objective	1	2	3	4	5	6	7	8	9	10	11	12	13	14
Hexachlorocyclopentadiene	µg/L	58	<0.01%	<0.01%	<0.02%	<0.02%	<0.02%	<0.01%	<0.01%	<0.01%	<0.01%	<0.02%	<0.02%	<0.01%	<0.01%	<0.0
Nitrobenzene	μg/L	4.9	<54%	<44%	<35%	<27%	<19%	<13%	<4%	<54%	<45%	<26%	<23%	<17%	<11%	<5
Гhallium	μg/L	2	<0.3%	<0.4%	<0.5%	<0.5%	<0.5%	<0.3%	<0.2%	<0.3%	<0.4%	<0.5%	<0.5%	<0.4%	<0.3%	<0.
Toluene	μg/L	85000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.0
TributyItin <sup>b</sup>	μg/L	0.0014						-	<46%							<5
1,1,1-Trichloroethane	μg/L	540000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.0
Objectives for protection of human	health - ca	rcinogens - 30	O-day aver	age limit <sup>c</sup>	d											
Acrylonitrile <sup>c d</sup>	μg/L	0.10													-	-
Aldrin b	μg/L	0.000022	<30%			-		-	-	<28%			-			-
Benzene	µg/L	5.9	<1%	<1%	<1%	<1%	<0.4%	<0.3%	<0.1%	<1%	<1%	<1%	<1%	<0.4%	<0.3%	<0.
Benzidine b	µg/L	0.000069														-
Beryllium d	µg/L	0.033														-
Bis(2-chloroethyl)ether b	μg/L	0.045						-								-
Bis(2-ethyl-hexyl)phthalate	µg/L	3.5	3%	19%	28%	32%	32%	24%	13%	3%	17%	31%	31%	29%	22%	13
Carbon tetrachloride	μg/L	0.90	<6%	<5%	<5%	<4%	<3%	<2%	<1%	<6%	<5%	<4%	<3%	<3%	<2%	<1
Chlordane	μg/L	0.000023	6%	44%	66%	75%	77%	57%	32%	6%	39%	72%	73%	68%	51%	30
Chlorodibromomethane	µg/L	8.6	<1%	<1%	<0.5%	<0.4%	<0.3%	<0.2%	<0.1%	<1%	<1%	<0.4%	<0.4%	<0.3%	<0.2%	<0.
Chloroform	µg/L	130	0.04%	0.05%	0.05%	0.04%	0.04%	0.03%	0.01%	0.04%	0.05%	0.04%	0.04%	0.03%	0.02%	0.0
ODT	µg/L	0.00017	0.3%	5%	7%	8%	8%	6%	3%	0.3%	4%	8%	8%	7%	6%	39
1,4-Dichlorobenzene	μg/L	18	0.3%	0.3%	0.3%	0.3%	0.2%	0.2%	0.1%	0.3%	0.3%	0.3%	0.3%	0.2%	0.1%	0.1
3,3-Dichlorobenzidine b	μg/L	0.0081						-								-
1,2-Dichloroethane	µg/L	28	<0.2%	<0.2%	<0.1%	<0.1%	<0.1%	<0.1%	<0.02%	<0.2%	<0.2%	<0.1%	<0.1%	<0.1%	<0.1%	<0.0
1,1-Dichloroethylene	μg/L	0.9	6%	5%	5%	4%	3%	2%	1%	6%	5%	4%	3%	3%	2%	1
Dichlorobromomethane	μg/L	6.2	<1%	<1%	<1%	<1%	<0.4%	<0.3%	<0.1%	<1%	<1%	<1%	<0.5%	<0.4%	<0.3%	<0.
Dichloromethane	μg/L	450	0.01%	0.01%	0.01%	0.01%	0.01%	<0.01%	<0.01%	0.01%	0.01%	0.01%	0.01%	0.01%	<0.01%	<0.0
1,3-dichloropropene	μg/L	8.9	<1%	<1%	<0.5%	<0.4%	<0.3%	<0.2%	<0.1%	<1%	<1%	<0.4%	<0.3%	<0.3%	<0.2%	<0.
Dieldrin	μg/L	0.00004	8%	19%	25%	27%	27%	20%	10%	8%	18%	26%	26%	24%	17%	10
2,4-Dinitrotoluene	μg/L	2.6	<0.5%	<1%	<1%	<1%	<1%	<1%	<0.5%	<0.5%	<1%	<1%	<1%	<1%	<1%	<0.
1,2-Diphenylhydrazine b	µg/L	0.16						-	<45%							<62
Halomethanes	μg/L	130	0.04%	0.04%	0.03%	0.03%	0.02%	0.01%	0.01%	0.04%	0.04%	0.03%	0.02%	0.02%	0.01%	0.0
Heptachlor <sup>b</sup>	μg/L	0.00005	<9%					-		<8%						-
Heptachlor Epoxide	μg/L	0.00002	1%	4%	6%	6%	6%	5%	3%	1%	3%	6%	6%	6%	4%	3
Hexachlorobenzene	μg/L	0.00021	2%	2%	2%	2%	1%	1%	0.3%	2%	2%	1%	1%	1%	1%	0.4
Hexachlorobutadiene	µg/L	14	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.0
Hexachloroethane	µg/L	2.5	<43%	<35%	<28%	<22%	<15%	<10%	<3%	<43%	<36%	<21%	<19%	<14%	<9%	<4
sophorone	μg/L	730	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.0
N-Nitrosodimethylamine	μg/L	7.3	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.0
N-Nitrosodi-N-Propylamine	µg/L	0.38	0.1%	0.2%	0.3%	0.3%	0.3%	0.2%	0.1%	0.1%	0.2%	0.3%	0.3%	0.3%	0.2%	0.1
N-Nitrosodiphenylamine	µg/L	2.5	<43%	<35%	<28%	<22%	<15%	<10%	<3%	<43%	<36%	<21%	<19%	<14%	<9%	<4
PAHs	µg/L	0.0088	2%	4%	6%	6%	6%	5%	2%	2%	4%	6%	6%	6%	4%	2'
PCBs	µg/L	0.000019	47%	64%	72%	72%	66%	49%	24%	46%	61%	69%	67%	60%	43%	24



						Est. P	ercentage	of Ocean	Plan objec	tive at Edg	ge of ZID by	y Flow Sce	nario			
Constituent	Units	Ocean Plan Objective				MPWSP					MP	WSP with	High Desa	I Brine Flo	ws	
		Objective	1	2	3	4	5	6	7	8	9	10	11	12	13	14
TCDD Equivalents d	μg/L	3.9E-09	2%	27%	42%	49%	50%	38%	21%	1%	24%	47%	48%	44%	33%	20%
1,1,2,2-Tetrachloroethane	μg/L	2.3	<2%	<2%	<2%	<1%	<1%	<1%	<0.3%	<2%	<2%	<1%	<1%	<1%	<1%	<0.3%
Tetrachloroethylene	μg/L	2.0	<3%	<2%	<2%	<2%	<1%	<1%	<0.3%	<3%	<2%	<2%	<2%	<1%	<1%	<0.4%
Toxaphene e	μg/L	2.1E-04	3%	27%	42%	47%	48%	36%	20%	3%	24%	45%	46%	43%	32%	19%
Trichloroethylene	μg/L	27	<0.2%	<0.2%	<0.2%	<0.1%	<0.1%	<0.1%	<0.02%	<0.2%	<0.2%	<0.1%	<0.1%	<0.1%	<0.1%	<0.03%
1,1,2-Trichloroethane	μg/L	9.4	<1%	<1%	<0.4%	<0.4%	<0.3%	<0.2%	<0.1%	<1%	<1%	<0.4%	<0.3%	<0.3%	<0.2%	<0.1%
2,4,6-Trichlorophenol b	μg/L	0.29	-						<25%	-					<75%	<34%
Vinyl chloride	μg/L	36	<0.1%	<0.1%	<0.1%	<0.1%	<0.1%	<0.04%	<0.01%	<0.1%	<0.1%	<0.1%	<0.1%	<0.05%	<0.03%	<0.02%

- a: Note that if the percentage was determined to be less than 0.01 percent, then a minimum value is shown as "<0.01%" (e.g., if the constituent was estimated to be 0.000001% of the objective, for simplicity, it is displayed as <0.01%). Also, shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.
- b: Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based the nature of the constituent. These constituents were measured individually for the secondary effluent and GWR concentrate, and these individual concentrations would comply with the Ocean Plan objectives.
- c: All observed values from all data sources were below the MRL, and the flow-weighted average of the MRLs is higher than the Ocean Plan objective. No compliance conclusions can be drawn for these constituents.
- d: Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time.
- e: Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.



Table A3 – Complete list of estimated concentrations of Ocean Plan constituents at the edge of the ZID for the Variant

											Estim	ated Co	ncentra	tion at E	dge of Z	ID by Fl	ow Scei	nario								
Constituent	Units	Ocean Plan		Varia	ant with	GWR Of	ffline			Varia	nt with I	Normal I	Flows		Vari	ant with	, , , , , , , , , , , , , , , , , , ,	esal Brin Offline	e Flows	and	Va	riant wit	h High [	Desal Br	ine Flow	/S
		Objective	15	16	17	18	19	20	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47
Objectives for prote	ection of n	narine aquati	c life -	6-month	median	limit																				
Arsenic	μg/L	8	3.9	4.1	4.1	3.9	3.3	3.3	3.8	4.0	3.9	3.4	3.3	3.3	3.9	4.1	4.1	3.3	3.3	3.3	3.9	3.9	4.0	3.3	3.3	3.3
Cadmium	μg/L	1	0.3	0.2	0.2	0.1	0.02	0.02	0.3	0.2	0.2	0.1	0.03	0.02	0.3	0.2	0.2	0.04	0.03	0.03	0.3	0.3	0.2	0.04	0.03	0.03
Chromium (Hexavalent)	μg/L	2	0.09	0.09	0.09	0.06	0.02	0.02	0.17	0.15	0.11	0.04	0.02	0.02	0.08	0.08	0.07	0.02	0.02	0.02	0.14	0.14	0.11	0.03	0.02	0.02
Copper	μg/L	3	1.9	2.0	2.1	2.1	2.0	2.0	2.3	2.3	2.3	2.1	2.1	2.1	1.9	2.1	2.1	2.0	2.0	2.0	2.3	2.3	2.2	2.1	2.1	2.1
Lead	μg/L	2	0.03	0.05	0.06	0.05	0.02	0.02	0.13	0.12	0.09	0.03	0.02	0.02	0.03	0.05	0.06	0.02	0.02	0.02	0.11	0.11	0.09	0.02	0.02	0.02
Mercury	μg/L	0.04	0.03	0.02	0.02	0.01	0.002	0.002	0.03	0.02	0.01	0.005	0.003	0.002	0.03	0.02	0.01	0.003	0.003	0.003	0.03	0.02	0.02	0.004	0.003	0.003
Nickel	μg/L	5	0.7	0.6	0.5	0.4	0.1	0.1	1.0	0.9	0.6	0.2	0.1	0.1	0.7	0.6	0.5	0.1	0.1	0.1	1.0	0.9	0.7	0.2	0.1	0.1
Selenium	μg/L	15	0.6	0.5	0.4	0.3	0.1	0.1	0.7	0.6	0.4	0.1	0.1	0.1	0.6	0.5	0.4	0.1	0.1	0.1	0.7	0.6	0.5	0.1	0.1	0.1
Silver	μg/L	0.7	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
Zinc	μg/L	20	8.2	8.7	8.8	8.7	8.3	8.3	10.2	10.1	9.6	8.6	8.4	8.4	8.2	8.7	8.8	8.3	8.3	8.3	9.9	9.9	9.6	8.4	8.4	8.4
Cyanide	μg/L	1	0.6	1.4	1.6	1.4	0.5	0.5	1.5	1.9	1.7	0.7	0.5	0.6	0.6	1.4	1.6	0.5	0.5	0.5	1.3	1.6	1.8	0.6	0.6	0.6
Total Chlorine Residual	μg/L	2			1	1	1	-		-	1		-				1	-	-		-		-		1	
Ammonia (as N) - 6-mo median	μg/L	600	39	474	648	581	239	251	1593	1551	1248	473	326	316	34	519	627	212	235	246	1333	1363	1227	335	327	320
Ammonia (as N) - Daily Max	μg/L	2,400	43	540	739	663	273	286	1819	1771	1425	540	372	361	37	591	716	242	268	281	1521	1555	1401	383	373	365
Acute Toxicity <sup>a</sup>	TUa	0.3		-	ı	-	-	ı	-	ı	ı	-	-			-	ı	ı	ı		ı	-	1	-	1	
Chronic Toxicity <sup>a</sup>	TUc	1			-	-		-		-	-		-				-	-	-		-		-		-	
Phenolic Compounds (non- chlorinated)	μg/L	30	5.5	4.8	3.9	2.7	0.7	0.6	7.1	5.9	4.2	1.5	0.9	0.8	5.6	4.6	3.8	0.9	0.8	0.7	6.9	6.4	4.7	1.0	0.9	0.8
Chlorinated Phenolics <sup>b</sup>	μg/L	1	<2.2	<1.8	<1.4	<1.0	<0.2	<0.2	<2.0	<1.6	<1.2	<0.4	<0.2	<0.2	<2.2	<1.7	<1.4	<0.3	<0.3	<0.3	<2.0	<1.9	<1.4	<0.3	<0.3	<0.2
Endosulfan	μg/L	0.009	3E-05	5E-04	7E-04	6E-04	3E-04	3E-04	2E-03	2E-03	1E-03	5E-04	3E-04	3E-04	3E-05	5E-04	7E-04	2E-04	3E-04	3E-04	1E-03	1E-03	1E-03	4E-04	3E-04	3E-04
Endrin	μg/L	0.002	2E-07	1E-06	2E-06	2E-06	6E-07	7E-07	4E-06	4E-06	3E-06	1E-06	9E-07	8E-07	2E-07	1E-06	2E-06	6E-07	6E-07	6E-07	4E-06	4E-06	3E-06	9E-07	9E-07	8E-07
HCH (Hexachlorocyclohexane)	μg/L	0.004	4E-05	6E-04	9E-04	8E-04	3E-04	3E-04	2E-03	2E-03	2E-03	7E-04	5E-04	4E-04	4E-05	7E-04	9E-04	3E-04	3E-04	3E-04	2E-03	2E-03	2E-03	5E-04	5E-04	4E-04
Radioactivity (Gross Beta) <sup>a</sup>	pci/L	0.0			1	1	1	-		-	1		-				1	-	-		-		-		1	
Radioactivity (Gross Alpha) <sup>a</sup>	pci/L	0.0																								

# MPWSP AND VARIANT OCEAN PLAN COMPLIANCE

											Estim	ated Co	ncentra	tion at E	dge of Z	ZID by FI	ow Sce	nario								
Constituent	Units	Ocean Plan		Varia	ant with	GWR O	ffline			Varia	nt with I	Normal I	Flows		Vari	ant with		esal Brin Offline	e Flows	and	Va	riant wit	th High [	Desal Bı	rine Flow	vs
		Objective	15	16	17	18	19	20	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47
Objectives for prote	ction of h		– non c	arcinoge	ens – 30	-day ave	erage lin	nit																		
Acrolein	μg/L	220	0.2	0.3	0.2	0.2	0.1	0.1	0.5	0.4	0.3	0.1	0.1	0.1	0.2	0.2	0.2	0.1	0.1	0.1	0.4	0.4	0.3	0.1	0.1	0.1
Antimony	μg/L	1200	0.01	0.02	0.02	0.01	0.01	0.01	0.04	0.04	0.03	0.01	0.01	0.01	0.01	0.02	0.02	0.01	0.01	0.01	0.04	0.03	0.03	0.01	0.01	0.01
Bis (2- chloroethoxy) methane	μg/L	4.4	<1.1	<0.8	<0.6	<0.4	<0.1	<0.1	<0.9	<0.7	<0.5	<0.2	<0.1	<0.1	<1.1	<0.8	<0.6	<0.1	<0.1	<0.1	<0.9	<0.8	<0.6	<0.1	<0.1	<0.1
Bis (2- chloroisopropyl) ether	μg/L	1200	<1.1	<0.8	<0.6	<0.4	<0.1	<0.1	<0.9	<0.7	<0.5	<0.2	<0.1	<0.1	<1.1	<0.8	<0.6	<0.1	<0.1	<0.1	<0.9	<0.8	<0.6	<0.1	<0.1	<0.1
Chlorobenzene	μg/L	570	< 0.05	<0.05	<0.04	<0.02	<0.01	<0.01	<0.05	<0.04	< 0.03	<0.01	<0.01	<0.01	<0.06	<0.04	< 0.03	<0.01	<0.01	<0.01	<0.05	< 0.05	<0.03	<0.01	<0.01	<0.01
Chromium (III)	μg/L	190000	1.1	0.9	0.7	0.5	0.1	0.1	1.2	1.0	0.7	0.2	0.1	0.1	1.2	0.9	0.7	0.1	0.1	0.1	1.2	1.1	0.8	0.2	0.1	0.1
Di-n-butyl phthalate	μg/L	3500	<1.1	<0.9	<0.7	<0.4	<0.1	<0.1	<0.9	<0.7	<0.5	<0.2	<0.1	<0.1	<1.1	<0.8	<0.6	<0.1	<0.1	<0.1	<0.9	<0.9	<0.6	<0.1	<0.1	<0.1
Dichlorobenzenes	µg/L	5100	0.1	0.1	0.1	0.04	0.01	0.01	0.1	0.1	0.1	0.02	0.02	0.01	0.1	0.1	0.1	0.01	0.01	0.01	0.1	0.1	0.1	0.02	0.02	0.02
Diethyl phthalate	μg/L	33000	<0.1	<0.1	<0.1	<0.1	< 0.03	< 0.03	<0.1	<0.1	<0.1	< 0.04	< 0.03	< 0.03	<0.1	<0.1	<0.1	< 0.03	<0.03	< 0.03	<0.1	<0.1	<0.1	< 0.03	< 0.03	<0.03
Dimethyl phthalate	μg/L	820000	<0.1	<0.1	<0.1	<0.04	<0.01	<0.01	<0.1	<0.1	<0.05	<0.02	<0.01	<0.01	<0.1	<0.1	<0.1	<0.02	<0.02	<0.01	<0.1	<0.1	<0.1	<0.01	<0.01	<0.01
4,6-dinitro-2- methylphenol	µg/L	220	<5.3	<4.1	<3.1	<2.0	<0.4	<0.3	<4.5	<3.5	<2.4	<0.8	<0.4	<0.4	<5.4	<3.9	<3.0	<0.6	<0.5	<0.4	<4.7	<4.2	<2.9	<0.6	<0.5	<0.4
2,4-Dinitrophenol b	μg/L	4.0	<5.4	<4.1	<3.0	<1.9	<0.4	<0.3	<4.6	<3.5	<2.3	<0.8	<0.4	< 0.3	<5.6	<3.8	<2.9	<0.6	<0.5	<0.4	<4.8	<4.3	<2.8	<0.5	<0.5	<0.4
Ethylbenzene	µg/L	4100	< 0.05	<0.05	<0.04	<0.02	<0.01	<0.01	< 0.05	< 0.04	< 0.03	<0.01	<0.01	< 0.01	< 0.06	< 0.04	< 0.03	< 0.01	<0.01	<0.01	< 0.05	< 0.05	< 0.03	< 0.01	< 0.01	<0.01
Fluoranthene	μg/L	15	0.01	0.01	0.01	0.004	0.001	0.001	0.01	0.01	0.005	0.001	0.001	0.001	0.01	0.01	0.01	0.001	0.001	0.001	0.01	0.01	0.01	0.001	0.001	0.001
Hexachlorocyclope ntadiene	µg/L	58	<0.01	<0.01	<0.01	<0.01	<0.003	<0.003	<0.01	<0.01	<0.01	<0.003	<0.003	<0.003	<0.01	<0.01	<0.01	<0.003	<0.003	<0.003	<0.01	<0.01	<0.01	<0.003	<0.003	<0.003
Nitrobenzene	μg/L	4.9	<2.6	<2.0	<1.4	<0.9	<0.2	<0.1	<2.2	<1.6	<1.1	<0.3	<0.2	<0.1	<2.7	<1.8	<1.4	<0.3	<0.2	<0.2	<2.3	<2.0	<1.3	<0.2	<0.2	<0.2
Thallium	μg/L	2	0.01	0.01	0.01	0.01	0.004	0.004	0.03	0.03	0.02	0.01	0.01	0.01	0.01	0.01	0.01	0.004	0.004	0.004	0.03	0.03	0.02	0.01	0.01	0.01
Toluene	ua/L	85000	0.05	0.05	0.04	0.02	0.01	0.01	0.06	0.05	0.04	0.01	0.01	0.01	0.06	0.04	0.03	0.01	0.01	0.01	0.06	0.06	0.04	0.01	0.01	0.01
Tributyltin b	ua/L	0.0014	<0.005	<0.004	<0.003	<0.002	<0.001	<0.001	<0.005	<0.004	<0.003	<0.001	<0.001	<0.001	<0.005	<0.004	<0.003	<0.001	<0.001	<0.001	<0.005	<0.004	<0.003	<0.001	<0.001	<0.001
1,1,1- Trichloroethane	µg/L	540000	<0.05	<0.05	<0.04	<0.02	<0.01	<0.01	<0.05	<0.04	<0.03	<0.01	<0.01	<0.01	<0.06	<0.04	<0.03	<0.01	<0.01	<0.01	<0.05	<0.05	<0.03	<0.01	<0.01	<0.01
Objectives for prote	ction of h	uman health	- carcin	ogens	- 30-day	average	e limit 👊																			
Acrylonitrile c	μg/L	0.10	0.002	0.03	0.04	0.03	0.01	0.01	0.1	0.1	0.1	0.03	0.02	0.02	0.001	0.03	0.04	0.01	0.01	0.01	0.1	0.1	0.1	0.02	0.02	0.02
Aldrin b	μg/L	0.000022	<9E-06	<8E-05	<1E-04	<1E-04	<4E-05	<4E-05	<8E-05	<1E-04	<1E-04	<5E-05	<4E-05	<4E-05	<8E-06	<9E-05	<1E-04	<3E-05	<4E-05	<4E-05	<6E-05	<9E-05	<1E-04	<4E-05	<4E-05	<4E-05
Benzene	μg/L	5.9	< 0.05	<0.05	<0.04	<0.02	<0.01	<0.01	< 0.05	< 0.04	< 0.03	<0.01	<0.01	<0.01	<0.06	< 0.04	< 0.03	<0.01	<0.01	<0.01	<0.05	< 0.05	<0.03	<0.01	< 0.01	<0.01
Benzidine b	μg/L	0.000069	<5.4	<4.2	<3.1	<2.0	<0.4	< 0.3	<4.6	<3.6	<2.4	<0.8	<0.4	<0.4	<5.6	<4.0	<3.0	<0.6	<0.5	<0.5	<4.8	<4.3	<2.9	<0.6	<0.5	<0.4
Beryllium °	μg/L	0.033	4E-06	3E-06	2E-06	1E-06	2E-07	4E-07	3E-06	2E-06	1E-06	5E-07	2E-07	2E-07	3E-06	2E-06	1E-06	3E-07	2E-07	2E-07	3E-06	2E-06	1E-06	3E-07	2E-07	2E-07
Bis(2- chloroethyl)ether b	µg/L	0.045	<2.6	<2.0	<1.4	<0.9	<0.2	<0.1	<2.2	<1.7	<1.1	<0.4	<0.2	<0.1	<2.7	<1.8	<1.4	<0.3	<0.2	<0.2	<2.3	<2.0	<1.3	<0.3	<0.2	<0.2
Bis(2-ethyl- hexyl)phthalate	μg/L	3.5	0.1	0.9	1.2	1.1	0.4	0.5	2.9	2.9	2.3	0.9	0.6	0.6	0.1	1.0	1.2	0.4	0.4	0.5	2.5	2.5	2.3	0.6	0.6	0.6
Carbon tetrachloride	μg/L	0.90	0.05	0.05	0.04	0.02	0.01	0.01	0.06	0.05	0.04	0.01	0.01	0.01	0.06	0.04	0.03	0.01	0.01	0.01	0.06	0.06	0.04	0.01	0.01	0.01
Chlordane	μg/L	0.000023	2E-06	1E-05	2E-05	2E-05	7E-06	7E-06	5E-05	4E-05	4E-05	1E-05	9E-06	9E-06	2E-06	2E-05	2E-05	6E-06	7E-06	7E-06	4E-05	4E-05	4E-05	1E-05	9E-06	9E-06
Chlorodibromo- methane	μg/L	8.6	0.1	0.1	0.1	0.05	0.02	0.02	0.1	0.1	0.1	0.03	0.02	0.02	0.1	0.1	0.1	0.02	0.02	0.02	0.1	0.1	0.1	0.02	0.02	0.02
Chloroform	μg/L	130	0.1	0.4	0.5	0.5	0.2	0.2	1.3	1.3	1.0	0.4	0.3	0.3	0.1	0.4	0.5	0.2	0.2	0.2	1.1	1.1	1.0	0.3	0.3	0.3



											Estim	ated Co	ncentra	tion at E	dge of Z	ZID by FI	ow Scer	nario								
Constituent	Units	Ocean Plan		Varia	ant with	GWR O	ffline			Varia	nt with I	Normal I	Flows		Vari	ant with		sal Brin Offline	e Flows	and	Va	riant wit	th High [	Desal Br	ine Flow	vs
		Objective	15	16	17	18	19	20	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47
DDT	μg/L	0.00017	8E-07	1E-05	1E-05	1E-05	5E-06	6E-06	2E-06	1E-05	1E-05	6E-06	5E-06	5E-06	7E-07	1E-05	1E-05	5E-06	5E-06	6E-06	2E-06	6E-06	1E-05	5E-06	5E-06	5E-06
1,4- Dichlorobenzene	μg/L	18	0.1	0.1	0.1	0.04	0.01	0.01	0.1	0.1	0.1	0.02	0.02	0.01	0.1	0.1	0.1	0.01	0.01	0.01	0.1	0.1	0.1	0.02	0.02	0.02
3,3- Dichlorobenzidine <sup>b</sup>	μg/L	0.0081	<5.4	<4.2	<3.1	<2.0	<0.4	<0.3	<4.6	<3.6	<2.4	<0.8	<0.4	<0.4	<5.6	<4.0	<3.0	<0.6	<0.5	<0.4	<4.8	<4.3	<2.9	<0.6	<0.5	<0.4
1,2-Dichloroethane	μg/L	28	<0.05	<0.05	<0.04	< 0.02	<0.01	<0.01	< 0.05	<0.04	<0.03	<0.01	<0.01	<0.01	<0.06	< 0.04	<0.03	<0.01	<0.01	<0.01	<0.05	< 0.05	< 0.03	<0.01	<0.01	<0.01
1,1- Dichloroethylene	μg/L	0.9	0.05	0.05	0.04	0.02	0.01	0.01	0.05	0.04	0.03	0.01	0.01	0.01	0.06	0.04	0.03	0.01	0.01	0.01	0.05	0.05	0.03	0.01	0.01	0.01
Dichlorobromo- methane	μg/L	6.2	0.1	0.1	0.1	0.05	0.02	0.02	0.1	0.1	0.1	0.03	0.02	0.02	0.1	0.1	0.1	0.02	0.02	0.02	0.1	0.1	0.1	0.02	0.02	0.02
Dichloromethane	μg/L	450	0.05	0.05	0.04	0.03	0.01	0.01	0.08	0.07	0.05	0.02	0.01	0.01	0.06	0.05	0.04	0.01	0.01	0.01	0.07	0.07	0.05	0.01	0.01	0.01
1,3- dichloropropene	μg/L	8.9	0.05	0.05	0.04	0.03	0.01	0.01	0.07	0.05	0.04	0.01	0.01	0.01	0.06	0.04	0.04	0.01	0.01	0.01	0.07	0.06	0.04	0.01	0.01	0.01
Dieldrin	μg/L	0.00004	4E-06	2E-05	2E-05	2E-05	9E-06	9E-06	4E-06	2E-05	2E-05	9E-06	8E-06	8E-06	4E-06	2E-05	2E-05	8E-06	9E-06	9E-06	4E-06	1E-05	2E-05	7E-06	8E-06	8E-06
2,4-Dinitrotoluene	μg/L	2.6	<0.01	<0.03	<0.04	< 0.03	<0.01	<0.01	<0.01	<0.03	<0.03	<0.01	<0.01	<0.01	<0.01	< 0.03	<0.03	<0.01	<0.01	<0.01	<0.01	< 0.02	<0.03	<0.01	<0.01	<0.01
1,2- Diphenylhydrazine b	μg/L	0.16	<1.1	<0.8	<0.6	<0.4	<0.1	<0.1	<0.9	<0.7	<0.5	<0.2	<0.1	<0.1	<1.1	<0.8	<0.6	<0.1	<0.1	<0.1	<0.9	<0.8	<0.6	<0.1	<0.1	<0.1
Halomethanes	μg/L	130	0.1	0.1	0.05	0.04	0.01	0.01	0.1	0.1	0.1	0.02	0.01	0.01	0.1	0.1	0.05	0.01	0.01	0.01	0.1	0.1	0.1	0.02	0.01	0.01
Heptachlor b	μg/L	0.00005	<7E-06	<1E-04	<1E-04	<1E-04	<6E-05	<6E-05	<8E-05	<1E-04	<1E-04	<7E-05	<5E-05	<6E-05	<6E-06	<1E-04	<1E-04	<5E-05	<5E-05	<6E-05	<6E-05	<1E-04	<1E-04	<5E-05	<6E-05	<6E-05
Heptachlor Epoxide	μg/L	0.00002	2E-07	1E-06	1E-06	1E-06	5E-07	5E-07	3E-06	3E-06	3E-06	1E-06	7E-07	7E-07	2E-07	1E-06	1E-06	4E-07	5E-07	5E-07	3E-06	3E-06	3E-06	7E-07	7E-07	7E-07
Hexachlorobenzene	μg/L	0.00021	4E-06	4E-06	3E-06	2E-06	7E-07	6E-07	6E-06	5E-06	4E-06	1E-06	8E-07	8E-07	4E-06	4E-06	3E-06	8E-07	8E-07	7E-07	6E-06	6E-06	4E-06	1E-06	9E-07	8E-07
Hexachlorobutadiene	μg/L	14	3E-08	1E-07	1E-07	1E-07	5E-08	5E-08	4E-07	3E-07	3E-07	1E-07	7E-08	7E-08	3E-08	1E-07	1E-07	5E-08	5E-08	5E-08	3E-07	3E-07	3E-07	7E-08	7E-08	7E-08
Hexachloroethane	μg/L	2.5	<1.1	<0.8	<0.6	<0.4	<0.1	<0.1	<0.9	<0.7	<0.5	<0.1	<0.1	<0.1	<1.1	<0.7	<0.6	<0.1	<0.1	<0.1	<0.9	<0.8	<0.5	<0.1	<0.1	<0.1
Isophorone	μg/L	730	<0.05	<0.05	<0.04	<0.02	<0.01	<0.01	<0.05	<0.04	<0.03	<0.01	<0.01	<0.01	<0.06	<0.04	<0.03	<0.01	<0.01	<0.01	<0.05	<0.05	<0.03	<0.01	<0.01	<0.01
N-Nitrosodimethylamine	μg/L	7.3	0.0003	0.001	0.001	0.001	0.0005	0.001	0.001	0.002	0.002	0.001	0.001	0.001	0.0003	0.001	0.001	0.0004	0.0005	0.001	0.001	0.001	0.002	0.001	0.001	0.001
N-Nitrosodi-N- Propylamine	μg/L	0.38	0.0003	0.001	0.001	0.001	0.0004	0.001	0.0004	0.001	0.001	0.0005	0.0004	0.0004	0.0003	0.001	0.001	0.0004	0.0004	0.0004	0.0003	0.001	0.001	0.0004	0.0004	0.0004
N-Nitrosodiphenylamine	μg/L	2.5	<1.1	<0.8	<0.6	<0.4	<0.1	<0.1	<0.9	<0.7	<0.5	<0.1	<0.1	<0.1	<1.1	<0.7	<0.6	<0.1	<0.1	<0.1	<0.9	<0.8	<0.5	<0.1	<0.1	<0.1
PAHs	μg/L	0.0088	0.0002	0.0005	0.0007	0.0006	0.0002	0.0002	0.0016	0.0015	0.0012	0.0005	0.0003	0.0003	0.0002	0.0006	0.0007	0.0002	0.0002	0.0002	0.0014	0.0014	0.0012	0.0003	0.0003	0.0003
PCBs	μg/L	0.000019	9E-06	1E-05	1E-05	1E-05	4E-06	4E-06	3E-05	3E-05	2E-05	9E-06	6E-06	5E-06	9E-06	1E-05	1E-05	4E-06	4E-06	4E-06	3E-05	3E-05	2E-05	6E-06	6E-06	6E-06
TCDD Equivalents °	μg/L	3.9E-09	1E-10	2E-09	2E-09	2E-09	8E-10	8E-10	5E-09	5E-09	4E-09	2E-09	1E-09	1E-09	8E-11	2E-09	2E-09	7E-10	8E-10	8E-10	4E-09	4E-09	4E-09	1E-09	1E-09	1E-09
1,1,2,2- Tetrachloroethane	μg/L	2.3	<0.05	<0.05	<0.04	<0.02	<0.01	<0.01	<0.05	<0.04	<0.03	<0.01	<0.01	<0.01	<0.06	<0.04	<0.03	<0.01	<0.01	<0.01	<0.05	<0.05	<0.03	<0.01	<0.01	<0.01
Tetrachloroethylene	μg/L	2.0	<0.1	<0.05	<0.04	<0.02	<0.01	<0.01	<0.05	<0.04	<0.03	<0.01	<0.01	<0.01	<0.1	<0.04	<0.03	<0.01	<0.01	<0.01	<0.1	<0.05	<0.03	<0.01	<0.01	<0.01
Toxaphene e	μg/L	2.1E-04	7E-06	8E-05	1E-04	1E-04	4E-05	4E-05	3E-04	3E-04	2E-04	8E-05	5E-05	5E-05	7E-06	9E-05	1E-04	4E-05	4E-05	4E-05	2E-04	2E-04	2E-04	6E-05	5E-05	5E-05
Trichloroethylene	μg/L	27	<0.1	<0.05	<0.04	<0.02	<0.01	<0.01	<0.05	<0.04	<0.03	<0.01	<0.01	<0.01	<0.1	<0.04	<0.03	<0.01	<0.01	<0.01	<0.1	<0.05	<0.03	<0.01	<0.01	<0.01
1,1,2- Trichloroethane	μg/L	9.4	<0.1	<0.05	<0.04	<0.02	<0.01	<0.01	<0.05	<0.04	<0.03	<0.01	<0.01	<0.01	<0.1	<0.04	<0.03	<0.01	<0.01	<0.01	<0.1	<0.05	<0.03	<0.01	<0.01	<0.01
2,4,6- Trichlorophenol <sup>b</sup>	μg/L	0.29	<1.1	<0.8	<0.6	<0.4	<0.1	<0.1	<0.9	<0.7	<0.5	<0.1	<0.1	<0.1	<1.1	<0.7	<0.6	<0.1	<0.1	<0.1	<0.9	<0.8	<0.5	<0.1	<0.1	<0.1
Vinyl chloride	μg/L	36	<0.03	<0.03	<0.02	<0.02	<0.005	<0.01	<0.03	< 0.03	<0.02	<0.01	<0.005	<0.004	<0.03	<0.03	<0.02	<0.01	<0.01	<0.005	<0.03	<0.03	<0.02	<0.01	<0.01	<0.005

- a: Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based the nature of the constituent. b: All observed values from some data sources were below the MRL, and the flow-weighted average of the MRLs is higher than the Ocean Plan objective. No compliance conclusions can be drawn for these constituents.
- c: Acrylonitrile was only detected in one potential source water for the Variant Project. It was not detected in any potential source waters for the MPWSP Project; therefore, a compliance determination cannot be made for the MPWSP Project and only partial determination can be made for the Variant Project.
- d: Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time.
- e: Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.

Table A4 – Complete list of estimated concentrations at the edge of the ZID expressed as a percentage of Ocean Plan<sup>a</sup>

		_								Est. F	ercenta	ge of O	cean Pla	n object	ive at E	dge of Z	ID by FI	ow Scer	nario							
Constituent	Units	Ocean Plan		Varia	ant with	GWR O	ffline			Varia	nt with	Normal	Flows		Varia	ant with	<b>.</b>	esal Brir Offline	e Flows	and	Va	riant wit	h High [	Desal Br	ine Flow	/S
		Objective	15	16	17	18	19	20	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47
Objectives for prote	ection of n	narine aquati	c life - (	6-month	median	limit																				
Arsenic	μg/L	8	49%	52%	51%	48%	41%	41%	48%	50%	48%	42%	41%	41%	49%	51%	51%	41%	41%	41%	48%	49%	49%	41%	41%	41%
Cadmium	μg/L	1	32%	25%	18%	12%	2%	2%	31%	24%	16%	5%	3%	2%	32%	23%	18%	4%	3%	3%	32%	28%	19%	4%	3%	3%
Chromium (Hexavalent)	μg/L	2	5%	5%	4%	3%	1%	1%	8%	7%	5%	2%	1%	1%	4%	4%	4%	1%	1%	1%	7%	7%	5%	1%	1%	1%
Copper	μg/L	3	64%	68%	70%	70%	68%	68%	78%	77%	75%	70%	69%	69%	64%	68%	70%	68%	68%	68%	75%	76%	75%	69%	69%	69%
Lead	μg/L	2	2%	3%	3%	2%	1%	1%	6%	6%	5%	2%	1%	1%	2%	3%	3%	1%	1%	1%	6%	6%	5%	1%	1%	1%
Mercury	μg/L	0.04	66%	52%	38%	25%	6%	5%	65%	50%	34%	12%	7%	6%	68%	49%	37%	9%	8%	7%	66%	59%	40%	9%	8%	7%
Nickel	μg/L	5	14%	13%	11%	8%	2%	2%	21%	17%	13%	4%	3%	2%	14%	12%	11%	3%	2%	2%	20%	18%	14%	3%	3%	3%
Selenium	μg/L	15	4%	3%	3%	2%	0.5%	0.4%	5%	4%	3%	1%	1%	0%	4%	3%	3%	1%	1%	0.5%	5%	4%	3%	1%	1%	1%
Silver	μg/L	0.7	26%	26%	26%	25%	24%	23%	29%	28%	27%	24%	24%	24%	26%	26%	26%	24%	24%	24%	29%	28%	27%	24%	24%	24%
Zinc	μg/L	20	41%	43%	44%	44%	41%	41%	51%	50%	48%	43%	42%	42%	41%	43%	44%	41%	41%	41%	49%	49%	48%	42%	42%	42%
Cyanide	μg/L	1	61%	138%	163%	139%	53%	55%	150%	189%	173%	71%	55%	56%	61%	144%	158%	49%	53%	55%	135%	158%	176%	55%	56%	57%
Total Chlorine Residual	μg/L	2			ı	1	1		1		ı	ı	I	ı			ı	ı			ı		1	1	I	
Ammonia (as N) - 6-mo median	μg/L	600	7%	79%	108%	97%	40%	42%	266%	258%	208%	79%	54%	53%	6%	86%	105%	35%	39%	41%	222%	227%	205%	56%	54%	53%
Ammonia (as N) - Daily Max	μg/L	2,400	2%	22%	31%	28%	11%	12%	76%	74%	59%	23%	16%	15%	2%	25%	30%	10%	11%	12%	63%	65%	58%	16%	16%	15%
Acute Toxicity <sup>a</sup>	TUa	0.3			-				-		-	-		-			-	-			-		1	-	-	
Chronic Toxicity <sup>a</sup>	TUc	1	-																							
Phenolic Compounds (non- chlorinated)	μg/L	30	18%	16%	13%	9%	2%	2%	24%	20%	14%	5%	3%	3%	19%	15%	13%	3%	3%	2%	23%	21%	16%	3%	3%	3%
Chlorinated Phenolics <sup>b</sup>	μg/L	1			1		<23%	<21%	1		1	<41%	<24%	<22%			1	<31%	<28%	<25%	1	-	ı	<30%	<27%	<24%
Endosulfan	μg/L	0.009	0.4%	6%	8%	7%	3%	3%	19%	18%	15%	6%	4%	4%	0%	6%	7%	3%	3%	3%	16%	16%	15%	4%	4%	4%
Endrin	μg/L	0.002	0.01%	0.1%	0.1%	0.1%	0.03%	0.03%	0.2%	0.2%	0.2%	0.1%	0.04%	0.04%	0.01%	0.1%	0.1%	0.03%	0.03%	0.03%	0.2%	0.2%	0.2%	0.04%	0.04%	0.04%
HCH (Hexachlorocyclohexane)	μg/L	0.004	1%	16%	22%	20%	8%	9%	55%	53%	43%	16%	11%	11%	1%	18%	22%	7%	8%	8%	46%	47%	42%	12%	11%	11%
Radioactivity (Gross Beta) <sup>a</sup>	pci/L	0.0			1				-		-	-	-	-			-	-			-		1	1	1	
Radioactivity (Gross Alpha) <sup>a</sup>	pci/L	0.0						-																		

# MPWSP AND VARIANT OCEAN PLAN COMPLIANCE

										Est. F	Percenta	ge of O	cean Pla	ın object	tive at E	dge of Z	ID by Fl	ow Scer	nario							
Constituent	Units	Ocean Plan		Vari	ant with	GWR O	ffline			Varia	ant with	Normal	Flows		Vari	ant with		esal Brir Offline	ne Flows	and	Va	riant wit	h High I	Desal Bı	rine Flov	ws
		Objective	15	16	17	18	19	20	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47
Objectives for prote Acrolein	ection of h µg/L	uman health 220	- non c	arcinogo 0.1%	ens – 30 0.1%	0.1%	erage lin 0.03%	0.03%	0.2%	0.2%	0.1%	0.1%	0.03%	0.03%	0.1%	0.1%	0.1%	0.03%	0.03%	0.03%	0.2%	0.2%	0.2%	0.04%	0.04%	0.03%
Antimony	µg/L	1200	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Bis (2- chloroethoxy) methane	μg/L	4.4	<24%	<19%	<14%	<9%	<2%	<2%	<20%	<16%	<11%	<4%	<2%	<2%	<25%	<18%	<13%	<3%	<2%	<2%	<21%	<19%	<13%	<3%	<2%	<2%
Bis (2- chloroisopropyl) ether	μg/L	1200	<0.1%	<0.1%	<0.1%	<0.03%	<0.01%	<0.01%	<0.1%	<0.1%	<0.04%	<0.01%	<0.01%	<0.01%	<0.1%	<0.1%	<0.05%	<0.01%	<0.01%	<0.01%	<0.1%	<0.1%	<0.05%	<0.01%	<0.01%	<0.01%
Chlorobenzene	μg/L	570	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Chromium (III)	μg/L	190000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Di-n-butyl phthalate Dichlorobenzenes	μg/L μg/L	3500 5100	<0.03%	<0.02% <0.01%	<0.02% <0.01%	<0.01%	<0.01%	<0.01% <0.01%	<0.03% <0.01%	<0.02%	<0.01%	<0.01%	<0.01%	<0.01% <0.01%	<0.03% <0.01%	<0.02%	<0.02% <0.01%	<0.01% <0.01%	<0.01%	<0.01% <0.01%	<0.03% <0.01%	<0.02% <0.01%	<0.02% <0.01%	<0.01%	<0.01%	<0.01%
Diethyl phthalate	μg/L μg/L	33000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Dimethyl phthalate	ua/L	820000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
4,6-dinitro-2- methylphenol	µg/L	220	<2%	<2%	<1%	<1%	<0.2%	<0.2%	<2%	<2%	<1%	<0.4%	<0.2%	<0.2%	<2%	<2%	<1%	<0.3%	<0.2%	<0.2%	<2%	<2%	<1%	<0.3%	<0.2%	<0.2%
2,4-Dinitrophenol b	μq/L	4.0			<74%	<47%	<9%	<7%			<58%	<19%	<10%	<8%			<72%	<14%	<12%	<10%			<70%	<14%	<11%	<9%
Ethylbenzene	µg/L	4100	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Fluoranthene	μg/L	15	0.1%	0.1%	0.04%	0.02%	<0.01%	<0.01%	0.1%	0.05%	0.03%	0.01%	<0.01%	<0.01%	0.1%	0.1%	0.04%	0.01%	0.01%	<0.01%	0.1%	0.1%	0.04%	0.01%	0.01%	<0.01%
Hexachlorocyclope ntadiene	μg/L	58	<0.01%	<0.02%	<0.02%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.02%	<0.02%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Nitrobenzene	μg/L	4.9	<53%	<40%	<28%	<18%	<3%	<2%	<45%	<34%	<22%	<7%	<4%	<3%	<54%	<37%	<28%	<5%	<5%	<4%	<47%	<42%	<27%	<5%	<4%	<3%
Thallium	μg/L	2	0.3%	1%	1%	1%	0.2%	0.2%	1%	1%	1%	0.4%	0.3%	0.3%	0.3%	1%	1%	0.2%	0.2%	0.2%	1%	1%	1%	0.3%	0.3%	0.3%
Toluene	μg/L	85000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Tributyltin b	μg/L	0.0014					<41%	<36%				<69%	<42%	<37%				<53%	<49%	<44%				<51%	<46%	<42%
1,1,1- Trichloroethane	μg/L	540000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Objectives for prote	ction of h					average										,										
Acrylonitrile c	μg/L	0.10	2%	28%	38%	34%	14%	14%	94%	92%	74%	28%	19%	19%	1%	30%	37%	13%	14%	15%	79%	81%	73%	20%	19%	19%
Aldrin b	μg/L	0.000022	<41%												<38%											
Benzene Benzidine b	μg/L	5.9 0.000069	<1%	<1%	<1%	<0.4%	<0.1%	<0.1%	<1%	<1%	<0.5%	<0.2%	<0.1%	<0.1%	<1%	<1%	<1%	<0.1%	<0.1%	<0.1%	<1%	<1%	<1%	<0.1%	<0.1%	<0.1%
Beryllium °	μg/L μg/L	0.000069	0.01%	0.01%	0.01%	<0.01%	<0.01%	<0.01%	0.01	0.01%	<0.01%	<0.01%	<0.01%	<0.01%	0.01%	0.01%	<0.01%	<0.01%	<0.01%	<0.01%	0.01%	0.01%	<0.01%	<0.01%	<0.01%	<0.01%
Bis(2- chloroethyl)ether b	μg/L	0.045																								
Bis(2-ethyl- hexyl)phthalate	μg/L	3.5	3%	26%	34%	31%	12%	13%	84%	81%	65%	25%	17%	17%	3%	28%	33%	11%	12%	13%	70%	72%	64%	18%	17%	17%
Carbon tetrachloride	μg/L	0.90	6%	5%	4%	3%	1%	1%	7%	6%	4%	1%	1%	1%	6%	5%	4%	1%	1%	1%	7%	6%	5%	1%	1%	1%
Chlordane	μg/L	0.000023	8%	60%	81%	72%	30%	31%	199%	193%	155%	59%	40%	39%	7%	66%	79%	26%	29%	30%	167%	170%	153%	42%	40%	40%
Chlorodibromo- methane	μg/L	8.6	1%	1%	1%	1%	0.2%	0.2%	1%	1%	1%	0.4%	0.2%	0.2%	1%	1%	1%	0.2%	0.2%	0.2%	1%	1%	1%	0.3%	0.2%	0.2%
Chloroform	μg/L	130	0.1%	0.3%	0.4%	0.4%	0.1%	0.2%	1%	1%	1%	0.3%	0.2%	0.2%	0.1%	0.3%	0.4%	0.1%	0.1%	0.2%	1%	1%	1%	0.2%	0.2%	0.2%



										Est. F	Percenta	ige of O	cean Pla	an object	tive at E	dge of Z	ID by FI	ow Scer	nario							
Constituent	Units	Ocean Plan		Vari	ant with	GWR O	ffline			Varia	nt with	Normal	Flows		Vari	ant with	High De	esal Brir Offline	e Flows	and	Va	riant wi	th High [	Desal Br	ine Flov	vs
		Objective	15	16	17	18	19	20	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47
DDT	μg/L	0.00017	0.5%	6%	9%	8%	3%	3%	1%	6%	7%	3%	3%	3%	0.4%	7%	8%	3%	3%	3%	1%	4%	7%	3%	3%	3%
1,4- Dichlorobenzene	μg/L	18	0.3%	0.3%	0.3%	0.2%	0.1%	0.1%	1%	1%	0.4%	0.1%	0.1%	0.1%	0.3%	0.3%	0.3%	0.1%	0.1%	0.1%	1%	1%	0.4%	0.1%	0.1%	0.1%
3,3- Dichlorobenzidine <sup>b</sup>	μg/L	0.0081			-			1										-								
1,2-Dichloroethane	μg/L	28	<0.2%	<0.2%	<0.1%	<0.1%	<0.02%	<0.02%	<0.2%	<0.1%	<0.1%	<0.04%	<0.02%	<0.02%	<0.2%	<0.2%	<0.1%	<0.03%	<0.03%	<0.02%	<0.2%	<0.2%	<0.1%	<0.03%	<0.02%	<0.02%
1,1- Dichloroethylene	μg/L	0.9	6%	5%	4%	3%	1%	1%	6%	5%	3%	1%	1%	1%	6%	5%	4%	1%	1%	1%	6%	5%	4%	1%	1%	1%
Dichlorobromo- methane	μg/L	6.2	1%	1%	1%	1%	0.3%	0.3%	2%	2%	1%	1%	0.3%	0.3%	1%	1%	1%	0.3%	0.3%	0.3%	2%	2%	2%	0.4%	0.4%	0.3%
Dichloromethane	μg/L	450	0.01%	0.01%	0.01%	0.01%	<0.01%	<0.01%	0.02%	0.01%	0.01%	<0.01%	<0.01%	<0.01%	0.01%	0.01%	0.01%	<0.01%	<0.01%	<0.01%	0.02%	0.02%	0.01%	<0.01%	<0.01%	<0.01%
1,3- dichloropropene	μg/L	8.9	1%	1%	0.4%	0.3%	0.1%	0.1%	1%	1%	0.4%	0.1%	0.1%	0.1%	1%	0.5%	0.4%	0.1%	0.1%	0.1%	1%	1%	0.5%	0.1%	0.1%	0.1%
Dieldrin	μg/L	0.00004	10%	47%	61%	53%	21%	22%	11%	41%	48%	22%	19%	21%	10%	50%	59%	19%	21%	22%	10%	26%	48%	18%	20%	21%
2,4-Dinitrotoluene	μg/L	2.6	<0.5%	<1%	<1%	<1%	<0.5%	<1%	<0.4%	<1%	<1%	<0.5%	<0.4%	<0.4%	<0.5%	<1%	<1%	<0.4%	<0.5%	<0.5%	<0.4%	<1%	<1%	<0.4%	<0.4%	<0.4%
1,2- Diphenylhydrazine b	μg/L	0.16			1		<51%	<42%					<54%	<45%				<76%	<67%	<56%				<72%	<62%	<53%
Halomethanes	μg/L	130	0.04%	0.04%	0.04%	0.03%	0.01%	0.01%	0.1%	0.1%	0.05%	0.02%	0.01%	0.01%	0.04%	0.04%	0.04%	0.01%	0.01%	0.01%	0.1%	0.1%	0.05%	0.01%	0.01%	0.01%
Heptachlor b	μg/L	0.00005	<14%												<12%											
Heptachlor Epoxide	μg/L	0.00002	1%	5%	7%	6%	2%	3%	17%	16%	13%	5%	3%	3%	1%	6%	7%	2%	2%	3%	14%	14%	13%	3%	3%	3%
Hexachlorobenzene	μg/L	0.00021	2%	2%	2%	1%	0.3%	0.3%	3%	3%	2%	1%	0.4%	0.4%	2%	2%	2%	0.4%	0.4%	0.3%	3%	3%	2%	0.5%	0.4%	0.4%
Hexachlorobutadiene	μg/L	14							-																	
Hexachloroethane	μg/L	2.5	<42%	<32%	<23%	<15%	<3%	<2%	<36%	<27%	<18%	<6%	<3%	<2%	<43%	<30%	<23%	<4%	<4%	<3%	<37%	<33%	<22%	<4%	<4%	<3%
Isophorone	μg/L	730	<0.01%	<0.01%				-	<0.01%	<0.01%					<0.01%	<0.01%					<0.01%	<0.01%				
N-Nitrosodimethylamine	μg/L	7.3		0.01%	0.02%	0.02%	0.01%	0.01%	0.02%	0.02%	0.02%	0.01%	0.01%	0.01%		0.02%	0.02%	0.01%	0.01%	0.01%	0.02%	0.02%	0.02%	0.01%	0.01%	0.01%
N-Nitrosodi-N- Propylamine	μg/L	0.38	0.1%	0.3%	0.3%	0.3%	0.1%	0.1%	0.1%	0.2%	0.3%	0.1%	0.1%	0.1%	0.1%	0.3%	0.3%	0.1%	0.1%	0.1%	0.1%	0.2%	0.3%	0.1%	0.1%	0.1%
N-Nitrosodiphenylamine	μg/L	2.5	<42%	<32%	<23%	<15%	<3%	<2%	<36%	<27%	<18%	<6%	<3%	<2%	<43%	<30%	<23%	<4%	<4%	<3%	<37%	<33%	<22%	<4%	<4%	<3%
PAHs	μg/L	0.0088	2%	6%	8%	7%	3%	3%	18%	18%	14%	5%	4%	3%	2%	7%	7%	2%	3%	3%	16%	16%	14%	4%	4%	4%
PCBs	μg/L	0.000019	47%	71%	77%	63%	22%	23%	169%	156%	121%	45%	30%	28%	47%	73%	74%	22%	23%	23%	149%	147%	124%	32%	30%	29%
TCDD Equivalents c	μg/L	3.9E-09	2%	39%	53%	48%	20%	21%	131%	128%	103%	39%	27%	26%	2%	42%	52%	17%	19%	20%	110%	112%	101%	28%	27%	26%
1,1,2,2- Tetrachloroethane	μg/L	2.3	<2%	<2%	<2%	<1%	<0.3%	<0.2%	<2%	<2%	<1%	<0.4%	<0.3%	<0.2%	<2%	<2%	<2%	<0.3%	<0.3%	<0.3%	<2%	<2%	<1%	<0.3%	<0.3%	<0.3%
Tetrachloroethylene	μg/L	2.0	3%	2%	2%	1%	0.3%	0.3%	2%	2%	1%	1%	0.3%	0.3%	3%	2%	2%	0.4%	0.4%	0.3%	3%	2%	2%	0.4%	0.3%	0.3%
Toxaphene e	μg/L	2.1E-04	4%	38%	51%	46%	19%	20%	126%	122%	98%	37%	26%	25%	3%	41%	50%	17%	19%	19%	105%	108%	97%	26%	26%	25%
Trichloroethylene	μg/L	27	0.2%	0.2%	0.1%	0.1%	0.02%	0.02%	0.2%	0.2%	0.1%	0.04%	0.02%	0.02%	0.2%	0.2%	0.1%	0.03%	0.03%	0.02%	0.2%	0.2%	0.1%	0.03%	0.03%	0.02%
1,1,2- Trichloroethane	μg/L	9.4	1%	0.5%	0.4%	0.3%	0.1%	0.1%	1%	0.4%	0.3%	0.1%	0.1%	0.1%	1%	0.5%	0.4%	0.1%	0.1%	0.1%	1%	1%	0.4%	0.1%	0.1%	0.1%
2,4,6- Trichlorophenol <sup>b</sup>	μg/L	0.29					25%	20%				51%	27%	21%				39%	33%	27%				37%	31%	26%
Vinyl chloride	μg/L	36	0.1%	0.1%	0.1%	0.05%	0.01%	0.01%	0.1%	0.1%	0.1%	0.02%	0.01%	0.01%	0.1%	0.1%	0.1%	0.02%	0.01%	0.01%	0.1%	0.1%	0.1%	0.02%	0.01%	0.01%

- a: Note that if the percentage was determined to be less than 0.01 percent, then a minimum value is shown as "<0.01%" (e.g., if the constituent was estimated to be 0.000001% of the objective, for simplicity, it is displayed as <0.01%). Also, shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.
- b: Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based the nature of the constituent. These constituents were measured individually for the secondary effluent and GWR concentrate, and these individual concentrations would comply with the Ocean Plan objectives.
- c: All observed values from all data sources were below the MRL, and the flow-weighted average of the MRLs is higher than the Ocean Plan objective. No compliance conclusions can be drawn for these constituents.
- d: Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time.
- e: Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.



# **Appendix B**

Trussell Technologies, Inc (Trussell Tech), 2017. "Ocean Plan Compliance Assessment for the Pure Water Monterey Groundwater Replenishment Project." *Technical Memorandum prepared for MRWPCA and MPWMD*. September.



# **Appendix C**

Trussell Technologies, Inc (Trussell Tech), 2016. "Revised Ocean Plan Compliance Assessment for the Monterey Peninsula Water Supply Project and Project Variant." *Technical Memorandum prepared for MRWPCA and MPWMD*. July.

# DRAFT Revised Ocean Plan Compliance Assessment for the Monterey Peninsula Water Supply Project and Project Variant

**Technical Memorandum**July 2016

# Prepared for:





# DRAFT Revised Ocean Plan Compliance Assessment for the Monterey Peninsula Water Supply Project and Project Variant

# **Technical Memorandum**



Prepared By:

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# **Table of Contents**

1 Introduction	2
1.1 Treatment through the Proposed CalAm Desalination Facility	3
1.2 Treatment through the RTP and Proposed AWT Facilities	3
1.3 California Ocean Plan	4
1.4 Future Ocean Discharges	5
1.5 Objective of Technical Memorandum	8
2 Methodology for Ocean Plan Compliance Assessment	8
2.1 Methodology for Determination of Discharge Water Quality	
2.1.1 Secondary Effluent	10
2.1.2 Desalination Brine	10
2.1.3 Combined Ocean Discharge Concentrations	11
2.2 Ocean Modeling Methodology	12
2.2.1 Ocean Modeling Scenarios	13
2.2.2 Ocean Modeling Assumptions	17
3 Ocean Plan Compliance Results	
3.1 Water Quality of Combined Discharge	
3.2 Ocean Modeling Results	20
3.3 Ocean Plan Compliance Results	22
4 Conclusions	28
5 References	29
Appendix A	30
Appendix B	30
Provided the second sec	



### 1 Introduction

In response to State Water Resources Control Board (SWRCB) Water Rights Orders WR 95-10 and WR 2009-0060, two proposed projects are in development on the Monterey Peninsula to provide potable water to offset pending reductions of Carmel River water diversions: (1) a seawater desalination project known as the **Monterey Peninsula Water Supply Project** (MPWSP), and (2) a groundwater replenishment project known as the **Pure Water Monterey Groundwater Replenishment Project** (GWR Project). The capacity of the MPWSP is dependent on whether the GWR Project is constructed.

If the GWR Project is not constructed, the MPWSP would entail California American Water ("CalAm") building a seawater desalination facility capable of producing 9.6 million gallons per day (mgd) of drinking water. In a variation of that project where the GWR Project is constructed, known as the **Monterey Peninsula Water Supply Project Variant** ("Variant"), CalAm would build a smaller desalination facility capable of producing 6.4 mgd of drinking water, and a partnership between the Monterey Peninsula Water Management District (MPWMD) and the Monterey Regional Water Pollution Control Agency (MRWPCA) would build an advanced water treatment facility ("AWT Facility") capable of producing up to 3,700 acre-feet per year (AFY) (3.3 mgd)<sup>1</sup> of highly purified recycled water to enable CalAm to extract 3,500 AFY (3.1 mgd) from the Seaside Groundwater Basin for delivery to their customers (the AWT Facility is part of the GWR Project).

The AWT Facility would purify secondary-treated wastewater (*i.e.*, secondary effluent) from MRWPCA's Regional Treatment Plant (RTP), and this highly purified recycled water would be injected into the Seaside Groundwater Basin and later extracted for municipal water supplies. Both the proposed desalination facility and the proposed AWT Facility would employ reverse osmosis (RO) membranes to purify the waters, and as a result, both projects would produce RO concentrate waste streams that would be disposed through the existing MRWPCA ocean outfall: the brine concentrate from the desalination facility ("Desal Brine"), and the RO concentrate from the AWT Facility ("GWR Concentrate").

The goal of this technical memorandum is to analyze whether the discharges from the proposed projects through the existing ocean outfall would impact marine water quality, and thus, human health, marine biological resources, or beneficial uses of the receiving waters. A similar assessment of the GWR Project on its own was previously performed (Trussell Technologies, 2015, see Appendix B), and so this document provides complementary information focused on the MPWSP and the Variant projects.

The original version of this document (Trussell Technologies, 2015b) and an addendum report to that document (Trussell Technologies, 2015c) were included in both the GWR Project Consolidated Final Environmental Impact Report (CFEIR) and the MPWSP draft Environmental Impact Report (EIR). This version has been updated to include new water quality data and flow

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<sup>&</sup>lt;sup>1</sup> One million gallons per day is equal to 1,121 acre-feet per year. The AWT Facility would be capable of producing up to 4 mgd of highly purified recycled water on a daily basis, but production would fluctuate throughout the year, such that the average annual production would be 3.3 mgd (3,700 AFY) in a non-drought year.

scenarios for the MPWSP and Variant to address data gaps noted in the original analyses (2015b and 2015c).

#### 1.1 Treatment through the Proposed CalAm Desalination Facility

This section describes the proposed treatment train for the MPWSP and Variant desalination facility. Seawater from the Monterey Bay would be extracted through subsurface slant wells beneath the ocean floor and piped to a new CalAm-owned desalination facility. This facility would consist of granular media pressure filters, cartridge filters, a two-pass RO membrane system, RO product-water stabilization (for corrosion control), and disinfection (Figure 1). The RO process is expected to recover 42 percent of the influent seawater flow as product water, while the remainder of the concentrated influent water becomes the Desal Brine. The MPWSP and Variant product water (desalinated water) would be used for municipal drinking water, while the Desal Brine would be blended with (1) available RTP secondary effluent, (2) brine that is trucked and stored at the RTP, and (3) GWR Concentrate (for the Variant only), and discharged to the ocean through the existing MRWPCA ocean outfall. The volume of Desal Brine is dependent on the project size: 13.98 and 8.99 mgd for the MPWSP and Variant, respectively.

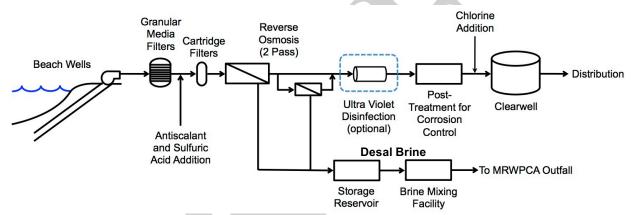


Figure 1 – Schematic of CalAm desalination facilities

## 1.2 Treatment through the RTP and Proposed AWT Facilities

The existing MRWPCA RTP treatment process includes screening, primary sedimentation, secondary biological treatment through trickling filters followed by a solids contactor (*i.e.*, bioflocculation), and clarification (Figure 2). Much of the secondary effluent undergoes tertiary treatment (granular media filtration and disinfection) to produce recycled water used for agricultural irrigation. The unused secondary effluent is discharged to the Monterey Bay through the MRWPCA outfall. MRWPCA also accepts trucked brine waste for ocean disposal ("hauled brine"), which is stored in a pond and mixed with secondary effluent for disposal.

The proposed AWT Facility would include several advanced treatment technologies for purifying the secondary effluent: ozone (O<sub>3</sub>), biologically active filtration (BAF) (this is an optional unit process), membrane filtration (MF), RO, and an advanced oxidation process (AOP) using ultraviolet light ("UV") and hydrogen peroxide. MRWPCA and the MPWMD conducted a pilot-scale study of the ozone, MF, and RO components of the AWT Facility from December 2013 through July 2014, successfully demonstrating the ability of the various treatment processes to produce highly purified recycled water that complies with the California

Groundwater Replenishment Water Recycling Criteria ("Groundwater Replenishment Regulations"),<sup>2</sup> the SWRCB's Anti-degradation and Recycled Water Policies,<sup>3</sup> and the Water Quality Control Plan for the Central Coastal Basin (Basin Plan)<sup>4</sup> standards, objectives and guidelines for groundwater. Water quality monitoring of the concentrate from the RO was also conducted during the pilot-scale study.

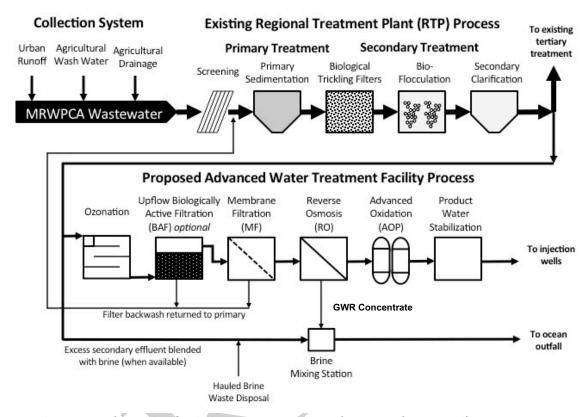


Figure 2 – Schematic of existing MRWPCA RTP and proposed AWT Facility treatment

#### 1.3 California Ocean Plan

The SWRCB 2012 Ocean Plan ("Ocean Plan") sets forth water quality objectives for the ocean with the intent of preserving the quality of the ocean water for beneficial uses, including the protection of both human and aquatic ecosystem health (SWRCB, 2012). Regional Water Quality Control Boards utilize these objectives to develop water quality-based effluent limitations for ocean dischargers that have a reasonable potential to exceed the water quality objectives.

When municipal wastewater flows are released from an outfall, the wastewater and ocean water undergo rapid mixing due to the momentum (from specially designed diffusers) and buoyancy of

<sup>&</sup>lt;sup>2</sup> SWRCB (2014) Water Recycling Criteria. Title 22, Division 4, Chapter 3, California Code of Regulations.

<sup>&</sup>lt;sup>3</sup> See http://www.swrcb.ca.gov/plans policies/

<sup>&</sup>lt;sup>4</sup> See http://www.waterboards.ca.gov/centralcoast/publications\_forms/publications/basin\_plan/docs/basin\_plan\_2011.pdf

the discharge.<sup>5</sup> The mixing occurring in the rising plume is affected by the buoyancy and momentum of the discharge, a process referred to as initial dilution (NRC, 1993). For rising plumes, the Ocean Plan defines the initial dilution as complete when "the diluting wastewater ceases to rise in the water column and first begins to spread horizontally," (*i.e.*, when the momentum from the discharge has dissipated). For more saline discharges, a sinking plume can form when the discharge is denser than the ambient water (also known as a negatively buoyant plume). In the case of negatively buoyant plumes, the Ocean Plan defines the initial dilution as complete when "the momentum induced velocity of the discharge ceases to produce significant mixing of the waste, or the diluting plume reaches a fixed distance from the discharge to be specified by the Regional Board, whichever results in the lower estimate for initial dilution."

The Ocean Plan objectives are to be met after the initial dilution of the discharge. The initial dilution occurs in an area known as the zone of initial dilution (ZID). The extent of dilution in the ZID is quantified and referred to as the minimum probable initial dilution ( $D_m$ ). The water quality objectives established in the Ocean Plan are adjusted by the  $D_m$  to derive the National Pollutant Discharge Elimination System (NPDES) permit limits for a wastewater discharge prior to ocean dilution.

The current MRWPCA wastewater discharge is governed by NPDES permit R3-2014-0013 issued by the Central Coast Regional Water Quality Control Board ("RWQCB"). Because the existing NPDES permit for the MRWPCA ocean outfall must be amended to discharge Desal Brine, comparing future discharge concentrations to the current NPDES permit limits (that will likely change when the permit is amended) would not be an appropriate metric or threshold for determining whether the proposed projects would have a significant impact on marine water quality. Instead, compliance with the Ocean Plan objectives was selected as an appropriate threshold for determining whether or not the proposed projects would result in a significant impact requiring mitigation.

Dr. Philip Roberts, a Professor in the School of Civil and Environmental Engineering at the Georgia Institute of Technology, conducted modeling of the ocean discharge and estimated  $D_m$  values for scenarios involving different flows of the proposed projects and different ambient ocean conditions. These ocean modeling results were combined with projected discharge water quality to assess compliance with the Ocean Plan.

# 1.4 Future Ocean Discharges

A summary schematic of the MPWSP and Variant is presented in Figure 3. For the MPWSP, 23.58 mgd of ocean water (design capacity) would be treated in the desalination facility; an RO recovery of 42% would lead to an MPWSP Desal Brine flow of 13.98 mgd that would be discharged through the outfall. Secondary effluent from the RTP would also be discharged through the outfall, although the flow would be variable depending on both the raw wastewater flow and the proportion being processed through the tertiary treatment system at the Salinas Valley Reclamation Plant (SVRP) to produce recycled water for agricultural irrigation. The third

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<sup>&</sup>lt;sup>5</sup> Municipal wastewater effluent, being effectively fresh water in terms of salinity, is less dense than seawater and thus rises (due to buoyancy) while it mixes with ocean water. GWR Concentrate, whether by itself or mixed with municipal wastewater effluent, is less dense than seawater and also rises (due to buoyancy) while it mixes with ocean water.

and final discharge component is hauled brine that is trucked to the RTP and blended with secondary effluent prior to discharge. The maximum anticipated flow of this stream is 0.1 mgd (blend of brine and secondary effluent). These three discharge components (Desal Brine, secondary effluent, and hauled brine) would be mixed at the proposed Brine Mixing Facility prior to ocean discharge.

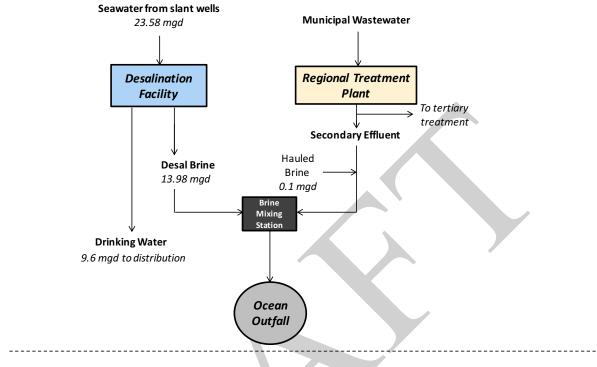
For the Variant, 15.93 mgd of ocean water (design capacity) would be pumped to the desalination facility, and an RO recovery of 42% would result in a Variant Desal Brine flow of 8.99 mgd. The Variant would include the GWR Project, which involves the addition of new source waters to the RTP that would alter the water quality of the secondary effluent produced by the RTP. The secondary effluent in the Variant is referred to as "Variant secondary effluent," and would be different in quality from the MPWSP secondary effluent. Under the GWR Project, a portion of the secondary effluent would be fed to the AWT Facility, and the resultant GWR Concentrate (maximum 0.94 mgd) would be discharged through the outfall. The hauled brine received at the RTP would continue to be blended with secondary effluent prior to discharge, the quality of the blended brine and secondary effluent will change as a result of the change in secondary effluent quality; the hauled brine for the Variant is referred to as "Variant hauled brine." The discharge components for the MPWSP and Variant are summarized in Table 1.

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Project	Desal Brine	Secondary Effluent	Variant Secondary Effluent	Hauled Brine	Variant Hauled Brine <sup>a</sup>	GWR Concentrate
MPWSP	✓	✓		<b>√</b>		
1111 1101	(13.98 mgd)	(flow varies)		(0.1 mgd)		
Variant	✓		<b>√</b>		✓	<b>√</b>
Variani	(8.99 mgd)		(flow varies)		(0.1 mgd)	(0.94 mgd)

Table 1 - Discharge waters Included in each analysis

<sup>&</sup>lt;sup>a</sup> This is placed in a separate category because it contains Variant secondary effluent.

#### **MPWSP**



# MPWSP Variant ("Variant")

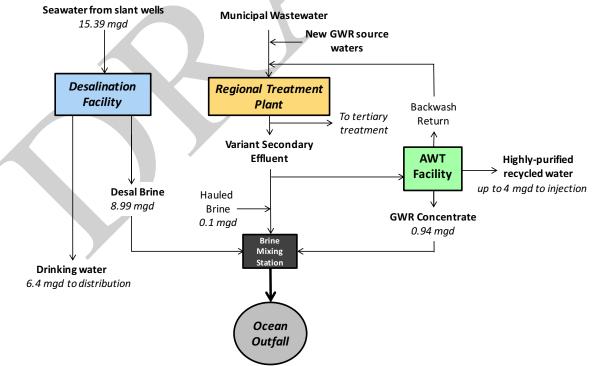


Figure 3 – Flow schematics for the MPWSP and Variant projects (specified flow rates are at design capacity)

#### 1.5 Objective of Technical Memorandum

Trussell Technologies, Inc. ("Trussell Tech") estimated worst-case in-pipe water quality for the various ocean discharge scenarios (*i.e.*, prior to dilution through ocean mixing) for the proposed projects. Dr. Roberts' ocean discharge modeling and the results of the water quality analysis were then used to provide an assessment of whether the proposed projects would consistently meet Ocean Plan water quality objectives. The objective of this technical memorandum is to summarize the assumptions, methodology, results and conclusions of the Ocean Plan compliance assessment for the MPWSP and Variant.

# 2 Methodology for Ocean Plan Compliance Assessment

Water quality data from various sources for the different treatment process influent and waste streams were compiled. Trussell Tech combined these data for different flow scenarios and used ocean modeling results (i.e.,  $D_m$  values) to assess compliance of different discharge scenarios with the Ocean Plan objectives. This section documents the data sources and provides further detail on the methodology used to perform this analysis. A summary of the methodology is presented in Figure 4.

## 2.1 Methodology for Determination of Discharge Water Quality

The amounts and combinations of various wastewaters that would be disposed through the MRWPCA outfall will vary depending on the capacity, seasonal and daily flow characteristics, and extent and timing of implementation of the proposed projects.

Detailed discussions about the methods used to determine the discharge water qualities related to the GWR Project were previously discussed and can be found in Appendix B. This previous analysis included water quality estimates of the secondary effluent, Variant secondary effluent, hauled brine, Variant hauled brine, and the GWR Concentrate (*i.e.*, all of the discharges except for the Desal Brine). In the previous analysis, Trussell Tech assumed that the highest observed values for the various Ocean Plan constituents within each type of water flowing to and treated at the RTP, including the AWT Facility as applicable, to be the worst-case water quality. These same data and assumptions were used in the analysis described in this memorandum. Use of these worst-case water quality concentrations ensures that the analysis in this memorandum is conservative related to the Ocean Plan compliance assessment (and thus, the impact analysis for the MPWSP environmental review processes).

To determine the impact of the MPWSP and Variant, the worst-case water quality of the Desal Brine was estimated using available data from CalAm's temporary test subsurface slant well on the CEMEX mine property in Marina, California. Long-term pumping and water quality

therefore the results were questionable. Therefore, although the cyanide concentrations reported by MBAS are presented, they are not used in the analysis for evaluating compliance with the Ocean Plan objectives.

<sup>&</sup>lt;sup>6</sup> The exception to this statement is cyanide. In mid-2011, Monterey Bay Analytical Service (MBAS) began performing the cyanide analysis on the RTP secondary effluent, at which time the reported values increased by an order of magnitude. Because no operational or source water composition changes took place at this time that would result in such an increase, it is reasonable to conclude the increase is an artifact of the change in analysis method and therefore the results were questionable. Therefore, although the exempted concentrations reported by MBAS are

sampling from this well began in April 2015.<sup>7</sup> As in the previous Ocean Plan compliance assessments, the highest observed concentrations in the slant well were used for this Ocean Plan compliance assessment.

The methodology for determining the water quality of the Desal Brine and secondary effluent is further described in this section (the methodology for all other discharge waters can be found in Appendix B). A summary of which discharge waters are considered for both the MPWSP and Variant, and which data sources were used in the determination of the water quality for each discharge stream is shown in Figure 4.

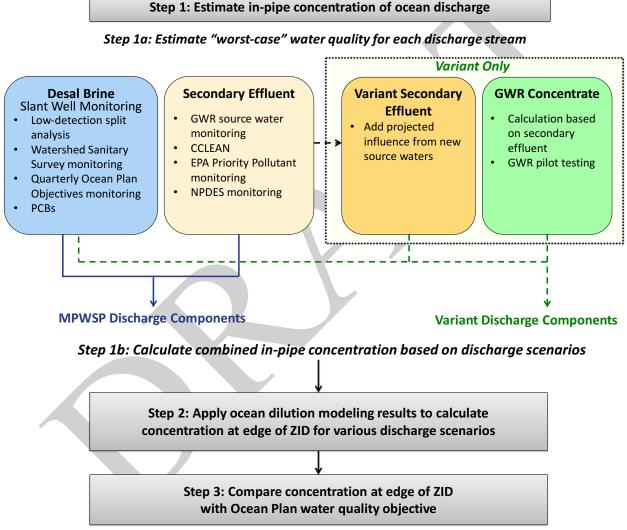


Figure 4 – Logic flow chart for determination of MPWSP and Variant compliance with Ocean Plan objectives.

<sup>&</sup>lt;sup>7</sup> The well was shut down on June 5, 2015 to assess regional trends in aquifer water levels and resumed pumping October 27, 2015. The well was shut down again between March 4, 2016 and May 2, 2016 for discharge line repairs. No water quality data were collected during shutdown periods.

#### 2.1.1 Secondary Effluent

For the MPWSP, the discharged secondary effluent would not be impacted by additional source waters that would be brought in for the Variant; therefore, the historical secondary effluent quality was used in the analysis. The following sources of data were considered for selecting a secondary effluent concentration for each constituent in the analysis:

- Secondary effluent water quality monitoring conducted for the GWR Project from July 2013 through June 2014.
- Historical NPDES compliance water quality data collected semi-annually by MRWPCA (2005-2014).
- Historical Priority Pollutant data collected annually by MRWPCA (2004-2014).
- Water quality data collected by the Central Coast Long-Term Environmental Assessment Network (CCLEAN) (2008-2015).

The secondary effluent concentration for each constituent selected for the analysis was the maximum reported value from the above sources. In some cases, constituents were not detected (ND) in any of the source waters; in these cases, the values are reported as ND(<MRL). In cases where the analysis of a constituent that was detected but not quantified, the result is reported as less than the Method Reporting Limit ND(<MRL). Because the actual concentration could be any value equal to or less than the MRL, the conservative approach is to use the value of the MRL. For some ND constituents, the MRL exceeds the Ocean Plan objective, and thus no compliance determination can be made. A detailed discussion of the cases where a constituent was reported as less than the MRL is included in the GWR Project technical memorandum in Appendix B (Trussell Technologies, 2015a).

#### 2.1.2 Desalination Brine

Trussell Tech used the following four sources of data for the Desal Brine water quality assessment:

• A one-time 7-day composite sample from the test slant well with separate analysis of particulate and dissolved phase fractions of constituents using low-detection CCLEAN analysis techniques (February 18-25, 2016). The maximum total concentration was used in this analysis (*i.e.* the sum of the concentration in the particulate and dissolved phase

<sup>&</sup>lt;sup>8</sup> The lowest amount of an analyte in a sample that can be quantitatively determined with stated, acceptable precision and accuracy under stated analytical conditions (*i.e.*, the lower limit of quantitation). Therefore, acceptable quality control and quality assurance procedures are calibrated to the MRL, or lower. To take into account day-to-day fluctuations in instrument sensitivity, analyst performance, and other factors, the MRL is established at three times the Method Detection Limit (or greater). The Method Detection Limit is the minimum concentration of a substance that can be measured and reported with 99% confidence that the analyte concentration is greater than zero. (40 Code of Federal Regulations Section136 Appendix B).

<sup>&</sup>lt;sup>9</sup> This phenomenon is common in the implementation of the Ocean Plan where for some constituents, suitable analytical methods are not capable of measuring low enough to quantify the minimum toxicologically relevant concentrations. For these constituents, a discharge is considered compliant if the monitoring results are less than the MRL.

fractions). Of the constituents analyzed with this split phase method, all were detected 100% in the dissolved phase, except PCBs, which were detected 99% in the dissolved phase.

- CalAm Watershed Sanitary Survey monitoring program monthly test slant well sampling water quality results (May 2015 February 2016). 12
- Quarterly sampling of the test slant well for constituents specified in the Ocean Plan (November 2015 and February 2016).
- Test slant well sampling by Geoscience Support Services, Inc. ("Geoscience") every other month for polychlorinated biphenyls (PCBs) (May 2015 February 2016). 11

The maximum value observed in any of the data sources was assumed to be the "worst-case" water quality for the raw seawater feeding the desalination facility. If a constituent was ND in all samples, and multiple analysis methods were used with varying MRL values, the highest MRL was assumed for compliance analysis; the exception to this statement is when data was available from the low detection limit 7-day composite sample. As for the secondary effluent water quality, if the sample results of a constituent reported the concentration as less than the MRL, the MRL was assumed for compliance analysis and the concentration is reported as ND( $\leq$ MRL) in this TM. Equation 1 was used to calculate a conservative estimate of the Desal Brine concentration ( $C_{Brine}$ ) for each constituent by using a concentration factor of 1.73, which was calculated assuming complete rejection of the constituent in the feed water ( $C_{Feed}$ ) and a 42 percent recovery ( $\binom{0}{NR}$ ) through the seawater RO membranes.

$$C_{Brine} = \frac{C_{Feed}}{1 - \%_{R}} \tag{1}$$

The original Technical Memorandum (TM) (Trussell Technologies, 2015b) noted that no data were available for several Ocean Plan constituents. For constituents that lacked Desal Brine data, a concentration of zero was assumed for the previous analysis, such that the partial influence of the other discharge streams could still be assessed. Thus, a complete "worst-case" assessment for these constituents was not previously possible. The updated analysis discussed in this TM includes data for all of the constituents where no data were previously available, except for toxicity, which will be discussed in Section 2.2.

#### 2.1.3 Combined Ocean Discharge Concentrations

Having estimated the worst-case concentrations for each of the discharge components, the combined concentration prior to discharge was determined as a flow-weighted average of the contributions of each of the discharge components appropriate for the MPWSP and Variant.

<sup>&</sup>lt;sup>10</sup> Only method detection limits were provided for these results. When a constituent was ND in this dataset, the method detection limit was used for analysis.

<sup>&</sup>lt;sup>11</sup> Hexachlorobutadiene, hexachlorobenzene, HCH, heptachlor, Aldrin, chlordane, DDT, heptachlor epoxide, dieldrin, Endrin, endosulfans, toxaphene, PCBs

<sup>&</sup>lt;sup>12</sup> The well was shut down on June 5, 2015 to assess regional trends in aquifer water levels and resumed pumping October 27, 2015. The well was shut down again between March 4, 2016 and May 2, 2016 for discharge line repairs. No water quality data were collected during shutdown periods.

#### 2.2 Ocean Modeling Methodology

In order to determine Ocean Plan compliance, Trussell Tech used the following information: (1) the in-pipe (*i.e.*, pre-ocean dilution) concentration of a constituent ( $C_{in-pipe}$ ) that was developed as discussed in the previous section, (2) the minimum probable dilution for the ocean mixing ( $D_m$ ) for the discharge flow scenarios that were modeled by Dr. Roberts<sup>13</sup> (Roberts, P. J. W, 2016), and (3) the background concentration of the constituent in the ocean ( $C_{Background}$ ) that is specified in Table 3 of the Ocean Plan (SWRCB, 2012). With this information, the concentration at the edge of the zone of initial dilution ( $C_{ZID}$ ) was calculated using the following equation:

$$C_{\text{ZID}} = \frac{C_{\text{In-pipe}} + D_{\text{m}} * C_{\text{Background}}}{1 + D_{\text{m}}}$$
 (2)

The  $C_{ZID}$  was then compared to the Ocean Plan water quality objectives <sup>14</sup> in Table 1 of the Ocean Plan (SWRCB, 2012). In this table, there are three categories of objectives: (1) Objectives for Protection of Marine Aquatic Life, (2) Objectives for Protection of Human Health – Non-Carcinogens, and (3) Objectives for Protection of Human Health – Carcinogens. There are three objectives for each constituent included in the first category (for marine aquatic life): six-month median, daily maximum and instantaneous maximum concentration. For the other two categories, there is one objective: 30-day average concentration. When a constituent had three objectives, the lowest objective, the six-month median, was used to estimate compliance. This approach was taken because the discharge scenarios, discussed in further detail below, could be experienced for six months, and therefore the 6-month median objective would need to be met. For the ammonia objectives (specifically, the total ammonia concentration calculated as the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>), expressed in  $\mu g/L$  as N) the daily maximum and 6-month median objectives were evaluated.

For each discharge scenario, if the C<sub>ZID</sub> was below the Ocean Plan objective, then it was assumed that the discharge would comply with the Ocean Plan. However, if the C<sub>ZID</sub> exceeds the Ocean Plan objective, then it was concluded that the discharge scenario could violate the Ocean Plan objective. Note that this approach could not be applied for some constituents, *viz.*, acute toxicity, chronic toxicity, and radioactivity. Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based on the nature of the constituents. These constituents were measured individually for the secondary effluent and GWR Concentrate, and these individual concentrations would comply with the Ocean Plan

 $<sup>^{13}</sup>$  The Ocean Plan defines  $D_m$  differently than Dr. Roberts. A value of 1 must be subtracted from the dilution estimates provided by Dr. Roberts prior to using Equation 1.

<sup>&</sup>lt;sup>14</sup> Note that the Ocean Plan also defines effluent limitations for oil and grease, suspended solids, settleable solids, turbidity, and pH (see Ocean Plan Table 2). These parameters were not evaluated in this assessment. It is assumed that, if necessary, the pH of the water would be adjusted to be within acceptable limits prior to discharge. Oil and grease, suspended solids, settable solids, and turbidity in the GWR Concentrate and Desal Brine would be significantly lower than the secondary effluent. Prior to the AWT Facility RO treatment process, the process flow would be treated by MF, which will reduce these parameters, and the waste stream from the MF will be returned to RTP headworks. Prior to the Desalination Facility RO treatment process, the process flow would be treated by granular media filters and cartridge filters, which reduce these parameters. The waste stream from the granular media filter would be further treated in gravity thickening basins prior to any discharge of the decant through the ocean outfall. The cartridge filters will be disposed off-site and the solids will not be returned to the process.

objectives. Toxicity testing on the seawater was not included in the analysis for this TM; it will be evaluated by another method not discussed in this TM.

Dr. Roberts performed modeling of 16 discharge scenarios for the MPWSP and Variant that include combinations of Desal Brine, secondary effluent, GWR Concentrate, and hauled brine (Roberts, P. J. W, 2016). All scenarios assume the maximum flow rates for the GWR Concentrate, Desal Brine and hauled brine, which is a conservative assumption in terms of constituent loading and minimum dilution.

#### 2.2.1 Ocean Modeling Scenarios

The modeled scenarios are summarized in Tables 2 and 3 for the MPWSP and the Variant, respectively. The baseline MPWSP discharge scenario in Table 2 that has no Desal Brine (*i.e.* Scenario 1) is shown for completeness, but will not be analyzed in this TM as this flow scenario would fall under MRWPCA's existing NPDES permit, for which a D<sub>m</sub> value is already established. The Variant discharge scenarios that have no Desal Brine (*i.e.* Scenarios 11 through 15) have already been analyzed and found to comply with the Ocean Plan (Trussell Tech 2015, see Appendix B); these scenarios are shown in Table 3 for completeness, but for simplicity, the analysis of these scenarios is not repeated in Section 3.

	D: 1 0 :	D	ischarge Flows (mg	d)
No.	Discharge Scenario	Secondary Effluent	Desal Brine	Hauled Brine <sup>a</sup>
1	Baseline - high secondary effluent b	19.78	0	0.1
2	Desal Brine with no secondary effluent	0	13.98	0.1
3	Desal Brine with low secondary effluent	1	13.98	0.1
4	Desal Brine with low secondary effluent	2	13.98	0.1
5	Desal Brine with moderate secondary effluent	9	13.98	0.1
6	Desal Brine with high secondary effluent b	19.78	13.98	0.1

Table 2 - Modeled flow scenarios for the MPWSP

#### **MPWSP Flow Scenarios:**

- (1) **Baseline high secondary effluent:** The baseline flow scenario with no Desal Brine. This scenario represents times when the desalination facility is offline, the demand for recycled water is lowest (*e.g.*, during winter months), and the SVRP is not operational.
- (2) **Desal Brine with no secondary effluent:** The maximum influence of the Desal Brine on the overall discharge (*i.e.*, no secondary effluent discharged). This scenario would be representative of conditions when demand for recycled water is highest (*e.g.*,

 $<sup>^{\</sup>overline{a}}$  Hauled brine was not included in the modeling of MPWSP flow scenarios; however, the change in both flow and TDS from the addition of hauled brine is less then 1% and thus is expected to have a negligible impact on the modeled  $D_m$ .

<sup>&</sup>lt;sup>b</sup> Note that RTP wastewater flows have been declining in recent years as a result of water conservation; while 19.78 mgd is higher than current RTP wastewater flows, this is expected to be a conservative scenario with respect to ocean modeling, compared to using the current wastewater flows of 16 to 18 mgd.

- during summer months), and all of the RTP secondary effluent is recycled through the SVRP for agricultural irrigation.
- (3-4) **Desal Brine with low secondary effluent:** Desal Brine discharged with a relatively low amount of secondary effluent, resulting in a negatively buoyant plume. This scenario represents times when demand for recycled water is high, but there is excess secondary effluent that is discharged to the ocean.
  - (5) **Desal Brine with moderate secondary effluent:** Desal Brine discharged with a relatively moderate secondary effluent flow that results in a plume with slightly negative buoyancy. This scenario would be representative of conditions when demand for recycled water is low, and there is excess secondary effluent that is discharged to the ocean.
  - (6) **Desal Brine with high secondary effluent:** Desal Brine discharged with a relatively high amount of secondary effluent, resulting in a positively buoyant plume. This scenario would be representative of conditions when demand for recycled water is lowest (*e.g.*, during winter months), and the SVRP is not operational.



Discharge Flows (mgd) No. **Discharge Scenario** Hauled **Secondary Effluent Desal Brine** Concentrate Brine a 0 0 Desal Brine only 8.99 0.1 2 Desal Brine with low secondary effluent 1 8.99 0 0.1 3 Desal Brine with low secondary effluent 2 8.99 0 0.1 Desal Brine with moderate secondary 4 5.8 8.99 0 0.1 effluent 8.99 5 Desal Brine with high secondary effluent b 19.78 0.1 Desal Brine with GWR Concentrate and no 6 0 0.94 8.99 0.1 secondary effluent Desal Brine with GWR Concentrate and 7 1 8.99 0.94 0.1 low secondary effluent Desal Brine with GWR Concentrate and 8 3 0.94 8.99 0.1 low secondary effluent Desal Brine with GWR Concentrate and 9 5.3 8.99 0.94 0.1 moderate secondary effluent Desal Brine with GWR Concentrate and 15.92 8.99 10 0.94 0.1 high secondary effluent

Table 3 - Modeled flow scenarios for the Variant

24.7

23.7

0

0.4

3

0

0

0

0

0

0.94

0.94

0.94

0.94

0.94

0.1

0.1

0.1

0.1

0.1

#### Variant Flow Scenarios:

RTP design capacity with GWR

current port configuration c

GWR Concentrate c

oceanic conditions c

GWR concentrate c

RTP capacity with GWR Concentrate with

Minimum secondary effluent flow with

Minimum secondary effluent flow with GWR Concentrate during Davidson

Moderate secondary effluent flow with

Concentrate c

11

12

13

14

15

(1) **Desal Brine only:** Desal Brine discharged without secondary effluent or GWR Concentrate. This scenario would be representative of conditions when the smaller (6.4 mgd) desalination facility is in operation, but the AWT Facility is not operating

 $<sup>^{\</sup>overline{a}}$  Hauled brine was not included in the modeling of Variant scenarios involving discharge of desalination brine. However, the change in both flow and TDS from the addition of hauled brine is less than 1% and thus is expected to have a negligible impact on the modeled  $D_m$ .

<sup>&</sup>lt;sup>b</sup> Note that RTP wastewater flows have been declining in recent years as a result of conservation; while 19.68 mgd is higher than current RTP wastewater flows, this is expected to be a conservative scenario with respect to ocean modeling, compared to using the current wastewater flows of 16 to 18 mgd.

<sup>&</sup>lt;sup>c</sup> Scenarios 11 through 15 were analyzed as part of a previous analysis (see Appendix B), and based on the documented assumptions, the GWR Concentrate would comply with the Ocean Plan objectives; therefore, these scenarios are not discussed further in this memorandum.

- (e.g., offline for maintenance), and all of the secondary effluent is recycled through the SVRP (e.g., during high irrigation water demand summer months).
- (2-3) **Desal Brine with low secondary effluent:** Desal Brine discharged with low secondary effluent flow, but no GWR Concentrate, which results in a negatively buoyant plume. This scenario would be representative of times when the smaller desalination facility is in operation, but the AWT Facility is not operating (*e.g.* offline for maintenance), and most of the secondary effluent is recycled through the SVRP (*e.g.*, during high irrigation water demand summer months).
  - (4) **Desal Brine with moderate secondary effluent:** Desal Brine discharged with a relatively moderate flow of secondary effluent, but no GWR concentrate, which results in a plume with slightly negative buoyancy. This scenario represents times when demand for recycled water is low (*e.g.*, during winter months), and the AWT Facility is not operating.
  - (5) **Desal Brine with high secondary effluent:** Desal Brine discharged with a relatively high flow of secondary effluent, but no GWR concentrate, resulting in a positively buoyant plume. This scenario would be representative of conditions when demand for recycled water is lowest (*e.g.*, during winter months), and neither the SVRP nor the AWT Facility are operational.
  - (6) **Desal Brine with GWR Concentrate and no secondary effluent:** Desal Brine discharged with GWR Concentrate and no secondary effluent. This scenario would be representative of the condition where both the desalination facility and the AWT Facility are in operation, and there is the highest demand for recycled water through the SVRP (*e.g.*, during summer months).
- (7-8) **Desal Brine with GWR Concentrate and low secondary effluent:** Desal Brine discharged with low secondary effluent flow and GWR Concentrate, which results in a negatively buoyant plume. This scenario would be representative of times when both the desalination facility and the AWT Facility are in operation, and most of the secondary effluent is recycled through the SVRP (*e.g.*, during high irrigation water demand summer months).
  - (9) **Desal Brine with GWR Concentrate and moderate secondary effluent:** Desal Brine discharged with GWR Concentrate and a relatively moderate secondary effluent flow that results in a plume with slightly negative buoyancy. This scenario represents times when both the desalination facility and the AWT Facility are operating, but demand for recycled water is low and there is excess secondary effluent discharged to the ocean.
- (10) **Desal Brine with GWR Concentrate and high secondary effluent:** Desal Brine discharged with GWR Concentrate and a relatively high flow of secondary effluent. The reduction of secondary effluent flow between Scenario 5 and this scenario is a result of the AWT Facility operation. This would be a typical discharge scenario when there is no demand for tertiary recycled water (*e.g.*, during winter months).
- (11-15) **Variant conditions with no Desal Brine contribution**: These scenarios represent a range of conditions that would exist when the CalAm desalination facilities were offline for any reason. These conditions were previously evaluated (Trussell Tech, 2015) and thus are not discussed further in this technical memorandum.

#### 2.2.2 Ocean Modeling Assumptions

Dr. Roberts documented the modeling assumptions and results in a technical memorandum (Roberts, P. J. W., 2016). The modeling assumptions were specific to ambient oceanic conditions: Davidson (November to March), Upwelling (April to August), and Oceanic (September to October). In order to conservatively demonstrate Ocean Plan compliance, the lowest  $D_m$  from the applicable ocean conditions was used for each flow scenario. For all scenarios, the ocean modeling was performed assuming all 129 operational diffuser ports were open.

Three methods were used when modeling the ocean mixing: (1) the Cederwall formula (for neutral and negatively buoyant plumes only), (2) the mathematical model  $UM_3$  in the United States Environmental Protection Agency's (EPA's) Visual Plume suite, and (3) the NRFIELD model (for positively buoyant plumes only), also from the EPA's Visual Plume suite (Roberts, P. J. W., 2016). When results were provided from multiple methods, the minimum predicted  $D_m$  value was used in this analysis as a conservative approach.

# 3 Ocean Plan Compliance Results

# 3.1 Water Quality of Combined Discharge

As described above, the first step in the Ocean Plan compliance analysis was to estimate the worst-case water quality for the future wastewater discharge components (*viz.*, Desal Brine, secondary effluent, hauled brine and GWR Concentrate). The estimated water quality for each type of discharge is provided in Table 4. The Desal Brine water quality previously assumed in Trussell Technologies, 2015b is also included in Table 4 for reference ("Previous Desal Brine"); only the updated Desal Brine water quality was used in this analysis ("Updated Desal Brine"). Specific assumptions and data sources for each constituent are documented in the Table 4 footnotes.

							_		
		Updated	Previous	Seconda	ry Effluent	Hauled	d Brine	GWR	
Constituent	Units	Desal Brine	Desal Brine	MPWSP	Variant	MPWSP	Variant	Concentrate	Footnotes
Objectives for pro	otectior	of marine	aquatic life	– 6-month	median limit	f			
Arsenic	μg/L	17.2	37.9	45	45	45	45	12	2,6,16,21
Cadmium	μg/L	5.0	7.9	1	1.2	1	1.2	6.4	1,7,15,21
Chromium (Hexavalent)	μg/L	ND(<0.03)	-	ND(<2)	2.7	130	130	14	3,7,15,21
Copper	μg/L	0.5	3.07	10	10.5	39	39	55	1,7,15,21,28
Lead	μg/L	ND(<0.5)	6.4	ND(<0.5)	0.82	0.76	0.82	4.3	1,3,7,15,21
Mercury	μg/L	0.414	ND(<0.3)	0.019	0.089	0.044	0.089	0.510	1,10,16,21
Nickel	μg/L	11.0	ND(<8.6)	5.2	13.1	5.2	13.1	69	1,7,15,21
Selenium	μg/L	ND(<0.09)	55.2	3	6.5	75	75	34	2,7,15,21
Silver	μg/L	0.50	0.064	ND(<0.19)	ND(<1.59)	ND(<0.19)	ND(<1.59)	ND(<0.19)	3,9,18,21
Zinc	μg/L	9.5	ND(<35)	20	48.4	20	48.4	255	1,7,15,21
Cyanide (MBAS data)	μg/L			81	89.5	81	89.5	143	1,7,16,20
Cyanide	μg/L	ND(<8.6)	ND(<8.6)	7.2	7.2	46	46	38	1,11,15,20,21
Total Chlorine Residual	μg/L		ND(<200)	ND(<200)	ND(<200)	ND(<200)	ND(<200)	ND(<200)	5
Ammonia (as N) 6-mo median	μg/L	143.1	ND(<86.2)	36,400	36,400	36,400	36,400	191,579	1,6,15,21,27

Table 4 – Estimated worst-case water quality for the various discharge waters

<sup>1 4</sup> 

<sup>&</sup>lt;sup>15</sup> Note that these ranges assign the transitional months to the ocean condition that is typically more restrictive at relevant discharge flows.

		Updated		Seconda	ry Effluent	Hauled	d Brine		
Constituent	Units	Desal	Previous	ĺ	l T			GWR	Footnotes
		Brine	Desal Brine	MPWSP	Variant	MPWSP	Variant	Concentrate	
Ammonia (as N) daily max	μg/L	143.1	ND(<86.2)	49,000	49,000	49,000	49,000	257,895	1,6,15,21,27
Acute Toxicity	TUa		_	2.3	2.3	2.3	2.3	0.77	1,12,16,17,24
Chronic Toxicity	TUc		_	40	40	80	40	100	1,12,16,17,24 1,6,14,15,23,25
Phenolic Compounds (non-chlorinated)	μg/L	ND(<86.2)	_	69	69	69	69	363	26
Chlorinated Phenolics	μg/L	ND(<34.5)	_	ND(<20)	ND(<20)	ND(<20)	ND(<20)	ND(<20)	3,9,18,23,25,26
Endosulfan	μg/L	ND(<3.4E-6)	6.7E-05	0.015	0.048	0.015	0.048	0.25	1,10,14,15,22,25
Endrin	μg/L	ND(<1.6E-6)	2.8E-05	0.000079	0.000079	0.000079	0.000079	0.00042	4,8,15,22
HCH (Hexachlorocyclohexane)	μg/L	0.000043	0.00068	0.034	0.060	0.034	0.060	0.314	1,15,22,25
Radioactivity (Gross Beta)	pCi/L	ND(<5.17)	_	32	32	307	307	34.8	1,6,12,16,17,23
Radioactivity (Gross Alpha)	pCi/L	22.4	_	18	18	457	457	14.4	1,6,12,16,17,23
Objectives for pr			health – noi						
Acrolein	μg/L	ND(<3.4)	-	ND(<5)	9.0	ND(<5)	9.0	47	3,7,15,23
Antimony  Dia (2 ablareathers) methons	μg/L μg/L	0.19 ND(<16.7)	16.6	0.65 ND(<0.5)	0.79 ND(<4.2)	0.65 ND(<0.5)	0.79 ND(<4.2)	4.1 ND(<1)	1,6,15,21 3,9,18,23
Bis (2-chloroethoxy) methane Bis (2-chloroisopropyl) ether	μg/L μg/L	ND(<16.7)	_	ND(<0.5)	ND(<4.2) ND(<4.2)	ND(<0.5)	ND(<4.2)	ND(<1) ND(<1)	3,9,18,23
Chlorobenzene	μg/L	ND(<0.9)	_	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
Chromium (III)	µg/L	17	106.9	3.0	7.3	87	87	38	2,6,15,21
Di-n-butyl phthalate	μg/L	ND(<16.7)	_	ND(<5)	ND(<7)	ND(<5)	ND(<7)	ND(<1)	3,9,18,23
Dichlorobenzenes	μg/L	ND(<0.9)	_	1.6	1.6	1.6	1.6	8	1,6,15,21
Diethyl phthalate	μg/L	ND(<0.9)	_	ND(<5)	ND(<5)	ND(<5)	ND(<5)	ND(<1)	3,9,18,23
Dimethyl phthalate	μg/L	ND(<0.9)	_	ND(<2)	ND(<2)	ND(<2)	ND(<2)	ND(<0.5)	3,9,18,23
4,6-dinitro-2-methylphenol	μg/L	ND(<84.5)	_	ND(<0.5)	ND(<20)	ND(<0.5)	ND(<20)	ND(<5)	3,9,18,23
2,4-dinitrophenol	μg/L	ND(<86.2)	_	ND(<0.5)	ND(<13) ND(<0.5)	ND(<0.5) ND(<0.5)	ND(<13)	ND(<5)	3,9,18,23
Ethylbenzene Fluoranthene	μg/L μg/L	ND(<0.9) ND(<0.2)	0.0019	ND(<0.5) 0.00654	0.00654	0.00654	ND(<0.5) 0.00654	ND(<0.5) 0.03442	3,9,18,21 4,9,18,23
Hexachlorocyclopentadiene	μg/L	ND(<0.2)	0.0019	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.05)	3,9,18,23
Nitrobenzene	μg/L	ND(<41.4)	_	ND(<0.5)	ND(<2.3)	ND(<0.5)	ND(<2.3)	ND(<1)	3,9,18,23
Thallium	μg/L	ND(<0.1)	ND(<1.7)	ND(<0.5)	0.69	ND(<0.5)	0.69	3.7	3,7,15,21
Toluene	μg/L	ND(<0.9)	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
Tributyltin	μg/L	ND(<0.08)	_	ND(<0.05)	ND(<0.05)	ND(<0.05)	ND(<0.05)	ND(<0.02)	3,13,18,23
1,1,1-trichloroethane	μg/L	ND(<0.9)		ND(<0.5)	ND(<0.05) ND(<0.5)	ND(<0.5)		ND(<0.02) ND(<0.5)	
1,1,1-trichloroethane Objectives for pr	μg/L otection	ND(<0.9) of human	health – car	ND(<0.5) cinogens -	ND(<0.05) ND(<0.5) - 30-day ave	ND(<0.5) rage limit	ND(<0.05) ND(<0.5)	ND(<0.5)	3,13,18,23 3,9,18,21
1,1,1-trichloroethane Objectives for pr Acrylonitrile	μg/L otection μg/L	ND(<0.9) of human ND(<3.4)	health – car –	ND(<0.5) cinogens - ND(<2)	ND(<0.05) ND(<0.5) - 30-day avel 2.5	ND(<0.5) rage limit ND(<2)	ND(<0.05) ND(<0.5)	ND(<0.5)	3,13,18,23 3,9,18,21 3,7,15,23
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin	μg/L otection μg/L μg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5)	health – car –	ND(<0.5) cinogens - ND(<2) ND(<0.005)	ND(<0.05) ND(<0.5) - 30-day ave 2.5 ND(<0.007)	ND(<0.5) rage limit ND(<2) ND(<0.005)	ND(<0.05) ND(<0.5) 2.5 ND(<0.007)	ND(<0.5) 13 ND(<0.01)	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene	μg/L otection μg/L μg/L μg/L	ND(<0.9) of human ND(<3.4)	health – car –	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5)	ND(<0.05) ND(<0.5) - 30-day ave 2.5 ND(<0.007) ND(<0.5)	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5)	ND(<0.05) ND(<0.5) 2.5 ND(<0.007) ND(<0.5)	ND(<0.5) 13 ND(<0.01) ND(<0.5)	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin	μg/L otection μg/L μg/L μg/L μg/L μg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9)	health – car –	ND(<0.5) cinogens - ND(<2) ND(<0.005)	ND(<0.05) ND(<0.5) - 30-day ave 2.5 ND(<0.007)	ND(<0.5) rage limit ND(<2) ND(<0.005)	ND(<0.05) ND(<0.5) 2.5 ND(<0.007)	ND(<0.5) 13 ND(<0.01)	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23
1,1,1-trichloroethane  Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether	μg/L otection μg/L μg/L μg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2)	health - can   -   ND(<0.9)   -   ND(<1.7)	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5) ND(<0.5)	ND(<0.05) ND(<0.5) - 30-day aver 2.5 ND(<0.007) ND(<0.5) ND(<19.8)	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5)	ND(<0.05) ND(<0.5) 2.5 ND(<0.007) ND(<0.5) ND(<19.8)	ND(<0.5) 13 ND(<0.01) ND(<0.5) ND(<0.05)	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate	µg/L pg/L µg/L µg/L µg/L µg/L µg/L µg/L µg/L µ	ND(<0.9) nof human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0)	health - car   -   ND(<0.9)   -   ND(<1.7)   -   ND(<1.0)	ND(<0.5) cinogens- ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 78	ND(<0.05) ND(<0.5) -30-day ave 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<0.69) ND(<4.2) 78	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5)	ND(<0.05) ND(<0.5) 2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.05)  ND(<0.05)  ND(<1)  411	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride	µg/L pg/L µg/L µg/L µg/L µg/L µg/L µg/L µg/L µ	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9)	ND(<0.9) - ND(<1.7) - ND(<1.0) ND(<0.5)	ND(<0.5) cinogens- ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5)	ND(<0.05) ND(<0.5) -30-day ave 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<0.69) ND(<4.2) 78 0.50	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5)	ND(<0.05) ND(<0.5) 2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.05)  ND(<0.05)  ND(<1)  411  2.66	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlordane	pg/L pg/L pg/L pg/L pg/L pg/L pg/L pg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5	health - car   -   ND(<0.9)   -   ND(<1.7)   -   ND(<1.0)	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.00068	ND(<0.05) ND(<0.5) -30-day ave 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<0.69) ND(<4.2) 78 0.50 0.00068	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.0068	ND(<0.05) ND(<0.5) 2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.05)  ND(<0.05)  ND(<1)  411  2.66  0.0036	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane	µg/L otection µg/L µg/L µg/L µg/L µg/L µg/L µg/L µg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9)	ND(<0.9) - ND(<1.7) - ND(<1.0) ND(<0.5) 0.0002	ND(<0.5) cinogens ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 0.00068 ND(<0.5)	ND(<0.05) ND(<0.5) -30-day ave 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<0.69) ND(<4.2) 78 0.50 0.00068 2.4	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) O.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5)	ND(<0.05) ND(<0.5) 2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.05)  ND(<0.05)  ND(<1)  411  2.66  0.0036  13	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane Chloroform	µg/L  pg/L	ND(<0.9) nof human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9)	ND(<0.9) - ND(<1.7) - ND(<1.0) ND(<0.5) 0.0002 -	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5)	ND(<0.05) ND(<0.5) -30-day ave 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<4.9) 78 0.50 0.00068 2.4 39	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5)	ND(<0.05) ND(<0.5) 2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.05)  ND(<0.05)  ND(<1)  411  2.66  0.0036  13  204	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 2,7,15,21
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane Chloroform DDT	µg/L   µg/L	ND(<0.9) nof human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6	ND(<0.9) - ND(<1.7) - ND(<1.0) ND(<0.5) 0.0002 - 0.00055	ND(<0.5) cinogens ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 2 0.00068	ND(<0.05) ND(<0.5) - 30-day ave 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) 78 0.50 0.00068 2.4 39 0.0001	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5)	ND(<0.05) ND(<0.5) 2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.05)  ND(<0.05)  ND(<1)  411  2.66  0.0036  13	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 2,7,15,21 4,7,14,19,22,25
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane Chloroform	µg/L  pg/L	ND(<0.9) nof human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9)	ND(<0.9) - ND(<1.7) - ND(<1.0) ND(<0.5) 0.0002 -	ND(<0.5) cinogens ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 2 0.00068 ND(<0.5) 2 1.6	ND(<0.05) ND(<0.5) -30-day ave 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<4.9) 78 0.50 0.00068 2.4 39	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5)	ND(<0.05) ND(<0.5) 2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.05)  ND(<0.05)  ND(<1)  411  2.66  0.0036  13  204  0.006	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 2,7,15,21 4,7,14,19,22,25 1,6,15,21
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene	µg/L   µg/L	ND(<0.9) nof human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9)	ND(<0.9) - ND(<1.7) - ND(<1.0) ND(<0.5) 0.0002 - 0.00055	ND(<0.5) cinogens ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 2 0.00068	ND(<0.05) ND(<0.5) -30-day average 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<4.2) 78 0.50 0.00068 2.4 39 0.0001 1.6	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0012 1.6	ND(<0.05) ND(<0.5) 2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.05)  ND(<0.05)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 2,7,15,21 4,7,14,19,22,25
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethylene	µg/L   µg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9) ND(<86.2) ND(<0.9) ND(<0.9) ND(<0.9) ND(<0.9) ND(<0.9)	ND(<0.9) - ND(<1.7) - ND(<1.0) ND(<0.5) 0.0002 - 0.00055 ND(<0.9) -	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) - ND(<0.5)	ND(<0.05) ND(<0.5) -30-day aver 2.5 ND(<0.07) ND(<0.5) ND(<19.8) ND(<4.2) 78 0.50 0.00068 2.4 39 0.0001 1.6 ND(<19)	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0012 1.6 ND(<0.025) ND(<0.5) ND(<0.5)	ND(<0.05) ND(<0.5) 2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<19) ND(<0.5)	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4  ND(<2)	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 4,7,14,19,22,25 1,6,15,21 3,9,18,23 3,9,18,23 3,9,18,21 3,9,18,21
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethylene Dichlorobromomethane	pg/L   pg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9) ND(<86.2) ND(<0.9) ND(<86.2) ND(<0.9) ND(<0.9) ND(<0.9) ND(<0.9) ND(<0.9) ND(<0.9) ND(<0.9) ND(<0.9)	ND(<0.9) - ND(<1.0) ND(<1.0) ND(<0.5) 0.0002 - 0.00055 ND(<0.9) ND(<0.9) ND(<0.9) ND(<0.9) -	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.0068 ND(<0.5) 2 0.0001 1.6 ND(<0.025) ND(<0.5) ND(<0.5) ND(<0.5)	ND(<0.05) ND(<0.5) -30-day aver 2.5 ND(<0.07) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) 78 0.50 0.00068 2.4 39 0.0001 1.6 ND(<19) ND(<19) ND(<0.5) ND(<0.5)	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0012 1.6 ND(<0.025) ND(<0.5) ND(<0.5) ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5)  2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<0.5) 0.5 2.6	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4  ND(<2)  ND(<0.5)  ND(<0.5)	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 2,7,15,21 2,7,15,21 4,7,14,19,22,25 1,6,15,21 3,9,18,23 3,9,18,23 3,9,18,21 3,9,18,21 3,9,18,21 3,7,15,21
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethylene Dichlorobromomethane Dichlorobromomethane	µg/L   µg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9) ND(<86.2) ND(<0.9) ND(<86.2) ND(<0.9)	ND(<0.9)  ND(<1.0)  ND(<1.0)  ND(<1.0)  ND(<0.5)  0.0002  - 0.00055  ND(<0.9)  ND(<0.9)  ND(<0.9)  ND(<0.9)	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0001 1.6 ND(<0.025) ND(<0.5) ND(<0.5) ND(<0.5) 0.55	ND(<0.05) ND(<0.5) ND(<0.5) -30-day aver 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) 78 0.50 0.00068 2.4 39 0.0001 1.6 ND(<19) ND(<0.5) ND(<0.5) ND(<0.5)	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0012 1.6 ND(<0.025) ND(<0.5) ND(<0.5) 0.5 ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5)  2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<0.5) 0.5 2.6 0.64	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.5)  ND(<0.05)  ND(<0.5)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4  ND(<2)  ND(<0.5)  ND(<0.5)  ND(<0.5)  14  3.4	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 2,7,15,21 4,7,14,19,22,25 1,6,15,21 3,9,18,23 3,9,18,23 3,9,18,21 3,9,18,21 3,7,15,21 1,7,15,21
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethane 1,1-dichloroethylene Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane	pg/L   pg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9) ND(<6.9) ND(<6.9) ND(<0.9)	ND(<0.9)	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.005) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0001 1.6 ND(<0.025) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5) -30-day aver 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) 78 0.50 0.00068 2.4 39 0.0001 1.6 ND(<19) ND(<0.5) ND(<0.5) ND(<0.5)	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0012 1.6 ND(<0.025) ND(<0.5) 0.5 ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5)  2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<0.5) 0.5 2.6 0.64 0.56	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4  ND(<2)  ND(<0.5)  ND(<0.5)  ND(<0.5)  14  3.4  3.0	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,23 3,9,18,23 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 2,7,15,21 4,7,14,19,22,25 1,6,15,21 3,9,18,23 3,9,18,21 3,9,18,21 3,9,18,21 3,7,15,21 1,7,15,21 3,7,15,21 3,7,15,21
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlordane Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethane 1,1-dichloroethylene Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane Dichloromethane 1,3-dichloropropene Dieldrin	µg/L   µg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9) ND(<86.2) ND(<0.9)	ND(<0.9)  ND(<1.0)  ND(<1.0)  ND(<1.0)  ND(<0.5)  0.0002  - 0.00055  ND(<0.9)  ND(<0.9)  ND(<0.9)  ND(<0.9)	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.05) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0001 1.6 ND(<0.05) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 0.0001 0.0001	ND(<0.05) ND(<0.5) ND(<0.5) -30-day aver 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) 78 0.50 0.00068 2.4 39 0.0001 1.6 ND(<19) ND(<0.5) ND(<0.5) ND(<0.5) -2.6 0.64 0.56 0.0001	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.00112 1.6 ND(<0.025) ND(<0.5) 0.5 ND(<0.5) 0.5 ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5)  2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<0.5) 0.5 2.6 0.64 0.56 0.0006	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4  ND(<2)  ND(<0.5)  ND(<0.5)  ND(<0.5)  14  3.4  3.0  0.0033	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,7,14,19,22,25 1,6,15,21 3,9,18,23 3,9,18,21 3,9,18,21 3,9,18,21 3,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlordane Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethylene Dichlorobromomethane Dichlorobromomethane 1,1-dichloroethylene Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane 1,3-dichloropropene Dieldrin 2,4-dinitrotoluene	µg/L   µg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9) ND(<86.2) ND(<0.9)	ND(<0.9)	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0001 1.6 ND(<0.05) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 0.0001 1.6 ND(<0.025) ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5) -30-day averone 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) -78 -0.50 -0.00068 -0.0001 -0.00068 -0.0001 -0.00068 -0.0001 -0.00068 -0.0001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.00112 1.6 ND(<0.025) ND(<0.5) 0.5 ND(<0.5) 0.5 ND(<0.5) 0.5 ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.07) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<0.5) 0.5 2.6 0.64 0.56 0.0006 ND(<2)	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4  ND(<2)  ND(<0.5)  ND(<0.5)  14  3.4  3.0  0.0033  ND(<0.1)	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,7,14,19,22,25 1,6,15,21 4,7,14,19,22,25 1,6,15,21 3,9,18,23 3,9,18,21 3,9,18,21 3,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,19,22 3,9,18,23
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlordane Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethane 1,1-dichloroethylene Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane Dichlorobromomethane Dichloromethane 1,3-dichloropropene Dieldrin	pg/L   pg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9) ND(<86.2) ND(<0.9)	ND(<0.9)	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0001 1.6 ND(<0.05) ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5) -30-day aver 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) 78 0.50 0.00068 2.4 39 0.0001 1.6 ND(<19) ND(<0.5) ND(<0.5) ND(<0.5) -2.6 0.64 0.56 0.0001	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.00112 1.6 ND(<0.025) ND(<0.5) 0.5 ND(<0.5) 0.5 ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5)  2.5 ND(<0.007) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<0.5) 0.5 2.6 0.64 0.56 0.0006	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4  ND(<2)  ND(<0.5)  ND(<0.5)  ND(<0.5)  14  3.4  3.0  0.0033	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 2,7,15,21 4,7,14,19,22,25 1,6,15,21 3,9,18,23 3,9,18,21 3,7,15,21 1,7,15,21 1,7,15,21 4,7,19,22 3,9,18,23 3,9,18,23 3,9,18,23 3,9,18,23
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlordane Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethylene Dichlorobromomethane Dichlorobromomethane 1,1-dichloroethylene Dichlorobromomethane Dichlorobromomethane 1,3-dichloropropene Dieldrin 2,4-dinitrotoluene 1,2-diphenylhydrazine	µg/L   µg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9) ND(<86.2) ND(<0.9) 1.7E-6 ND(<0.9)	ND(<0.9)	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0001 1.6 ND(<0.05) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 0.0001 1.6 ND(<0.025) ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5) -30-day averone 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) -78 -0.50 -0.00068 -0.0001 -0.00068 -0.0001 -0.00068 -0.0001 -0.00068 -0.0001 -0.00001	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0012 1.6 ND(<0.025) ND(<0.5) 0.5 ND(<0.5) 0.5 ND(<0.5) 0.5 ND(<0.5) 0.55 ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.07) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<0.5) 0.5 2.6 0.64 0.56 0.0006 ND(<2) ND(<4.2) ND(<4.2)	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4  ND(<2)  ND(<0.5)  ND(<0.5)  14  3.4  3.0  0.0033  ND(<0.1)  ND(<1)	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 4,7,14,19,22,25 1,6,15,21 3,9,18,23 3,9,18,21 3,9,18,21 3,7,15,21 1,7,15,21 1,7,15,21 4,7,19,22 3,9,18,23
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethylene Dichlorobromomethane Dichlorobromomethane 1,1-dichloroethylene Dichlorobromomethane Dichlorobromomethane 1,3-dichloropropene Dieldrin 2,4-dinitrotoluene 1,2-diphenylhydrazine Halomethanes	pg/L   pg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) 1.7E-6 ND(<0.9) ND(<86.2) ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9)	ND(<0.9) - ND(<0.9) - ND(<1.0) ND(<0.5) 0.0002 - 0.00055 ND(<0.9) - ND(<0.9) ND(<0.9) ND(<0.9) - ND(<0.9) - ND(<0.9) - ND(<0.9) - ND(<0.9)	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.05) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0001 .0.6 ND(<0.05) ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5) -30-day averone 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) -78 -0.50 -0.00068 -0.0001 -0	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0012 1.6 ND(<0.025) ND(<0.5) 0.5 ND(<0.5) 0.5 ND(<0.5) 0.5 ND(<0.5) 0.73	ND(<0.05) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.07) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<0.5) 0.5 2.6 0.64 0.56 0.0006 ND(<2) ND(<2) ND(<4.2) 1.4	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4  ND(<2)  ND(<0.5)  ND(<0.5)  ND(<0.5)  14  3.4  3.0  0.0033  ND(<0.1)  ND(<1)  7.5	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 4,7,14,19,22,25 1,6,15,21 3,9,18,23 3,9,18,21 3,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 1,7,15,21 2,7,15,21 3,7,15,21 4,7,19,22 3,9,18,23 3,9,18,23 2,7,14,15,21
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane Chlorofibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethylene Dichlorobromomethane Dichlorobromomethane 1,1-dichloroethylene Dichlorobromomethane Dichlorobromomethane 1,3-dichloropropene Dieldrin 2,4-dinitrotoluene 1,2-diphenylhydrazine Halomethanes Heptachlor	pg/L   pg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9) ND(<86.2) ND(<0.9)	ND(<0.9) ND(<0.9) ND(<1.0) ND(<1.0) ND(<0.5) 0.0002 - 0.00055 ND(<0.9) ND(<0.9) ND(<0.9) ND(<0.9) ND(<0.9) ND(<0.9) ND(<0.9) 8.8E-05 8.6E-06	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0001 .0.6 ND(<0.025) ND(<0.5) ND(<0.5) ND(<0.5) .0.55 ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5) -30-day averone 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) -78 -0.50 -0.00068 -0.0001 -0	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0012 1.6 ND(<0.025) ND(<0.5) 0.5 ND(<0.5) 0.5 ND(<0.5) 0.73 ND(<0.01)	ND(<0.05) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<0.5) 0.5 2.6 0.64 0.56 0.0006 ND(<2) ND(<2) ND(<4.2) 1.4 ND(<0.01)	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4  ND(<2)  ND(<0.5)  ND(<0.5)  14  3.4  3.0  0.0033  ND(<0.1)  ND(<1)  7.5  ND(<0.01)	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,21 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 2,7,15,21 2,7,15,21 3,9,18,23 3,9,18,21 3,7,15,21 1,7,15,21 1,7,15,21 3,7,15,21 4,7,19,22 3,9,18,23 3,9,18,23 3,9,18,23 2,7,14,15,21 3,9,18,23 2,7,14,15,21 3,9,18,23 2,7,14,15,21 3,9,18,23
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlordane Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethylene Dichlorobromomethane Dichlorobromomethane 1,1-dichloroethylene Dichlorobromomethane 1,3-dichloropropene Dieldrin 2,4-dinitrotoluene 1,2-diphenylhydrazine Halomethanes Heptachlor Heptachlor epoxide	pg/L   pg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) 1.7E-6 ND(<0.9) ND(<86.2) ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9)	ND(<0.9) ND(<0.9) ND(<1.0) ND(<1.0) ND(<0.5) 0.0002 - 0.00055 ND(<0.9)	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5) - 2 0.0001 1.6 ND(<0.05) ND(<0.5) ND(<0.5) ND(<0.5) - 2 0.0001 1.6 ND(<0.05) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 0.55 ND(<0.5) 0.0001 ND(<2) ND(<0.5) 0.54 ND(<0.01) 0.000078	ND(<0.05) ND(<0.5) ND(<0.5) -30-day aver 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) 78 0.50 0.00068 2.4 39 0.0001 1.6 ND(<19) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) -2.6 0.64 0.56 0.0001 ND(<2) ND(<2) ND(<4.2) 1.4 ND(<0.01) 0.000079 0.000078	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0012 1.6 ND(<0.025) ND(<0.5) 0.5 ND(<0.5) 0.5 ND(<0.5) 0.73 ND(<0.01) 0.000079 0.000078	ND(<0.05) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<0.5) 0.5 2.6 0.64 0.56 0.0006 ND(<2) ND(<2) ND(<4.2) 1.4 ND(<0.01) 0.000079 0.000078	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4  ND(<2)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<0.1)  14  3.4  3.0  0.0033  ND(<0.1)  ND(<1)  7.5  ND(<0.1)  0.000416	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,23 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 4,7,14,19,22,25 1,6,15,21 3,9,18,23 3,9,18,21 3,7,15,21 4,7,15,21 4,7,15,21 3,7,15,21 4,7,19,22 3,9,18,23 3,9,18,23 3,9,18,21 3,7,15,21 4,7,19,22 3,9,18,23 3,9,18,23 3,9,18,23 3,9,18,23 4,7,14,15,21 4,7,19,22 3,9,18,23 3,9,18,23 2,7,14,15,21 3,9,18,23 4,8,15,22 4,8,15,22,23
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethylene Dichlorobromomethane Dichlorobromomethane 1,1-dichloroethylene Dichlorobromomethane 1,3-dichloropropene Dieldrin 2,4-dinitrotoluene 1,2-diphenylhydrazine Halomethanes Heptachlor Heptachlor epoxide Hexachlorobenzene	pg/L   pg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9)	ND(<0.9) ND(<0.9) ND(<1.0) ND(<1.0) ND(<0.5) 0.0002 - 0.00055 ND(<0.9)	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0001 1.6 ND(<0.05) ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5) -30-day averone 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) -78 -0.50 -0.00068 -0.0001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.00001 -0.000079	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.00112 1.6 ND(<0.025) ND(<0.5) 0.5 ND(<0.5) 0.5 ND(<0.5) 0.73 ND(<0.5) 0.73 ND(<0.01) 0.000079 0.000078 0.000009 ND(<0.5)	ND(<0.05) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<0.5) 0.5 2.6 0.64 0.56 0.0006 ND(<2) ND(<2) ND(<4.2) 1.4 ND(<0.01) 0.000079	ND(<0.5)  13  ND(<0.01)  ND(<0.5)  ND(<0.5)  ND(<0.05)  ND(<0.5)  ND(<0.5)  ND(<1)  411  2.66  0.0036  13  204  0.006  8.4  ND(<2)  ND(<0.5)  ND(<0.5)  ND(<0.5)  ND(<0.1)  14  3.4  3.0  0.0033  ND(<0.1)  ND(<1)  7.5  ND(<0.01)  0.000416  0.000411	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,23 3,9,18,23 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 4,7,14,19,22,25 1,6,15,21 3,9,18,23 3,9,18,21 3,9,18,21 3,7,15,21 4,7,15,21 4,7,15,21 3,7,15,21 4,7,19,22 3,9,18,23 3,9,18,23 3,9,18,23 3,9,18,23 3,9,18,23 3,9,18,23 3,9,18,23 3,9,18,23 3,9,18,23 3,9,18,23 3,9,18,23 2,7,14,15,21 3,9,18,23 4,8,15,22
1,1,1-trichloroethane Objectives for pr Acrylonitrile Aldrin Benzene Benzidine Beryllium Bis(2-chloroethyl)ether Bis(2-ethyl-hexyl)phthalate Carbon tetrachloride Chlorodibromomethane Chlorodibromomethane Chloroform DDT 1,4-dichlorobenzene 3,3-dichlorobenzidine 1,2-dichloroethylene Dichlorobromomethane Dichlorobromomethane 1,1-dichloroethylene Dichlorobromomethane 1,3-dichloropropene Dieldrin 2,4-dinitrotoluene 1,2-diphenylhydrazine Halomethanes Heptachlor Heptachlor epoxide Hexachlorobenzene Hexachlorobutadiene	pg/L   pg/L	ND(<0.9) of human ND(<3.4) ND(<6.7E-5) ND(<0.9) ND(<86.2) ND(<86.2) ND(<0.9) ND(<41.4) ND(<1.0) ND(<0.9) 1.45E-5 ND(<0.9) ND(<0.9) 1.7E-6 ND(<0.9)	ND(<0.9) ND(<0.9) ND(<1.0) ND(<1.0) ND(<0.5) 0.0002 - 0.00055 ND(<0.9)	ND(<0.5) cinogens - ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0001 1.6 ND(<0.05) ND(<0.5) 0.0001 ND(<2) ND(<0.5) 0.0001 ND(<2) ND(<0.5) 0.54 ND(<0.01) 0.000079 0.000078	ND(<0.05) ND(<0.5) ND(<0.5) -30-day aver 2.5 ND(<0.007) ND(<0.5) ND(<19.8) ND(<19.8) ND(<4.2) 78 0.50 0.00068 2.4 39 0.0001 1.6 ND(<19) ND(<0.5) ND(<0.5) 2.6 0.64 0.56 0.0001 ND(<2) ND(<2) ND(<4.2) 1.4 ND(<0.01) 0.000079 0.000078	ND(<0.5) rage limit ND(<2) ND(<0.005) ND(<0.5) ND(<0.5) ND(<0.5) 0.0052 ND(<0.5) 78 ND(<0.5) 0.00068 ND(<0.5) 2 0.0012 1.6 ND(<0.025) ND(<0.5) 0.5 ND(<0.5) 0.5 ND(<0.5) 0.73 ND(<0.01) 0.000079 0.000078 0.000009	ND(<0.05) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<19.8) 0.0052 ND(<4.2) 78 0.50 0.00068 2.4 39 0.0012 1.6 ND(<19) ND(<0.5) 0.5 2.6 0.64 0.56 0.0006 ND(<2) ND(<2) ND(<4.2) 1.4 ND(<0.01) 0.000079 0.000078 0.000009	ND(<0.5)  13 ND(<0.01) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.5) ND(<1) 411 2.66 0.0036 13 204 0.006 8.4 ND(<2) ND(<0.5) ND(<0.5) ND(<0.5) ND(<0.1) 41 3.4 3.0 0.0033 ND(<0.1) ND(<1) 7.5 ND(<0.01) 0.000416 0.000411 0.000047	3,13,18,23 3,9,18,21 3,7,15,23 3,9,18,23 3,9,18,23 3,9,18,23 3,9,17,18,21 3,9,18,23 2,6,15,23 3,7,15,21 4,8,14,15,22,25 3,7,15,21 4,7,14,19,22,25 1,6,15,21 3,9,18,23 3,9,18,21 3,7,15,21 4,7,15,21 4,7,15,21 3,7,15,21 4,7,19,22 3,9,18,23 3,9,18,21 3,7,15,21 4,7,19,22 3,9,18,23 3,9,18,23 4,7,14,15,21 3,9,18,23 4,7,14,15,21 4,7,19,22 3,9,18,23 4,7,14,15,21 4,7,19,22 3,9,18,23 4,7,14,15,21 4,7,19,22 3,9,18,23 4,7,14,15,21 4,7,19,22 4,8,15,22,23 4,8,15,22,23

		Updated	Previous	Seconda	ry Effluent	Hauled	d Brine	GWR	
Constituent	Units	Desal Brine	Desal Brine	MPWSP	Variant	MPWSP	Variant	Concentrate	Footnotes
N-Nitrosodi-N-Propylamine	μg/L	ND(<0.003)	ND(<0.003)	0.076	0.076	0.076	0.076	0.019	2,6,16,17,23
N-Nitrosodiphenylamine	μg/L	ND(<16.7)	_	ND(<0.5)	ND(<2.3)	ND(<0.5)	ND(<2.3)	ND(<1)	3,9,18,23
PAHs	μg/L	2.2E-3	0.012	0.03	0.03	0.03	0.03	0.19	4,8,14,15,22,25
PCBs	μg/L	0.00013	0.002	0.00068	0.00068	0.00068	0.00068	0.00357	4,8,14,15,22,25
TCDD Equivalents	μg/L	ND (<2.5E-5)	_	1.37E-7	1.42E-7	1.37E-7	1.42E-7	7.46E-7	4,13,14,15,23,25
1,1,2,2-tetrachloroethane	μg/L	ND(<0.9)	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
Tetrachloroethylene	μg/L	ND(<0.9)	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
Toxaphene	μg/L	3.97E-5	ND(<0.0013)	0.0071	0.0071	0.0071	0.0071	0.0373	4,8,15,22
Trichloroethylene	μg/L	ND(<0.9)	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
1,1,2-trichloroethane	μg/L	ND(<0.9)	ND(<0.9)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21
2,4,6-trichlorophenol	μg/L	ND(<16.7)	_	ND(<0.5)	ND(<2.3)	ND(<0.5)	ND(<2.3)	ND(<1)	3,9,18,23
Vinyl chloride	μg/L	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	ND(<0.5)	3,9,18,21

#### **Table 4 Footnotes:**

#### MPWSP Secondary Effluent and Hauled Brine

- <sup>1</sup> The value reported is based on MRWPCA historical data.
- <sup>2</sup> The value reported is based on secondary effluent data collected during the GWR Project source water monitoring programs (not impacted by the proposed new source waters), and are representative of future water quality under the MPWSP scenario.
- <sup>3</sup> The MRL provided represents the limit from NPDES monitoring data for secondary effluent and hauled waste. In cases where constituents had varying MRLs, in general, the lowest MRL is reported.
- <sup>4</sup> RTP effluent value presented based on CCLEAN data.

#### Total Chlorine Residual

<sup>5</sup> For all waters, it is assumed that dechlorination will be provided such that the total chlorine residual will be below detection.

#### Variant Secondary Effluent and Hauled Brine

- <sup>6</sup> Existing RTP effluent exceeds concentrations observed in other proposed source waters; the value reported is the existing secondary effluent value.
- <sup>7</sup> The proposed new source waters may increase the secondary effluent concentration; the value reported is based on predicted source water blends.
- <sup>8</sup> RTP effluent value is based on CCLEAN data; no other source waters were considered due to MRL differences.
- <sup>9</sup> MRL provided represents the maximum flow-weighted MRL based on the blend of source waters.
- <sup>10</sup> The only water with a detected concentration was the RTP effluent, however the flow-weighted concentration increases due to higher MRLs for the proposed new source waters.
- <sup>11</sup> Additional source water data are not available; the reported value is for RTP effluent.
- <sup>12</sup> Calculation of the flow-weighted concentration was not feasible due to constituent. The maximum observed value is reported.
- <sup>13</sup> Agricultural Wash Water data are based on an aerated sample, instead of a raw water sample.
- <sup>14</sup> This value in the Ocean Plan is an aggregate of several congeners or compounds. Per the approach described in the Ocean Plan, for cases where the individual congeners/compounds were less than the MRL, a value of 0 is assumed in calculating the aggregate value.

#### GWR Concentrate Data

- <sup>15</sup> The value presented represents a calculated value assuming no removal prior to RO, complete rejection through RO membrane, and an 81% RO recovery.
- <sup>16</sup> The value represents the maximum value observed during the pilot testing study.
- <sup>17</sup> The calculated value for the AWT Facility data (described in note 15) was not used in the analysis because it was not considered representative. It is expected that the value would increase as a result of treatment through the AWT Facility (*e.g.* formation of N-Nitrosodimethylamine as a disinfection by-product), or that it will not concentrate linearly through the RO (*e.g.* toxicity and radioactivity).
- <sup>18</sup> The MRL provided represents the limit from the source water and pilot testing monitoring programs.

<sup>19</sup> The value presented represents a calculated value assuming 93% and 84% removal through primary and secondary treatment for DDT and dieldrin, respectively, and 36% and 44% removal through ozone for DDT and dieldrin, respectively, complete rejection through the RO membrane, and an 81% RO recovery. The assumed removals are based on results from ozone bench-scale testing of Blanco Drain water blended with secondary effluent and low detection sampling through the RTP.

#### Cyanide Data

<sup>20</sup> In mid-2011, MBAS began performing the cyanide analysis on the RTP effluent, at which time the reported values increased by an order of magnitude. Because no operational or source water composition changes took place at this time that would result in such an increase, it is reasonable to conclude the increase is an artifact of the change in analysis method and therefore questionable. Therefore, the cyanide values as measured by MBAS are listed separately from other cyanide values, and the MBAS data were not be used in the analysis for evaluating compliance with the Ocean Plan objectives.

#### Desal Brine Data

- <sup>21</sup> The value reported is based on test slant well data collected through the Watershed Sanitary Survey.
- <sup>22</sup> The value reported is based on data from the one-time 7-day composite sample from the test slant well. If ND, the method detection limit was used for the analysis instead of the MRL. MRLs were not available for this data set.
- <sup>23</sup> The value reported is based on data from the test slant well collected through the quarterly Ocean Plan constituents monitoring.

  24 Acute and chronic toxicity have not been measured or estimated
- <sup>25</sup> This value in the Ocean Plan is an aggregate of several congeners or compounds. Per the approach described in the Ocean Plan, for cases where the individual congeners/compounds were less than the MRL, a value of 0 is assumed in calculating the aggregate value.
- <sup>26</sup> Chlorinated phenolic compounds is the sum of the following: 4-chloro-3-methylphenol, 2-chlorophenol, pentachlorophenol, 2,4,5-trichlorophenol, and 2,4,6-trichlorophenol. Non-chlorinated phenolic compounds is the sum of the following: 2,4-dimethylphenol, 4,6-Dinitro-2-methylphenol, 2,4-dinitrophenol, 2-methylphenol, 4methylphenol, 2-nitrophenol, 4-nitrophenol, and phenol.

#### General

- <sup>27</sup> Ammonia (as N) represents the total ammonia concentration, *i.e.* the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>).
- <sup>28</sup> The value reported for the Variant secondary effluent was calculated using the median of the data collected for the new source waters and is an estimate of the potential increase in concentration of the secondary effluent based on predicted source water blends. The value reported for the Desal Brine was calculated with the median of the data collected from the test slant well and assuming a 42% recovery through the RO. The median values were used because the maximum values detected in both sources appear to be outliers, and because the Ocean Plan objective is a 6-month median concentration, it is reasonable to use the median value detected from these source waters.

# 3.2 Ocean Modeling Results

The estimated minimum probable dilution (D<sub>m</sub>) for each discharge scenario is presented in Tables 5 and 6 (Roberts, P. J. W., 2016). For discharge scenarios that were modeled with more than one modeling method, the lowest  $D_m$  (i.e., most conservative) is reported in the tables below. For the MPWSP, the flow scenarios in which little or no secondary effluent was discharged (Scenarios 2, 3 and 4) resulted in the lowest D<sub>m</sub> values as a result of the discharge plume being negatively buoyant. At higher secondary effluent flows, the discharge plume would be positively buoyant, resulting in an increased D<sub>m</sub>, as evidenced in Scenario 6. The same trend was observed for Variant scenarios.

Table 5 – Flow scenarios and modeled D<sub>m</sub> values used for Ocean Plan compliance analysis for MPWSP

N	Discharge Scenario	Dis	charge flows (mg	d)	
No.	(Ocean Condition)	Secondary effluent	Desal Brine	Hauled brine <sup>a</sup>	D <sub>m</sub> <sup>b</sup>
2	Desal Brine with no secondary effluent	0	13.98	0.1	14.6
3	Desal Brine with low secondary effluent	1	13.98	0.1	15.2
4	Desal Brine with low secondary effluent	2	13.98	0.1	16.0
5	Desal Brine with moderate secondary effluent	9	13.98	0.1	34.3
6	Desal Brine with high secondary effluent °	19.78	13.98	0.1	153

 $<sup>^{</sup>a}$  Hauled brine was not included in the modeling of MPWSP flow scenarios; however, the change in both flow and TDS from the addition of hauled brine is less than 1% and thus is expected to have a negligible impact on the modeled  $D_{\rm m}$ .

<sup>&</sup>lt;sup>c</sup> Note that RTP wastewater flows have been declining in recent years as a result of conservation; while 19.68 mgd is higher than current RTP wastewater flows, this is expected to be a conservative scenario with respect to ocean modeling, compared to using the current wastewater flows of 16 to 18 mgd.



<sup>&</sup>lt;sup>b</sup> Several models were used to predict the minimal probable dilution value (UM<sub>3</sub>, Cederwall for neutral and negatively buoyant plumes, and NRFIELD for buoyant plumes). Values included here are the model results ( $D_m$  values) that resulted in the lowest  $D_m$ . A value of 1 has also been subtracted from Dr. Roberts' values to take into account the different definition of dilution/ $D_m$  provided by Dr. Roberts versus the Ocean Plan.

Table 6 – Flow scenarios and modeled D<sub>m</sub> values used for Ocean Plan compliance analysis for Variant

			Discharge FI	ows (mgd)		
No.	Discharge Scenario	Secondary Effluent	Desal Brine	GWR Concentrate	Hauled Brine <sup>a</sup>	D <sub>m</sub> <sup>b</sup>
1	Desal Brine only	0	8.99	0	0.1	14.9
2	Desal Brine with low secondary effluent	1	8.99	0	0.1	15.7
3	Desal Brine with low secondary effluent	2	8.99	0	0.1	16.7
4	Desal Brine with moderate secondary effluent	5.8	8.99	0	0.1	31.5
5	Desal Brine with high secondary effluent b	19.78	8.99	0	0.1	104
6	Desal Brine with GWR Concentrate and no secondary effluent	0	8.99	0.94	0.1	15.6
7	Desal Brine with GWR Concentrate and low secondary effluent	1	8.99	0.94	0.1	16.4
8	Desal Brine with GWR Concentrate and low secondary effluent	3	8.99	0.94	0.1	20.3
9	Desal Brine with GWR Concentrate and moderate secondary effluent	5.3	8.99	0.94	0.1	54.4
10	Desal Brine with GWR Concentrate and high secondary effluent	15.92	8.99	0.94	0.1	194

<sup>&</sup>lt;sup>a</sup> Hauled brine was not included in the modeling of Variant scenarios involving discharge of desalination brine. However, the change in both flow and TDS from the addition of hauled brine is less than 1% and thus is expected to have a negligible impact on the modeled  $D_m$ .

# 3.3 Ocean Plan Compliance Results

The flow-weighted in-pipe concentration for each constituent was calculated for each modeled discharge scenario using the water quality presented in Table 4 and the discharge flows presented in Tables 2 and 3. The in-pipe concentration was then used to calculate the concentration at the edge of the ZID using the  $D_m$  values presented in Tables 5 and 6. The resulting concentrations for each constituent in each scenario were compared to the Ocean Plan objectives to assess compliance. The estimated concentrations for the 15 flow scenarios (5 for the MPWSP and 10 for the Variant) for all constituents are presented as concentrations at the edge of the ZID (Appendix A, Table A1 and A3) and as a percentage of the Ocean Plan objective (Appendix A, Table A2 and A4).

<sup>&</sup>lt;sup>b</sup> Several models were used to predict the minimal probable dilution value (UM<sub>3</sub>, Cederwall for neutral and negatively buoyant plumes, and NRFIELD for buoyant plumes). Values included here are the model results ( $D_m$  values) that resulted in the lowest  $D_m$ . A value of 1 has also been subtracted from Dr. Roberts' values to take into account the different definition of dilution/ $D_m$  provided by Dr. Roberts versus the Ocean Plan.

It was identified that some constituents are estimated to exceed the Ocean Plan objective for some discharge scenarios. Seventeen<sup>16</sup> constituents were highlighted to potentially exceed the Ocean Plan water quality objectives; however, ten<sup>17</sup> of these constituents were never detected above the MRL in any of the source waters, and the MRLs are higher than the Ocean Plan objective.<sup>18</sup> Due to this insufficient analytical sensitivity, no compliance conclusion can be drawn for these constituents. This is a typical occurrence for ocean discharges since the MRL of the approved compliance analysis method is higher than the Ocean Plan objective for certain constituents.

Of the constituents detected in the source waters, seven were identified as having potential to exceed the Ocean Plan objective in the Variant. Within this subset, acrylonitrile, beryllium and TCDD equivalents were detected in some of the source waters, but not in the others. For these analyses, the MRLs themselves were above the Ocean Plan objective. To assess the blended concentrations for these constituents, a value of zero was assumed for any sources when the concentration was below the MRL. This approach is a "best-case" scenario because it assumes the lowest possible concentration—namely, a value of zero—for any constituent below the reporting limit. This approach is still useful, however, to bracket the analysis and assess the potential for Ocean Plan compliance issues under best-case conditions. Through this method, TCDD equivalents shows potential to exceed the Ocean Plan objective for the Variant. The predicted concentration of acrylonitrile and beryllium at the edge of the ZID is less than the Ocean Plan objective and therefore did not show exceedances through this "best-case" analysis.

A list of the constituents that may exceed the Ocean Plan are shown at their estimated concentration at the edge of the ZID in Table 7 for the MPWSP and Table 8 for the Variant, and as the concentration at the edge of the ZID as a percentage of the Ocean Plan objective in Table 9 and 10 for the MPWSP and Variant, respectively. The "best-case" scenario compliance assessment results for TCDD equivalents is also included in these tables.

<sup>&</sup>lt;sup>16</sup> Ammonia, chlorinated phenolics, 2,4-dinitrophenol, tributyltin, acrylonitrile, aldrin, benzidine, beryllium, bis(2-chloroethyl)ether, chlordane, 3,3-dichlorobenzidine, 1,2-diphenylhydrazine, heptachlor, PCBs, TCDD equivalents, toxaphene, 2,4,6-trichlorophenol

<sup>&</sup>lt;sup>17</sup> Chlorinated phenolics, 2,4-dinitrophenol, tributyltin, aldrin, benzidine, bis(2-chloroethyl)ether, 3,3-dichlorobenzidine, 1,2-diphenylhydrazine, heptachlor, 2,4,6-trichlorophenol

<sup>&</sup>lt;sup>18</sup> The exceptions to this statement are: 2,4-dinitrophenol was ND in the MPWSP Secondary Effluent, and this MRL is lower than the Ocean Plan objective (*i.e.*, MRL = 0.5 ug/L versus 4 ug/L = objective); heptachlor was not detected above the MRL in the slant well, and this MRL is lower than the Ocean Plan objective (*i.e.*, MRL = 0.000000069 ug/L).

<sup>&</sup>lt;sup>19</sup> Additionally, the Ocean Plan states that for constituents that are made up of an aggregate of constituents, a concentration of 0 can be assumed for the individual constituents that are not detected above the MRL, such as TCDD equivalents.

<sup>&</sup>lt;sup>20</sup> Acrylonitrile was only detected in one potential source water for the Variant. It was not detected in any potential source waters for the MPWSP Project; therefore, a compliance determination cannot be made for the MPWSP Project and only partial determination can be made for the Variant.

Table 7 – Predicted concentrations at the edge of the ZID for Ocean Plan constituents of concern in the MPWSP <sup>a</sup>

			Estim	ated Concentra	ation at Edge o	f ZID by Scer	nario
Constituent	Units	Ocean Plan Objective			MPWSP		
		Objective	2	3	4	5	6
Objectives for protection of marine	aquatic lif	e - 6-month m	edian limit				
Ammonia (as N) – 6-mo median <sup>b</sup>	μg/L	600	25.7	172.1	287	409.0	139.2
Objectives for protection of human	health - ca	arcinogens - 3	0-day average l	imit <sup>c d</sup>			
Chlordane	μg/L	2.3E-05	1.23E-06	3.91E-06	6.00E-06	7.89E-06	2.65E-06
PCBs	μg/L	1.9E-05	8.76E-06	1.07E-05	1.20E-05	9.86E-06	2.94E-06
TCDD Equivalents d	μg/L	3.9E-09	6.23E-11	6.17E-10	1.05E-09	1.53E-09	5.22E-10
Toxaphene e	μg/L	2.1E-04	5.75E-06	3.42E-05	5.65E-05	7.99E-05	2.71E-05

<sup>&</sup>lt;sup>a</sup> Shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.



composite sample from the test slant well.

<sup>&</sup>lt;sup>b</sup> Ammonia (as N) represents the total ammonia concentration, *i.e.* the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>).

<sup>&</sup>lt;sup>c</sup> Acrylonitrile was only detected in one potential source water for the Variant Project. It was not detected in any potential source waters for the MPWSP Project; therefore, a compliance determination cannot be made for the MPWSP Project and only partial determination can be made for the Variant Project.

d Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time. When only the detected values were considered, acrylonitrile and beryllium did not exceed the Ocean Plan objective by 80% or more and therefore were not included in Tables 7 through 10.

Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day

Table 8 – Predicted concentrations at the edge of the ZID for Ocean Plan constituents of concern in the Variant <sup>a</sup>

		Ocean			Estimat	ed Conce	entration a	at Edge o	f ZID by S	cenario		
Constituent	Units	Plan					Var	iant				
		Objective	1	2	3	4	5	6	7	8	9	10
Objectives for p	rotection of n	narine aqua	tic life -	6-month	median lii	mit						
Ammonia (as N) – 6-mo median <sup>b</sup>	μg/L	600	34	245	396	446	239	1111	1154	1060	445	151
Objectives for p	rotection of h	uman healt	h - carcin	ogens -	30-day av	erage lin	nit <sup>c</sup>					
Chlordane	μg/L	2.3E-05	1.37E-6	5.24E-6	7.98E-6	8.61E-6	4.53E-6	2.15E-5	2.22E-5	2.03E-5	8.49E-6	2.86E-6
PCBs	μg/L	1.9E-05	8.72E-6	1.15E-5	1.33E-5	1.07E-5	4.85E-6	2.77E-5	2.76E-5	2.40E-5	9.68E-6	3.05E-6
TCDD Equivalents <sup>c</sup>	μg/L	3.9E-09	9.81E-11	9.26E-10	1.52E-9	1.73E-9	9.30E-10	4.30E-9	4.47E-9	4.11E-9	1.73E-9	5.87E-10
Toxaphene d	μg/L	2.1E-04	7.37E-6	4.84E-5	7.77E-5	8.72E-5	4.66E-5	2.17E-4	2.25E-4	2.07E-4	8.68E-5	2.94E-5

<sup>&</sup>lt;sup>a</sup> Shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.

<sup>&</sup>lt;sup>b</sup> Ammonia (as N) represents the total ammonia concentration, *i.e.* the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>).

<sup>&</sup>lt;sup>c</sup> Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time. When only the detected values were considered, acrylonitrile and beryllium did not exceed the Ocean Plan objective by 80% or more and therefore were not included in Tables 7 through 10.

<sup>d</sup> Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.

composite sample from the test slant well.

Table 9 – Predicted concentrations at the edge of the ZID expressed as percentage of Ocean Plan Objective for constituents of in the MPWSP <sup>a</sup>

	Units	Ocean Plan Objective	Est. Percentage of Ocean Plan objective at Edge of ZID by Scenario							
Constituent			MPWSP							
			2	3	4	5	6			
Objectives for protection of marine aquatic life - 6-month median limit										
Ammonia (as N) – 6-mo median <sup>b</sup>	μg/L	600 4%		29%	48%	68%	23%			
Objectives for protection of human health – carcinogens – 30-day average limit <sup>cd</sup>										
Chlordane	μg/L	2.3E-05	5%	17%	26%	34%	12%			
PCBs	μg/L	1.9E-05	46%	56%	63%	52%	15%			
TCDD Equivalents <sup>d</sup>	μg/L	3.9E-09	2%	16%	27%	39%	13%			
Toxaphene e	μg/L	2.1E-04	3%	16%	27%	38%	13%			

<sup>&</sup>lt;sup>a</sup> Shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.



<sup>&</sup>lt;sup>b</sup> Ammonia (as N) represents the total ammonia concentration, *i.e.* the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>).

<sup>&</sup>lt;sup>c</sup> Acrylonitrile was only detected in one potential source water for the Variant Project. It was not detected in any potential source waters for the MPWSP Project; therefore, a compliance determination cannot be made for the MPWSP Project and only partial determination can be made for the Variant Project.

d Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time. When only the detected values were considered, acrylonitrile and beryllium did not exceed the Ocean Plan objective by 80% or more and therefore were not included in Tables 7 through 10.

Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day

1.9E-05

3.9E-09

2.1E-04

μg/L

µg/L

μg/L

46%

3%

4%

61%

24%

23%

**PCBs** 

TCDD

Equivalents c

Toxaphene d

Constituent	Units	Ocean Plan Objective	Est. Percentage of Ocean Plan objective at Edge of ZID by Scenario									
			Variant									
			1	2	3	4	5	6	7	8	9	10
Objectives for protection of marine aquatic life - 6-month median limit												
Ammonia (as N) – 6-mo median <sup>b</sup>	μg/L	600	5.7%	41%	66%	74%	40%	185%	192%	177%	74%	25%
Objectives for protection of human health - carcinogens - 30-day average limit c												
Chlordane	μg/L	2.3E-05	6%	23%	35%	37%	20%	94%	97%	88%	37%	12%

Table 10 – Predicted concentrations at the edge of the ZID expressed as percentage of Ocean Plan

Objective for constituents of in the Variant <sup>a</sup>

70%

39%

37%

57%

44%

42%

26%

24%

22%

146%

110%

103%

145%

115%

107%

126%

105%

99%

51%

44%

41%

16%

15%

14%

Potential issues were identified to occur when there is no, or relatively low, secondary effluent flow mixed with hauled brine, GWR Concentrate and Desal Brine, as in Variant Scenarios 6, 7 and 8. The constituents of interest related to these scenarios are ammonia, chlordane, PCBs, TCDD equivalents, and toxaphene. Ammonia is expected to be the constituent with the highest exceedance, being 1.92 times the Ocean Plan objective in Scenario 7 (1 mgd secondary effluent with hauled brine, GWR Concentrate and Desal Brine). This scenario is problematic because constituents that have relatively high loadings in the secondary effluent are concentrated in the GWR Concentrate. This scenario assumes the GWR Concentrate flow is much smaller than the Desal Brine flow, such that the resulting discharge plume is negatively buoyant and achieves poor ocean dilution. Based on this analysis, Scenarios 6, 7 and 8 have been identified as having constituents that may exceed the Ocean Plan objective.

Chlordane, PCBs, and toxaphene were only detected when analyzed with low-detection methods, which have far greater sensitivity than standard methods. These results were used to investigate potential to exceed Ocean Plan objectives because these objectives are orders of magnitude below detection limits of methods currently used for discharge compliance.

<sup>&</sup>lt;sup>a</sup> Shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.

<sup>&</sup>lt;sup>b</sup> Ammonia (as N) represents the total ammonia concentration, *i.e.* the sum of unionized ammonia (NH<sub>3</sub>) and ionized ammonia (NH<sub>4</sub>).

<sup>&</sup>lt;sup>c</sup> Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time. When only the detected values were considered, acrylonitrile and beryllium did not exceed the Ocean Plan objective by 80% or more and therefore were not included in Tables 7 through 10.

<sup>d</sup> Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.

#### 4 Conclusions

The purpose of this analysis was to assess the ability of the MPWSP and Variant to comply with the Ocean Plan objectives. Trussell Tech used a conservative approach to estimate the water qualities of the secondary effluent, GWR Concentrate, Desal Brine and hauled brine for these projects. These water quality data were then combined for various discharge scenarios, and a concentration at the edge of the ZID was calculated for each constituent and scenario. Seventeen constituents showed potential to exceed the Ocean Plan objectives. These constituents can be divided into three categories:

- Detected concentrations exceed Ocean Plan objectives (Category I): four constituents were detected in all source waters and the blended concentration at the edge of the ZID exceeded the Ocean Plan objective
- Insufficient analytical sensitivity to determine compliance (Category II): ten constituents were not detected above the MRL in any of the source waters, but the MRL was not sensitive enough to demonstrate compliance with the Ocean Plan objective
- Combination of Categories I and II: discharge blends contain sources with exceedances of Ocean Plan objectives (Category I) and sources whose compliance is indeterminate (Category II).

Based on the data, assumptions, modeling, and analytical methodology presented in this technical memorandum, the Variant shows a potential to exceed certain Ocean Plan objectives under specific discharge scenarios. In particular, potential issues were identified for the Variant discharge scenarios involving low secondary effluent flows with Desal Brine and GWR Concentrate: discharges are predicted to exceed or come close to exceeding multiple Ocean Plan objectives, specifically those for ammonia, chlordane, PCBs, TCDD equivalents, and toxaphene. Ammonia clearly exceeds the Ocean Plan objective and must be resolved for the Variant. TCDD equivalents shows a potential to exceed the Ocean Plan objective through a best-case analysis. Chlordane, PCBs and toxaphene, which were predicted to exceed the objectives, were detected at concentrations that are orders of magnitude below detection limits of methods currently used for discharge compliance.

#### **5** References

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## **Appendix A**

Table A1 – Complete list of predicted concentrations of Ocean Plan constituents at the edge of the ZID for the MPWSP

		וטו נווו	e IVIPVV3P						
0	11.24	Ocean Plan	Estimated Concentration at Edge of ZID by Scenario						
Constituent	Units	Objective			MPWSP				
			2	3	4	5	6		
Objectives for protection of marine aq	uatic life	- 6-month media	an limit						
Arsenic	μg/L	8	3.9	4.0	4.1	3.7	3.2		
Cadmium	μg/L	1	0.3	0.3	0.3	0.1	0.02		
Chromium (Hexavalent)	μg/L	2	0.1	0.1	0.1	0.04	0.01		
Copper	μg/L	3	1.9	2.0	2.0	2.1	2.0		
Lead	μg/L	2	0.03	0.03	0.03	0.01	0.003		
Mercury	μg/L	0.04	0.03	0.02	0.02	0.01	0.002		
Nickel	μg/L	5	0.7	0.7	0.6	0.2	0.05		
Selenium	µg/L	15	0.04	0.05	0.05	0.04	0.01		
Silver	µg/L	0.7	0.2	<0.2	<0.2	<0.2	<0.2		
Zinc	μg/L	20	8.1	8.1	8.2	8.2	8.0		
Cyanide	µg/L	1	0.6	0.5	0.5	0.2	0.1		
Total Chlorine Residual	µg/L	2	_	_	-	-	-		
Ammonia (as N) - 6-mo median	µg/L	600	25.7	172.1	287	409.0	139.2		
Ammonia (as N) - Daily Max	μg/L	2,400	31.4	228.8	384	549.8	187.2		
Acute Toxicity a	TUa	0.3							
Chronic Toxicity a	TUc	1							
Phenolic Compounds (non-chlorinated)	μg/L	30	5.5	5.2	4.9	2.2	0.5		
Chlorinated Phenolics b	μg/L	1	<2.20	<2.06	<1.92	<0.82	<0.17		
Endosulfan	μg/L	0.009	7.05E-06	6.77E-05	1.15E-04	1.68E-04	5.72E-05		
Endrin	µg/L	0.002	1.35E-07	4.45E-07	6.86E-07	9.09E-07	3.05E-07		
HCH (Hexachlorocyclohexane)	µg/L	0.004	1.82E-05	1.56E-04	2.63E-04	3.81E-04	1.30E-04		
Radioactivity (Gross Beta) a	pCi/L	0.0							
Radioactivity (Gross Alpha) a	pCi/L	0.0							
Objectives for protection of human he	alth – non								
Acrolein	μg/L	220	<0.2	<0.2	<0.2	<0.1	<0.03		
Antimony	μg/L	1200	0.01	0.01	0.01	0.01	0.003		
Bis (2-chloroethoxy) methane	μg/L	4.4	<1.1	<1.0	<0.9	<0.3	<0.05		
Bis (2-chloroisopropyl) ether	μg/L	1200	<1.1	<1.0	<0.9	<0.3	<0.05		
Chlorobenzene	µg/L	570	<0.1	<0.1	<0.05	<0.02	<0.004		
Chromium (III)	µg/L	190000	1.1	1.0	0.9	0.3	0.1		
Di-n-butyl phthalate	µg/L	3500	<1.1	<1.0	<0.9	<0.3	<0.1		
Dichlorobenzenes	µg/L	5100	<0.1	0.1	0.1	0.03	0.01		
Diethyl phthalate	μg/L	33000	<0.1	<0.1	<0.1	<0.1	<0.02		
Dimethyl phthalate	µg/L	820000	<0.1	<0.1	<0.1	<0.04	<0.01		
4,6-dinitro-2-methylphenol	μg/L	220	<5.4	<4.8	<4.3	<1.5	<0.2		
2,4-Dinitrophenol b	μg/L	4.0	<5.5	<4.9	<4.4	<1.5	<0.2		
Ethylbenzene	μg/L	4100	<0.1	<0.1	<0.05	<0.02	<0.004		
Fluoranthene	μg/L	15	<0.01	0.01	0.01	0.003	0.0005		
Hexachlorocyclopentadiene	μg/L	58	<0.01	<0.01	<0.01	<0.01	<0.002		
Nitrobenzene	μg/L	4.9	<2.6	<2.4	<2.1	<0.7	<0.1		
Thallium	μg/L	2	<0.01	<0.01	<0.01	<0.01	<0.002		
Toluene	μg/L	85000	<0.06	<0.05	<0.05	<0.02	<0.004		
Tributyltin b	μg/L	0.0014 540000	<0.01 <0.1	<0.005 <0.1	<0.005 <0.05	<0.002 <0.02	<0.0004 <0.004		
1,1,1-Trichloroethane  Objectives for protection of human he	μg/L alth – card				<b>^</b> U.U5	<b>^</b> U.UZ	<u>\$0.004</u>		
Acrylonitrile cd	µg/L	0.10	., a orago iiiii						
Aldrin b	μg/L	0.000022	<6.51E-06	<2.63E-05	<4.18E-05	<5.70E-05	<1.92E-05		
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	Ocean Plan	Estimated Concentration at Edge of ZID by Scenario					
Units				MPWSP			
	Í	2	3	4	5	6	
μq/L	5.9	<0.1	<0.1	<0.05	<0.02	<0.004	
	0.000069	<5.5	<4.9	<4.4	<1.5	<0.2	
	0.033	2.38E-6	2.14E-6	1.91E-6	6.41E-7	1.00E-7	
	0.045	<2.6	<2.4	<2.1	<0.7	<0.1	
	3.5	0.1	0.4	0.7	0.9	0.3	
	0.90	<0.1	<0.1	<0.05	<0.02	<0.004	
	0.000023	1.23E-6	3.91E-6	6.00E-6	7.89E-6	2.65E-6	
	8.6	<0.1	<0.1	< 0.05	<0.02	<0.004	
μg/L	130	0.1	0.1	0.1	0.04	0.01	
μg/L	0.00017	1.53E-7	5.28E-7	8.21E-7	1.09E-6	3.68E-7	
	18	0.1	0.1	0.1	0.03	0.01	
μg/L	0.0081	<5.5	<4.9	<4.4	<1.5	<0.2	
μg/L	28	<0.1	<0.1	<0.05	<0.02	< 0.004	
	0.9	0.1	0.1	0.05	0.02	0.004	
	6.2	<0.1	<0.1	< 0.05	<0.02	<0.004	
	450	<0.1	0.1	0.05	0.02	0.004	
	8.9	<0.1	<0.1	<0.05	<0.02	<0.004	
		3.01E-6	3.15E-6			5.37E-7	
	2.6		< 0.02	<0.02	< 0.03	<0.01	
μg/L	0.16	<1.1	<1.0	<0.9	<0.3	<0.05	
μg/L	130	0.1	0.1	0.05	0.02	0.004	
	0.00005	<4.60E-06	<4.51E-05	<7.69E-05	<1.12E-04	<3.81E-05	
μg/L	0.00002	1.35E-07	4.45E-07	6.86E-07	9.09E-07	3.05E-07	
μg/L	0.00021	4.18E-06	4.08E-06	3.93E-06	1.99E-06	4.72E-07	
	14	2.60E-08	6.03E-08	8.68E-08	1.06E-07	3.52E-08	
	2.5	<1.1	<1.0	<0.9	<0.3	< 0.05	
µg/L	730	<0.1	<0.1	< 0.05	<0.02	<0.004	
µg/L	7.3	0.0002	0.0003	0.0003	0.0002	0.0001	
	0.38	0.0003	0.001	0.001	0.001	0.0003	
	2.5	<1.1	<1.0	<0.9	<0.3	< 0.05	
	0.0088	1.51E-04	2.48E-04	3.23E-04	3.45E-04	1.11E-04	
	0.000019	8.76E-06	1.07E-05	1.20E-05	9.86E-06	2.94E-06	
µg/L	3.9E-09	6.23E-11	6.17E-10		1.53E-09	5.22E-10	
μg/L	2.3	<0.1	<0.1	<0.05	<0.02	<0.004	
	2.0	<0.1	<0.1	<0.05	<0.02	<0.004	
µg/L	2.1E-04	5.75E-06	3.42E-05	5.65E-05	7.99E-05	2.71E-05	
µg/L	27	<0.1	<0.1	< 0.05	<0.02	<0.004	
	9.4	<0.1	<0.1	< 0.05	<0.02	<0.004	
	0.29	<1.1	<1.0	<0.9	<0.3	< 0.05	
	36	< 0.03	<0.03	< 0.03	<0.01	< 0.003	
	рд/L рд/L рд/L рд/L рд/L рд/L рд/L рд/L	ру/L 5.9  ру/L 0.000069  ру/L 0.033  ру/L 0.045  ру/L 0.90  ру/L 0.00023  ру/L 0.00017  ру/L 130  ру/L 0.00017  ру/L 18  ру/L 0.0081  ру/L 0.99  ру/L 0.0081  ру/L 0.99  ру/L 0.99  ру/L 0.9  ру/L 0.9  ру/L 0.9  ру/L 450  ру/L 450  ру/L 8.9  ру/L 0.00004  ру/L 2.6  ру/L 0.16  ру/L 130  ру/L 0.00005  ру/L 0.00002  ру/L 14  ру/L 130  ру/L 0.00002  ру/L 0.00001  ру/L 0.00088  ру/L 0.000019  ру/L 0.000019  ру/L 2.3  ру/L 2.0  ру/L 2.1E-04  ру/L 2.7  ру/L 0.29	Units         Ocean Plan Objective           µg/L         5.9         <0.1	Units         Ocean Plan Objective           µg/L         5.9         <0.1	Units         Objective Objective         2         3         4           µg/L         5.9         <0.1	Units   Ocean Plan Objective   2   3   4   5     μg/L   5.9   <0.1   <0.1   <0.05   <0.02     μg/L   0.000069   <5.5   <4.9   <4.4   <1.5     μg/L   0.033   2.38E-6   2.14E-6   1.91E-6   6.41E-7     μg/L   0.045   <2.6   <2.4   <2.1   <0.7     μg/L   0.90   <0.1   <0.1   <0.05   <0.02     μg/L   0.90   <0.1   <0.1   <0.05   <0.02     μg/L   0.90   <0.1   <0.1   <0.05   <0.02     μg/L   0.000023   1.23E-6   3.91E-6   6.00E-6   7.89E-6     μg/L   130   0.1   0.1   0.1   0.05   <0.02     μg/L   130   0.1   0.1   0.1   0.04     μg/L   0.00017   1.53E-7   5.28E-7   8.21E-7   1.09E-6     μg/L   18   0.1   0.1   0.1   0.03     μg/L   0.0081   <5.5   <4.9   <4.4   <1.5     μg/L   0.9   0.1   0.1   0.05   <0.02     μg/L   0.9   0.1   0.1   0.05   <0.02     μg/L   0.9   0.1   0.1   0.05   <0.02     μg/L   450   <0.1   <0.1   <0.05   <0.02     μg/L   8.9   <0.1   <0.1   <0.05   <0.02     μg/L   0.00004   3.01E-6   3.15E-6   3.21E-6   2.01E-6     μg/L   0.16   <1.1   <1.0   <0.9   <0.3     μg/L   0.16   <1.1   <1.0   <0.05   <0.02     μg/L   0.16   <1.1   <1.0   <0.9   <0.3     μg/L   0.00005   <4.60E-06   <4.51E-05   <7.69E-05   <1.12E-04     μg/L   0.00002   4.18E-06   4.08E-06   3.93E-06   1.99E-06     μg/L   0.38   0.003   0.001   0.001   0.005   <0.02     μg/L   0.0002   4.18E-06   4.08E-06   3.93E-06   1.99E-06     μg/L   0.38   0.0003   0.001   0.001   0.001     μg/L   0.0088   1.51E-04   2.48E-04   3.23E-04   3.45E-04     μg/L   0.000019   8.76E-06   1.07E-05   1.20E-05   9.86E-06     μg/L   0.00001   0.01   0.01   0.005   0.002     μg/L   0.00001   0.01	

<sup>&</sup>lt;sup>a</sup> Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based the nature of the constituent.

<sup>&</sup>lt;sup>b</sup> All observed values from some data sources were below the MRL, and the flow-weighted average of the MRLs is higher than the Ocean Plan objective. No compliance conclusions can be drawn for these constituents.

<sup>&</sup>lt;sup>c</sup> Acrylonitrile was only detected in one potential source water for the Variant Project. It was not detected in any potential source waters for the MPWSP Project; therefore, a compliance determination cannot be made for the MPWSP Project and only partial determination can be made for the Variant Project.

d Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance

determination at this time. When only the detected values were considered, acrylonitrile and beryllium did not exceed the Ocean Plan objective by 80% or more and therefore were not included in Tables 7 through 10.

<sup>e</sup> Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.

Table A2 – Complete list of predicted concentrations at the edge of the ZID expressed as a percentage of Ocean Plan<sup>a</sup>

Constituent Units Ocean Plan	ge of Ocean Plan Objective at Edge of ZID by Scenario <sup>a</sup>							
Constituent	rcentage of Ocean Plan Objective at Edge of ZID by Scenario <sup>a</sup>							
Objective	MPWSP							
2	3 4 5 6							
Objectives for protection of marine aquatic life - 6-month median limit								
Arsenic	50% 51% 46% 40%							
Cadmium μg/L 1 32%	29% 26% 10% 2%							
Chromium (Hexavalent) µg/L 2 3%	3% 3% 2% 1%							
Copper µg/L 3 64%	65% 67% 69% 68%							
Lead	2% 2% 1% 0.2%							
Mercury µg/L 0.04 67%	61% 54% 20% 4%							
Nickel µg/L 5 14%	13% 12% 5% 1%							
Selenium µg/L 15 0.3%	0.3% 0.4% 0.3% 0.1%							
Silver µg/L 0.7 26%	<26% <25% <24% <23%							
Zinc µg/L 20 40%	41% 41% 41% 40%							
Cyanide µg/L 1 57%	54% 51% 23% 5%							
Total Chlorine Residual µg/L 2 –								
Ammonia (as N) - 6-mo median µg/L 600 4%	29% 48% 68% 23%							
Ammonia (as N) - Daily Max µg/L 2,400 1%	10% 16% 23% 8%							
Acute Toxicity b TUa 0.3	1070 2070 070							
Chronic Toxicity b TUc 1								
Phenolic Compounds (non-chlorinated) µg/L 30 18%	17% 16% 7% 2%							
Chlorinated Phenolics c µg/L 1								
Endosulfan µg/L 0.009 0.1%	1% 1% 2% 1%							
Endrin µg/L 0.002 0.01%	0.02% 0.03% 0.05% 0.02%							
HCH (Hexachlorocyclohexane) µg/L 0.004 0.5%	4% 7% 10% 3%							
Radioactivity (Gross Beta) b pci/L 0.0	170 170 1070 070							
Radioactivity (Gross Alpha) b pci/L 0.0								
Objectives for protection of human health – non carcinogens – 30-day average	age limit							
Acrolein	<0.1% <0.1% <0.1% <0.01% <0.01%							
Antimony µg/L 1200 0.0010%	0.0011% 0.0012% 0.0009% 0.0002%							
Bis (2-chloroethoxy) methane µg/L 4.4 <24%	<22% <20% <7% <1%							
Bis (2-chloroisopropyl) ether µg/L 1200 <0.09%	<0.08% <0.07% <0.02% <0.01%							
Chlorobenzene µg/L 570 <0.01%	<0.01% <0.01% <0.01% <0.01% <0.01% <0.01%							
Chromium (III)	0.0005% 0.0005% 0.0002% 0.00003%							
Di-n-butyl phthalate	<0.03% <0.03% <0.01% <0.01%							
Dichlorobenzenes	0.001% 0.001% 0.001% 0.0002%							
Diethyl phthalate µg/L 33000 <0.01%	<0.01% <0.01% <0.01% <0.01% <0.01%							
Dimethyl phthalate         μg/L         820000         <0.01%	<0.01% <0.01% <0.01% <0.01% <0.01%							
4,6-dinitro-2-methylphenol µg/L 220 <2%	<2% <2% <1% <0.1%							
2,4-Dinitrophenol ° µg/L 4.0								
Ethylbenzene µg/L 4100 <0.01%	<0.01% <0.01% <0.01% <0.01%							
Fluoranthene µg/L 15 0.1%	0.1% 0.1% 0.02% 0.003%							
Hexachlorocyclopentadiene µg/L 58 <0.01%	<0.01% <0.01% <0.01% <0.01% <0.01%							
Nitrobenzene µg/L 4.9 <54%	<48% <43% <15% <2%							
Thallium µg/L 2 <0.3%	<0.4% <0.4% <0.4% <0.1%							
Toluene µg/L 85000 <0.01%	<0.01% <0.01% <0.01% <0.01% <0.01%							
Tributyltin ° μg/L 0.0014								
1,1,1-Trichloroethane µg/L 540000 <0.01%	<0.01% <0.01% <0.01% <0.01%							

		Ocean Plan	Percentage of Ocean Plan Objective at Edge of ZID by Scenario						
Constituent	Units	Objective			MPWSP				
		•	2	3	4	5	6		
Objectives for protection of human hea	alth – carc	inogens – 30-da	ay average lim	it					
Acrylonitrile de	μg/L	0.10			-				
Aldrin <sup>c</sup>	μg/L	0.000022			-				
Benzene	μg/L	5.9	<1%	<1%	<1%	<0.3%	<0.1%		
Benzidine <sup>c</sup>	μg/L	0.000069							
Beryllium <sup>e</sup>	μg/L	0.033	0%	0%	0%	0%	0%		
Bis(2-chloroethyl)ether c	μg/L	0.045							
Bis(2-ethyl-hexyl)phthalate	μg/L	3.5	3%	12%	19%	25%	9%		
Carbon tetrachloride	μg/L	0.90	<6%	<6%	<5%	<2%	<0.5%		
Chlordane	μg/L	0.000023	5%	17%	26%	34%	12%		
Chlorodibromomethane	μg/L	8.6	<1%	<1%	<1%	<0.2%	<0.05%		
Chloroform	μg/L	130	0.04%	0.04%	0.05%	0.03%	0.01%		
DDT	μg/L	0.00017	0.09%	0.31%	0.48%	0.64%	0.22%		
1,4-Dichlorobenzene	μg/L	18	0.3%	0.3%	0.3%	0.2%	0.05%		
3,3-Dichlorobenzidine c	μg/L	0.0081							
1,2-Dichloroethane	μg/L	28	<0.2%	<0.2%	<0.2%	<0.1%	<0.02%		
1,1-Dichloroethylene	μg/L	0.9	6%	6%	5%	2%	0.5%		
Dichlorobromomethane	μg/L	6.2	<1%	<1%	<1%	<0.3%	<0.1%		
Dichloromethane	μg/L	450	0.01%	0.01%	0.01%	0.005%	0.001%		
1,3-dichloropropene	μg/L	8.9	<1%	<1%	<1%	<0.2%	<0.05%		
Dieldrin	μg/L	0.00004	8%	8%	8%	5%	1%		
2,4-Dinitrotoluene	µg/L	2.6	<0.5%	<1%	<1%	<1%	<0.3%		
1,2-Diphenylhydrazine °	μg/L	0.16							
Halomethanes	μg/L	130	0.04%	0.04%	0.04%	0.02%	0.003%		
Heptachlor <sup>c</sup>	μg/L	0.00005	/	-					
Heptachlor Epoxide	μg/L	0.00002	1%	2%	3%	5%	2%		
Hexachlorobenzene	μg/L	0.00021	2%	2%	2%	1%	0.2%		
Hexachlorobutadiene	μg/L	14	1.86E-7%	4.30E-7%	6.20E-7%	7.60E-7%	2.52E-7%		
Hexachloroethane	μg/L	2.5	<43%	<38%	<35%	<12%	<2%		
Isophorone	μg/L	730	<0.008%	<0.007%	<0.007%	<0.003%	<0.001%		
N-Nitrosodimethylamine	μg/L	7.3	0.003%	0.004%	0.004%	0.003%	0.001%		
N-Nitrosodi-N-Propylamine	µg/L	0.38	0.1%	0.1%	0.2%	0.2%	0.1%		
N-Nitrosodiphenylamine	µg/L	2.5	<43%	<38%	<34%	<12%	<2%		
PAHs	µg/L	0.0088	2%	3%	4%	4%	1%		
PCBs	μg/L	0.000019	46%	56%	63%	52%	15%		
TCDD Equivalents e	μg/L	3.9E-09	2%	16%	27%	38%	13%		
1,1,2,2-1 etrachloroethane	µg/L	2.3	<2%	<2%	<2%	<1%	<0.2%		
Tetrachloroethylene	µg/L	2.0	<3%	<3%	<2%	<1%	<0.2%		
Toxaphene e	μg/L	2.1E-04	3%	16%	27%	38%	13%		
Trichloroethylene	μg/L	27	<0.2%	<0.2%	<0.2%	<0.1%	<0.02%		
1,1,2-Trichloroethane	µg/L	9.4	<1%	<1%	<1%	<0.2%	<0.04%		
2,4,6-Trichlorophenol c	µg/L	0.29	 -0 10/	 -0.10/	 -0 10/		<0.01%		
Vinyl chloride	μg/L	36	<0.1%	<0.1%	<0.1%	<0.04%	<0.01%		

<sup>&</sup>lt;sup>a</sup> Note that if the percentage as determined by using the MRL was less than 0.01 percent, then a minimum value is shown as "<0.01%" (e.g., if the MRL indicated the value was <0.000001%, for simplicity, it is displayed as <0.01%). Also, shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.

<sup>&</sup>lt;sup>b</sup> Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based the nature of the constituent. These constituents were measured individually for the secondary effluent and GWR concentrate, and these individual concentrations would comply with the Ocean Plan objectives.

<sup>&</sup>lt;sup>c</sup> All observed values from all data sources were below the MRL, and the flow-weighted average of the MRLs is higher than the Ocean Plan objective. No compliance conclusions can be drawn for these constituents.

d Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time. When only the detected values were considered, acrylonitrile and beryllium did not exceed the Ocean Plan objective by 80% or more and therefore were not included in Tables 7 through 10.

Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.

Table A3 – Complete list of predicted concentrations of Ocean Plan constituents at the edge of the ZID for the Variant

		Ocean			Estimat	ed Conce	entration	at Edge o	of ZID by	Scenario		
Constituent	Units	Plan					Va	riant				
		Objective	1	2	3	4	5	6	7	8	9	10
Objectives for protection of	of mari	ne aquatic	life - 6-n	nonth me	dian limi	t						
Arsenic	μg/L	8	3.9	4.0	4.1	3.8	3.3	3.8	4.0	4.0	3.4	3.2
Cadmium	µg/L	1	0.3	0.3	0.2	0.1	0.02	0.3	0.3	0.2	0.1	0.01
Chromium (Hexavalent)	µg/L	2	0.09	0.09	0.09	0.06	0.02	0.16	0.2	0.1	0.05	0.01
Copper	µg/L	3	1.9	2.0	2.0	2.1	2.1	2.2	2.3	2.2	2.1	2.0
Lead	µg/L	2	0.03	0.03	0.03	0.02	0.01	0.1	0.05	0.04	0.02	0.004
Mercury	µg/L	0.04	0.03	0.02	0.02	0.01	0.002	0.03	0.02	0.02	0.01	0.002
Nickel	µg/L	5	0.7	0.7	0.6	0.4	0.1	1.0	0.9	0.7	0.3	0.1
Selenium	μg/L	15	0.1	0.1	0.1	0.1	0.05	0.2	0.2	0.2	0.1	0.03
Silver	μg/L	0.7	0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2
Zinc	µg/L	20	8.1	8.3	8.5	8.5	8.3	9.5	9.5	9.3	8.5	8.2
Cyanide	μg/L	1	0.6	0.6	0.5	0.3	0.1	0.7	0.7	0.5	0.2	0.05
Total Chlorine Residual	μg/L	2	_	_	-	_	-	-	_	_	_	_
Ammonia (as N) - 6-mo median	μg/L	600	34	245	396	446	239	1111	1154	1060	445	151
Ammonia (as N) - Daily Max	μg/L	2,400	43	328	531	600	322	1493	1551	1425	598	203
Acute Toxicity a	TUa	0.3										
Chronic Toxicity a	TUc	1										
Phenolic Compounds (non- chlorinated)	μg/L	30	5.4	5.0	4.7	2.4	0.7	6.7	6.2	4.8	1.8	0.4
Chlorinated Phenolics b	μg/L	1	<2.2	<2.0	<1.8	< 0.9	<0.2	<2.0	<1.8	<1.4	<0.5	<0.1
Endosulfan	μg/L	0.009	3.3E-05	3.1E-04	5.1E-04	5.9E-04	3.2E-04	1.5E-03	1.4E-03	1.4E-03	5.9E-04	2.0E-04
Endrin	μg/L	0.002	1.5E-07	6.0E-07	9.2E-07	9.9E-07	5.2E-07	2.5E-06	2.6E-06	2.3E-06	9.8E-07	3.3E-07
HCH (Hexachlorocyclohexane)	μg/L	0.004	4.4E-05	3.9E-04	6.4E-04	7.3E-04	3.9E-04	1.8E-03	1.9E-03	1.7E-03	7.3E-04	2.5E-04
Radioactivity (Gross Beta) a	pci/L	0.0										
Radioactivity (Gross Alpha) <sup>a</sup>	pci/L	0.0										
Objectives for protection of	of hum	an health –	non card	inogens	- 30-day	average	limit					
Acrolein	µg/L	220	0.2	0.2	0.3	0.2	0.1	0.5	0.4	0.4	0.1	0.04
Antimony	μg/L	1200	0.01	0.02	0.02	0.01	0.01	0.03	0.03	0.03	0.01	0.004
Bis (2-chloroethoxy) methane	μg/L	4.4	<1.0	<0.9	<0.8	<0.4	<0.1	<0.9	<0.8	<0.6	<0.2	<0.04
Bis (2-chloroisopropyl) ether	μg/L	1200	<1.0	<0.9	<0.8	<0.4	<0.1	<0.9	<0.8	<0.6	<0.2	<0.04
Chlorobenzene	μg/L	570	<0.1	<0.05	<0.04	<0.02	<0.01	<0.05	<0.05	<0.04	<0.01	<0.003
Chromium (III)	μg/L	190000	1.1	1.0	0.9	0.4	0.1	1.2	1.1	0.8	0.3	0.1
Di-n-butyl phthalate	µg/L	3500	<1.0	<0.9	<0.8	<0.4	<0.1	<0.9	<0.8	<0.6	<0.2	<0.1
Dichlorobenzenes	μg/L	5100	0.1	0.1	0.1	0.04	0.01	0.1	0.1	0.1	0.03	0.01
Diethyl phthalate	µg/L	33000	<0.1	<0.1	<0.1	<0.1	<0.04	<0.1	<0.1	<0.1	<0.04	<0.02
Dimethyl phthalate	μg/L	820000	<0.1	<0.1	<0.1	<0.04	<0.02	<0.1	<0.1	<0.05	<0.02	<0.01

		Ocean	Estimated Concentration at Edge of ZID by Scenario									
Constituent	Units	Plan					Va	riant				
		Objective	1	2	3	4	5	6	7	8	9	10
4,6-dinitro-2-methylphenol	μg/L	220	<5.3	<4.6	<4.1	<1.8	<0.4	<4.6	<4.1	<3.0	<1.0	<0.2
2,4-Dinitrophenol b	μg/L	4.0	<5.4	<4.7	<4.1	<1.8	<0.3	<4.7	<4.1	<3.0	<1.0	<0.2
Ethylbenzene	μg/L	4100	<0.1	<0.05	<0.04	<0.02	<0.01	< 0.05	<0.05	<0.04	<0.01	<0.003
Fluoranthene	μg/L	15	0.01	0.01	0.01	0.003	0.001	0.01	0.01	0.01	0.002	0.0003
Hexachlorocyclopentadiene	μg/L	58	<0.01	<0.01	<0.01	<0.01	<0.004	<0.01	<0.01	<0.01	<0.004	<0.002
Nitrobenzene	μg/L	4.9	<2.6	<2.2	<1.9	<0.8	<0.1	<2.2	<2.0	<1.4	< 0.5	<0.1
Thallium	μg/L	2	0.01	0.01	0.01	0.01	0.005	0.03	0.03	0.02	0.01	0.003
Toluene	μg/L	85000	<0.1	<0.05	<0.04	<0.02	<0.01	<0.05	< 0.05	<0.04	<0.01	<0.003
Tributyltin <sup>b</sup>	μg/L	0.0014	<0.01	<0.005	<0.004	<0.002	<0.001	<0.005	<0.004	<0.003	<0.001	<0.0003
1,1,1-Trichloroethane	μg/L	540000	<0.05	<0.05	<0.04	<0.02	<0.01	<0.05	<0.05	<0.04	<0.01	<0.003
Objectives for protection of												
Acrylonitrile c	μg/L	0.10	0.001	0.007	0.011	0.012	0.007	0.034	0.035	0.031	0.013	0.004
Aldrin <sup>b</sup>	μg/L	0.000022	<9.0E- 06	<4.9E- 05	<7.8E- 05	<8.7E- 05	<4.6E-05	<6.4E-05	<9.2E-05	<1.1E-04		<2.4E-05
Benzene	μg/L	5.9	<0.1	<0.05	<0.04	<0.02	<0.01	<0.05	<0.05	<0.04	<0.01	<0.003
Benzidine b	μg/L	0.000069	<5.4	<4.7	<4.2	<1.8	<0.4	<4.7	<4.2	<3.0	<1.0	<0.2
Beryllium <sup>c</sup>	μg/L	0.033	3.61E-6	3.10E-6	2.66E-6	1.08E-6	1.72E-7	3.14E-6	2.72E-6	1.88E-6	6.15E-7	1.03E-7
Bis(2-chloroethyl)ether b	μg/L	0.045	<2.6	<2.2	<1.9	<0.8	<0.2	<2.2	<2.0	<1.4	<0.5	<0.1
Bis(2-ethyl-hexyl)phthalate	μg/L	3.5	0.1	0.6	0.9	1.0	0.5	2.4	2.5	2.3	1.0	0.3
Carbon tetrachloride	μg/L	0.90	0.1	0.05	0.04	0.02	0.01	0.1	0.1	0.04	0.02	0.004
Chlordane	μg/L	0.000023		5.2E-06			4.5E-06		2.2E-05	2.0E-05	8.5E-06	2.9E-06
Chlorodibromomethane	μg/L	8.6	0.1	0.1	0.1	0.05	0.02	0.1	0.1	0.1	0.04	0.01
Chloroform	μg/L	130	0.1	0.3	0.5	0.5	0.3	1.2	1.3	1.2	0.5	0.2
DDT	μg/L	0.00017		8.1E-06		0.04	8.1E-06	3.7E-05	3.9E-05	3.6E-05	1.5E-05	5.1E-06
1,4-Dichlorobenzene	μg/L	18	0.1	0.1 <4.7	0.1 <4.2	<1.8	<0.4	0.1 <4.7	0.1 <4.2	0.1 <3.0	0.03	0.01
3,3-Dichlorobenzidine b 1,2-Dichloroethane	μg/L	0.0081 28	<5.4 <0.1		<0.04	<0.02	<0.4		<0.05	<0.04	<1.0	<0.2 <0.003
1,1-Dichloroethylene	μg/L	0.9	0.1	<0.05 0.05	0.04	0.02	0.01	<0.05 0.05	0.05	0.04	<0.01 0.01	0.003
Dichlorobromomethane	μg/L μg/L	6.2	0.1	0.05	0.04	0.02	0.01	0.05	0.05	0.04	0.01	0.003
Dichloromethane	μg/L	450	0.1	0.05	0.05	0.03	0.02	0.1	0.1	0.05	0.04	0.004
1,3-dichloropropene	μg/L	8.9	0.1	0.05	0.05	0.02	0.01	0.1	0.1	0.03	0.02	0.004
Dieldrin	μg/L μg/L	0.00004	3.3E-06	6.6E-06	8.8E-06	8.5E-06	4.2E-06	2.1E-05		2.0E-05	8.1E-06	2.7E-06
2,4-Dinitrotoluene	μg/L μg/L	2.6	<0.01	<0.02	<0.03	<0.03	<0.01	<0.01	<0.02	<0.03	<0.01	<0.01
1,2-Diphenylhydrazine b	µg/L	0.16	<1.0	<0.02	<0.8	<0.4	<0.1	<0.9	<0.8	<0.6	<0.01	<0.04
Halomethanes	μg/L	130	0.1	0:1	0.1	0.03	0.01	0.1	0.1	0.1	0.03	0.01
Heptachlor b	μg/L	0.00005	<7.0E-6	<6.5E-5	<1.1E-4	<1.2E-4	<6.6E-05	<6.3E-05	<1.1E-04	<1.5E-04	<7.5E-05	<3.4E-05
Heptachlor Epoxide	μg/L	0.00002	1.5E-7	6.0E-7	9.2E-7	9.9E-7	5.2E-7	2.5E-6	2.6E-6	2.3E-6	9.8E-7	3.3E-7
Hexachlorobenzene	μg/L	0.00021	4.1E-6	4.0E-6	3.8E-6	2.2E-6	7.0E-7	5.9E-6	5.5E-6	4.4E-6	1.6E-6	4.4E-7
Hexachlorobutadiene	μg/L	14	2.8E-8	7.7E-8	1.1E-7	1.2E-7	6.0E-8	2.9E-7	3.0E-7	2.7E-7	1.1E-7	3.8E-8
Hexachloroethane	μg/L	2.5	<1.0	< 0.9	<0.8	<0.3	<0.1	< 0.9	<0.8	<0.6	<0.2	<0.04
Isophorone	µg/L	730	<0.1	<0.05	<0.04	<0.02	<0.01	<0.05	<0.05	<0.04	<0.01	<0.003
N-Nitrosodimethylamine	µg/L	7.3	0.0003	0.001	0.001	0.001	0.001	0.001	0.001	0.002	0.001	0.0003
N-Nitrosodi-N-Propylamine	μg/L	0.38	0.0003	0.001	0.001	0.001	0.001	0.0003	0.001	0.001	0.001	0.0003
N-Nitrosodiphenylamine	μg/L	2.5	<1.0	<0.9	<0.8	<0.3	<0.1	<0.9	<0.8	<0.6	<0.2	<0.04
PAHs	μg/L	0.0088	0.0002	0.0003	0.0004	0.0004	0.0002	0.0012	0.0012	0.0010	0.0004	0.0001
PCBs	μg/L	0.000019	8.7E-6	1.2E-5	1.3E-5	1.1E-5	4.8E-6	2.8E-5	2.8E-5	2.4E-5	9.7E-6	3.0E-6
TCDD Equivalents c	μg/L	3.9E-09	9.8E-11	9.3E-10	1.5E-9	1.7E-9	9.3E-10	4.3E-9	4.5E-9	4.1E-9	1.7E-9	5.9E-10
1,1,2,2-Tetrachloroethane	μg/L	2.3	<0.1	<0.05	< 0.04	<0.02	<0.01	< 0.05	< 0.05	<0.04	<0.01	<0.003
Tetrachloroethylene	μg/L	2.0	<0.1	<0.05	<0.04	<0.02	<0.01	<0.05	<0.05	<0.04	<0.01	<0.003
Toxaphene e	μg/L	2.1E-04		4.8E-05			4.7E-05	2.2E-04	2.3E-04	2.1E-04	8.7E-05	2.9E-05
Trichloroethylene	μg/L	27	<0.1	<0.05	<0.04	<0.02	<0.01	<0.05	<0.05	<0.04	<0.01	<0.003
1,1,2-Trichloroethane	μg/L	9.4	<0.1	<0.05	<0.04	<0.02	<0.01	<0.05	<0.05	<0.04	<0.01	<0.003
2,4,6-Trichlorophenol b	μg/L	0.29	<1.0	<0.9	<0.8	<0.3	<0.1	<0.9	<0.8	<0.6	<0.2	<0.04
Vinyl chloride	μg/L	36	<0.03	<0.03	<0.03	<0.02	<0.005	<0.03	<0.03	<0.02	<0.01	<0.003

- <sup>a</sup> Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based the nature of the constituent. These constituents were measured individually for the secondary effluent and GWR concentrate, and these individual concentrations would comply with the Ocean Plan objectives.
- <sup>b</sup> All observed values from some data sources were below the MRL, and the flow-weighted average of the MRLs is higher than the Ocean Plan objective. No compliance conclusions can be drawn for these constituents.
- c Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time. When only the detected values were considered, acrylonitrile and beryllium did not exceed the Ocean Plan objective by 80% or more and therefore were not included in Tables 7 through 10.
  c Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.

Table A4 – Complete list of predicted concentrations at the edge of the ZID expressed as a percentage of Ocean Plan<sup>a</sup>

		Ocean	n Percentage of Ocean Plan Objective at Edge of ZID by Scenario <sup>a</sup>									
Constituent	Units	Plan					Va	riant				
		Objective	1	2	3	4	5	6	7	8	9	10
Objectives for protection	n of m	arine aqua	tic life -	6-month	median I	imit						
Arsenic	μg/L	8	49%	50%	51%	47%	41%	48%	49%	50%	43%	39%
Cadmium	μg/L	1	31%	27%	24%	11%	2%	31%	27%	20%	7%	1%
Chromium (Hexavalent)	μg/L	2	5%	5%	5%	3%	1%	8%	8%	6%	2%	1%
Copper	μg/L	3	64%	66%	68%	69%	68%	75%	75%	75%	70%	68%
Lead	μg/L	2	2%	2%	2%	1%	0.3%	3%	2%	2%	1%	0.2%
Mercury	μg/L	0.04	66%	58%	51%	23%	6%	64%	57%	42%	15%	4%
Nickel	μg/L	5	14%	13%	13%	7%	2%	20%	19%	15%	6%	1%
Selenium	μg/L	15	0.4%	1%	1%	1%	0.3%	2%	2%	1%	1%	0.2%
Silver	μg/L	0.7	26%	<27%	<27%	<26%	<24%	<26%	<26%	<27%	<25%	<24%
Zinc	μg/L	20	41%	42%	43%	43%	41%	47%	48%	47%	43%	41%
Cyanide	μg/L	1	57%	53%	49%	26%	7%	71%	65%	50%	18%	5%
Total Chlorine Residual	μg/L	2	-	_	_	_	_	_	-	_	_	_
Ammonia (as N) - 6-mo median	μg/L	600	6%	41%	66%	74%	40%	185%	192%	177%	74%	25%
Ammonia (as N) - Daily Max	μg/L	2,400	2%	14%	22%	25%	13%	62%	65%	59%	25%	8%
Acute Toxicity b	TUa	0.3										
Chronic Toxicity b	TUc	1										
Phenolic Compounds (non-chlorinated)	μg/L	30	<18%	<17%	<16%	<8%	<2%	<22%	<21%	<16%	<6%	<1%
Chlorinated Phenolics c	μg/L	1										
Endosulfan	μg/L	0.009	0.4%	3%	6%	7%	4%	16%	17%	15%	7%	2%
Endrin	μg/L	0.002	0.01%	0.03%	0.05%	0.05%	0.03%	0.1%	0.1%	0.1%	0.05%	0.02%
HCH (Hexachlorocyclohexane)	μg/L	0.004	1%	10%	16%	18%	10%	45%	47%	43%	18%	6%
Radioactivity (Gross Beta) <sup>b</sup>	pci/L	0.0										
Radioactivity (Gross Alpha) b	pci/L	0.0										
Objectives for protection	n of hu	ıman healt	h – non d	carcinog	ens – 30-c	lay avera	ge limit					
Acrolein	μg/L	220	0.1%	0.1%	0.1%	0.1%	0.03%	0.2%	0.2%	0.2%	0.1%	0.02%
Antimony	μg/L	1200	0.001%	0.001%	0.001%	0.001%	0.0005%	0.003%	0.003%	0.002%	0.001%	0.0003%
Bis (2-chloroethoxy) methane	μg/L	4.4	<24%	<21%	<18%	<8%	<2%	<21%	<18%	<13%	<5%	<1%

		Ocean		Per	centage o	of Ocean F	Plan Obje	ctive at E	dge of Z	ID by Sce	Percentage of Ocean Plan Objective at Edge of ZID by Scenario <sup>a</sup>							
Constituent	Units						Va	riant										
		Objective	1	2	3	4	5	6	7	8	9	10						
Bis (2-chloroisopropyl) ether	μg/L	1200	<0.1%	<0.1%	<0.1%	<0.03%	<0.01%	<0.1%	<0.1%	<0.05%	<0.02%	<0.004%						
Chlorobenzene	μg/L	570	<0.01%	<0.01%	<0.01%	<0.004%	<0.001%	<0.01%	<0.01%	<0.01%	<0.002%	<0.001%						
Chromium (III)	μg/L	190000	0.001%	0.001%	0.0005%	0.0002%	0.0001%	0.001%	0.001%	0.0004%	0.0001%	0.00003%						
Di-n-butyl phthalate	μg/L	3500	<0.03%	<0.03%	<0.02%	<0.01%	<0.003%	<0.03%	<0.02%	<0.02%	<0.01%	<0.001%						
Dichlorobenzenes	μg/L	5100	0.001%	0.001%	0.001%	0.001%		0.002%		0.001%	0.001%	0.0002%						
Diethyl phthalate	μg/L	33000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%		<0.01%	<0.01%	<0.01%						
Dimethyl phthalate	μg/L	820000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%			<0.01%	<0.01%						
4,6-dinitro-2-	μg/L	220	<2%	<2%	<2%	<1%	<0.2%	<2%	<2%	<1%	<0.5%	<0.1%						
methylphenol																		
2,4-Dinitrophenol c	μg/L	4.0																
Ethylbenzene	μg/L	4100	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%						
Fluoranthene	μg/L	15	0.1%	0.1%	0.1%	0.02%	0.004%	0.1%	0.1%	0.04%	0.01%	0.002%						
Hexachlorocyclopentadiene	μg/L	58	<0.01%	<0.01%	<0.02%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%						
Nitrobenzene	μg/L	4.9	<53%	<45%	<39%	<16%	<3%	<46%	<40%	<28%	<9%	<2%						
Thallium	μg/L	2	0.3%	0.5%	1%	0.5%	0.2%	1%	1%	1%	0.5%	0.2%						
Toluene	μg/L	85000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%						
Tributyltin c	μg/L	0.0014																
1,1,1-Trichloroethane	μg/L	540000	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%						
Objectives for protection								0.404	0.70/	0.404	100/	40/						
Acrylonitrile d	μg/L	0.10	1%	7%	11%	12%	7%	34%	35%	31%	13%	4%						
Aldrin <sup>c</sup>	μg/L	0.000022	-		-			-										
Benzene	μg/L	5.9	<1%	<1%	<1%	<0.4%	<0.1%	<1%	<1%	<1%	<0.2%	<0.1%						
Benzidine <sup>c</sup>	μg/L	0.000069			-	<b>-</b>												
Beryllium <sup>d</sup>	μg/L	0.033	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%						
Bis(2-chloroethyl)ether c	μg/L	0.045																
Bis(2-ethyl- hexyl)phthalate	μg/L	3.5	3%	16%	25%	28%	15%	69%	72%	66%	27%	9%						
Carbon tetrachloride	μg/L	0.90	6%	5%	5%	2%	1%	7%	6%	5%	2%	0.4%						
Chlordane	μg/L	0.000023	6%	23%	35%	37%	20%	94%	97%	88%	37%	12%						
Chlorodibromomethane	μg/L	8.6	1%	1%	1%	0.5%	0.2%	1%	1%	1%	0.4%	0.1%						
Chloroform	μg/L	130	0.1%	0.2%	0.3%	0.4%	0.2%	1%	1%	1%	0.4%	0.1%						
DDT	µg/L	0.00017	1%	5%	8%	9%	5%	22%	23%	21%	9%	3%						
1,4-Dichlorobenzene	μg/L μg/L	18	0.3%	0.3%	0.3%	0.2%	0.1%	1%	0.5%	0.4%	0.2%	0.05%						
		0.0081	0.5%		0.3%	0.276	0.176	170	0.5%	0.4 %	0.270	0.05%						
3,3-Dichlorobenzidine c	μg/L		<0.2%	<0.2%	<0.2%	<0.1%	<0.02%	<0.2%	<0.2%	<0.1%	O 0E0/	<0.01%						
1,2-Dichloroethane	μg/L	28	201	-01	=0/	201	407	00/	=0/	407	<0.05%	0.404						
1,1-Dichloroethylene	μg/L	0.9 6.2	6% 1%	5% 1%	5% 1%	2% 1%	0.3%	6% 2%	5% 2%	4% 2%	1% 1%	0.4%						
Dichlorobromomethane  Dichloromothane	μg/L				0.01%			0.01%	0.01%									
Dichloromethane	μg/L	450	0.01%	0.01%		0.005%	0.002%			0.01%	0.004%	0.001%						
1,3-dichloropropene	μg/L	8.9	1%	1%	1%	0.3%	0.1%	1%	1%	0.5%	0.2%	0.04%						
Dieldrin	μg/L	0.00004	8%	16%	22%	21%	11%	54%	55%	49%	20%	7%						
2,4-Dinitrotoluene	µg/L	2.6	<0.5%	<1%	<1%	<1%	<1%	<0.4%	<1%	<1%	<1%	<0.3%						
1,2-Diphenylhydrazine c	μg/L	0.16																
Halomethanes	μg/L	130	0.04%	0.04%	0.04%	0.03%	0.01%	0.1%	0.1%	0.1%	0.02%	0.01%						
Heptachlor c	μg/L	0.00005																
Heptachlor Epoxide	μg/L	0.00002	1%	3%	5%	5%	3%	12%	13%	12%	5%	2%						
Hexachlorobenzene	μg/L	0.00021	2%	2%	2%	1%	0.3%	3%	3%	2%	1%	0.2%						
Hexachlorobutadiene	μg/L	14	2E-7%	6E-7%	8E-7%	8E-7%	4E-7%	2E-6%	2E-6%	2E-6%	8E-7%	3E-7%						
Hexachloroethane	μg/L	2.5	<42%	<36%	<32%	<14%	<3%	<36%	<32%	<23%	<8%	<1%						
Isophorone	μg/L	730	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%	<0.01%						
N-Nitrosodimethylamine	μg/L	7.3	0.004%	0.01%	0.02%	0.01%	0.01%	0.01%	0.02%	0.02%	0.01%	0.005%						
N-Nitrosodi-N- Propylamine	μg/L	0.38	0.1%	0.2%	0.3%	0.3%	0.1%	0.1%	0.2%	0.3%	0.1%	0.1%						
N-Nitrosodiphenylamine	μg/L	2.5	<42%	<36%	<32%	<14%	<3%	<36%	<32%	<23%	<8%	<1%						
14 Talli 0300ipileriylalilille	µy/∟	2.0	י⊤∠ /0	-00/0	-UZ /0	- I <del>-</del> 70	· U /0	-00/0	-UL /0	-20/0	۰0 /0	`1/0						

		Ocean	Percentage of Ocean Plan Objective at Edge of ZID by Scenario <sup>a</sup>									
Constituent	Units	Plan Objective		Variant								
		Objective	1	2	3	4	5	6	7	8	9	10
PAHs	μg/L	0.0088	2%	3%	4%	4%	2%	14%	14%	12%	5%	1%
PCBs	μg/L	0.000019	46%	61%	70%	57%	26%	146%	145%	126%	51%	16%
TCDD Equivalents d	μg/L	3.9E-09	3%	24%	39%	44%	24%	110%	115%	105%	44%	15%
1,1,2,2- Tetrachloroethane	μg/L	2.3	<2%	<2%	<2%	<1%	<0.3%	<2%	<2%	<2%	<1%	<0.1%
Tetrachloroethylene	μg/L	2.0	<3%	<2%	<2%	<1%	<0.3%	<2%	<2%	<2%	<1%	<0.2%
Toxaphene e	μg/L	2.1E-04	4%	23%	37%	42%	22%	103%	107%	99%	41%	14%
Trichloroethylene	μg/L	27	<0.2%	<0.2%	<0.2%	<0.1%	<0.02%	<0.2%	<0.2%	<0.1%	<0.05%	<0.01%
1,1,2-Trichloroethane	μg/L	9.4	<1%	<1%	<0.5%	<0.2%	<0.1%	<1%	<0.5%	<0.4%	<0.1%	<0.03%
2,4,6-Trichlorophenol c	μg/L	0.29	-		ı		-		_	-		-
Vinyl chloride	μg/L	36	<0.1%	<0.1%	<0.1%	<0.04%	<0.01%	<0.1%	<0.1%	<0.1%	<0.03%	<0.01%

<sup>&</sup>lt;sup>a</sup> Note that if the percentage as determined by using the MRL was less than 0.01 percent, then a minimum value is shown as "<0.01%" (e.g., if the MRL indicated the value was <0.000001%, for simplicity, it is displayed as <0.01%). Also, shading indicates constituent is expected to be greater than 80 percent (orange shading) or exceed (red shading) the ocean plan objective for that discharge scenario.

<sup>c</sup> All observed values from all data sources were below the MRL, and the flow-weighted average of the MRLs is higher than the Ocean Plan objective. No compliance conclusions can be drawn for these constituents.



<sup>&</sup>lt;sup>b</sup> Calculating flow-weighted averages for toxicity (acute and chronic) and radioactivity (gross beta and gross alpha) is not appropriate based the nature of the constituent. These constituents were measured individually for the secondary effluent and GWR concentrate, and these individual concentrations would comply with the Ocean Plan objectives.

<sup>&</sup>lt;sup>d</sup> Acrylonitrile, beryllium and TCDD equivalents represent a special case; they were detected in some source waters, but were also not detected above the MRL in others, and the MRL values are above the Ocean Plan objectives. For these constituents, a value of 0 was assumed when it was not detected in a source water and the MRL was above the Ocean Plan objective. This assumption was made to show there is potential for the constituent to exceed the Ocean Plan objective in some flow scenarios, but there is not enough information to provide a complete compliance determination at this time. When only the detected values were considered, acrylonitrile and beryllium did not exceed the Ocean Plan objective by 80% or more and therefore were not included in Tables 7 through 10.

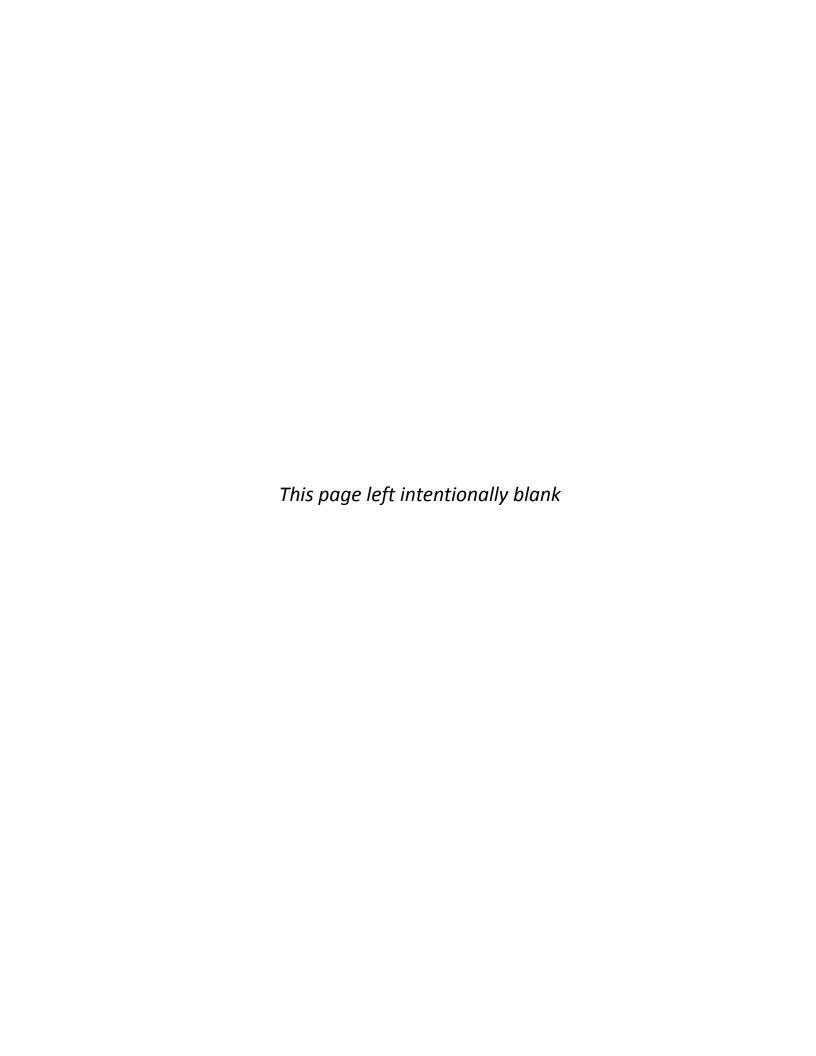
<sup>&</sup>lt;sup>e</sup> Toxaphene was only detected using the low-detection techniques of the CCLEAN program. It was detected once (09/2011) out of 12 samples collected from the secondary effluent from 2010 through 2015, and during the 7-day composite sample from the test slant well.



### **Appendix D**

Roberts, P. J. W, 2017. "Modeling Brine Disposal into Monterey Bay – Supplement." *Technical Memorandum to Environmental Science Associates (ESA)*. 22 September.

# Appendix F: 600 AFY RUWAP Recycled Water Urban Irrigation Use and Implications for CSIP Yields



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#### **MEMORANDUM**

TO: Bob Holden, MRWPCA

DATE: October 23, 2017

Denise Duffy, DD&A

FROM: Andrew Sterbenz, PE JOB #: MRWP.01.14

SUBJECT: 600 AFY RUWAP Recycled Water Urban Irrigation Use and Implications for CSIP

Yields

The purpose of this memorandum is to provide an additional scenario for the future Regional Urban Water Augmentation Project (RUWAP) Recycled Water Project for urban irrigation and its effects on Castroville Seawater Intrusion Project (CSIP) water supplies, with and without the Pure Water Monterey Groundwater Replenishment Project (Proposed Project). Our previous memorandum, <u>Future RUWAP Recycled Water Urban Irrigation Use and Implications for CSIP Yields</u>, dated 9/16/2015 and included as Appendix BB of the Final EIR for the Proposed Project, presented several scenarios for providing water for the RUWAP. This added scenario analysis was requested to reflect the currently proposed initial RUWAP demand of 600 AFY, which differs from the previously analyzed initial demand estimate of 540 AFY.

The new scenario is described as follows:

**600 AFY AWT Demand (600 AFY-AWT) Scenario:** In this scenario, MCWD and MRWPCA agree to share a pipeline as described in the original memorandum, and an initial 600 AFY of recycled water would be produced for existing MCWD customers along the proposed Product Water Pipeline alignment (i.e., the RUWAP pipeline option). Approximately 741 AFY of AWT Influent would be required to produce this water, accounting for the 19% loss of RO concentrate as ocean discharge. The Revised Table 2, below, adds a row reflecting the average monthly influent that would be required at the AWT Facility to produce a net 600 AFY for delivery.

Revised Table 2: RUWAP Urban Recycled Water Use by Treatment and Delivery Scenario (AFY)

Scenario	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Total
RW <sup>1</sup>	81	74	81	156	161	156	161	161	156	81	79	81	1,427
AWT Product <sup>2</sup>	100	91	100	192	199	192	199	199	192	100	97	100	1,761
AWT Influent <sup>1</sup>	66	60	66	126	130	126	130	130	126	66	64	66	1,156
Init-RW 1	31	28	31	59	61	59	61	61	59	31	30	31	540
Init-AWT <sup>2</sup>	38	35	38	73	75	73	75	75	73	38	37	38	666
600 AFY-RW <sup>1</sup>	34	31	34	65	68	65	68	68	65	34	33	34	600
600 AFY-AWT <sup>2</sup>	42	38	42	81	84	81	84	84	81	42	41	42	741

NOTES:

- 1. Values reflect urban recycled water deliveries.
- 2. Values reflect influent supply to the AWT Facility

The 600 AFY scenario was modeled using the same assumptions as in the previous analysis<sup>1</sup>. The resulting project yields under a normal water year building a drought reserve, under a normal water year with a full drought reserve and under a drought year starting with a full reserve are presented in Tables 8A, 8B and 8C, respectively (attached). Table 3 was then modified (below) to present the results of the additional scenario in the same context as the earlier analysis. As can be seen, the Proposed Project in conjunction with the RUWAP use provides a smaller benefit to the Castroville Seawater Intrusion Project (CSIP) than the Proposed Project without the RUWAP demand. However, both scenarios with the Proposed Project provide a significant increase in recycled water for the CSIP compared to the current condition.

Modified Table 3. Estimated Annual Recycled Water Yields Under Various Scenarios of MCWD

Demand and Pipelines<sup>2</sup>

	Existing	-	ed Project v MCWD Use	vith No	Shared Pipeline Scenario		
			IVICWD USE		600 AFY MCWD Use		
Year Type	SVRP to CSIP	AWT to SGB (injection amount)	MCWD	SVRP to CSIP	AWT to MCWD	SVRP to CSIP	
April to September							
Normal/wet building reserve	10 210	1,755	0	14,160	399	13,670	
Normal/wet reserve full	10,310	1,755	0	13,620	399	13,140	
Drought year use reserve for CSIP	10,460	855	0	14,560	399	14,060	
Total Annual							
Normal/wet building reserve	12 000	3,700	0	18,410	600	17,930	
Normal/wet reserve full	13,000	3,500	0	17,880	600	17,390	
Drought year use reserve for CSIP	15,470	2,500	0	21,200	600	20,620	

<sup>&</sup>lt;sup>2</sup> Updating the analysis to reflect the final water rights permits (Blanco Drain and Reclamation Ditch, with Tembladero Slough not issued) and more current municipal wastewater inflow data, the annual flow totals to CSIP become:

Year Type	Proposed Project without MCWD	Proposed Project with MCWD
Normal/wet building reserve	16,516	15,936
Normal/wet reserve full	16,156	15,936
Drought year using reserve	17,694	17,030

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<sup>&</sup>lt;sup>1</sup> The previous analysis in 2015 assumed the surface water diversions (Blanco Drain, Reclamation Ditch and Tembladero Slough) were available at the volumes in the diversion permit applications.

#### References:

Memorandum: <u>Future RUWAP Recycled Water Urban Irrigation Use and Implications for CSIP Yields</u>, dated 9/16/2015, prepared by Schaaf & Wheeler

#### Attachments:

- Table 8A, Source Water Analysis, Diversion Pattern for a Normal Year Building a Drought Reserve, 600 AFY RUWAP Demand as AWT Product
- Table 8B, Source Water Analysis, Diversion Pattern for a Normal Year with a Full Reserve, 600 AFY RUWAP Demand as AWT Product
- Table 8C, Source Water Analysis, Diversion Pattern for a Drought Year starting with a Full Reserve, 600 AFY RUWAP Demand as AWT Product

Table 8A: Source Water Analysis for the Pure Water Monterey Groundwater Replenishment Project

Diversion Pattern for a Normal Water Year Building a Drought Reserve, 600 AFY RUWAP Demand as AWT Product

						1	0/3/2017
Existing RTP Inflows (Average 2009 to 2013) 1,798 1,678 1,867 1,796 1,850 1,7	<u>June</u> <u>July</u>	Aug	<u>Sep</u>	<u>Oct</u>	Nov	<u>Dec</u>	Total
	799 1,893	1,888	1,813	1,844	1,762	1,776	21,764
New Source Water							
City of Salinas							
_	391 435	444	367	410	329	223	3,732
Agricultural Wash Water (AWW) to Ponds <sup>3</sup> 156 158 201 0 0	0 0	0	0	410	329	223	1,477
•	391 435	444	367	0	0	0	2,255
2 Salinas Urban Storm Water Runoff <sup>4</sup> 52 41 34 16 2 <i>Urban runoff to ponds</i> 52 41 34 0 0	0 0	0 <i>0</i>	2 0	8 <i>8</i>	23 23	47 47	225 205
Urban runoff to RTP       0       0       0       16       2	0 0	0	2	8 0	23 0	47 0	205
3 Rainfall (on SIWTF, 121 acre pond area) 5 26 24 21 11 3	1 0	0	2	6	14	24	132
	(52)	Ü	-	(28)	(15)	(12)	(251)
	138)			(143)	(138)	(143)	(1,257)
6 SIWTF pond storage balance <sup>8</sup> 684 763 847 647 362	0 0	0	0	253	466	605	(1,237)
	172 0	0	0	0	0	0	304
	563 435	444	369	0	0	0	2,579
Water Rights Applications to SWRCB							
9 Blanco Drain <sup>9</sup> 0 0 0 252 225 2	274 277	244	184	0	0	0	1,456
10 Reclamation Ditch at Davis Road <sup>10</sup> 0 0 162 97 1	132 129	121	80	0	0	0	721
11 Tembladero Slough at Castroville <sup>11</sup> 0 0 154 145	67 66	62	41	0	0	0	535
12 City of Monterey - Diversion at Lake El Estero 0 0 0 0	0 0	0	0	0	0	0	0
13 Subtotal New Waters Available 0 0 0 923 880 1,0	036 907	871	674	0	0	0	5,291
Total Projected Water Supply 1,798 1,678 1,867 2,719 2,730 2,8	835 2,800	2,759	2,487	1,844	1,762	1,776	27,055
<del></del>	June July	Aug	<u>Sep</u>	<u>Oct</u>	Nov	<u>Dec</u>	Tota
	750 1,866	1,854	1,698	984	448	18	12,955
	606 519 <b>356 2,385</b>	504 <b>2,358</b>	300 <b>1,998</b>	75 <b>1,059</b>	233 <b>681</b>	352 <b>370</b>	4,272 <b>17,227</b>
101AL CSIF Delitatio 401 034 1,030 1,766 2,067 2,3	2,365	2,330	1,330	1,055	001	3/0	17,227
	355 367	367	355	367	355	367	4,320
16 FEEDWATER TO ESTABLISH CSIP AREA DROUGHT RESERVE							
(200 AFY AWTF PRODUCT WATER) 14 42 38 42				42	41	42	248
17 FEEDWATER TO AWT FOR MCWD RUWAP <sup>18</sup> 42 38 42 81 84	81 84	84	81	42	41	42	741
18 TOTAL TO GWR ADVANCED WATER TREATMENT FACILITY 451 407 451 436 451 4	436 451	451	436	451	437	451	5,309
Total Projected Water Demand 912 1,062 1,481 2,224 2,537 2,7	792 2,836	2,808	2,434	1,510	1,118	821	22,536
		_				_	
	June July	Aug	Sep	<u>Oct</u>	Nov	<u>Dec</u>	<u>Total</u>
12	718 1,810 681 540	1,804 504	1,732 319	1,059 0	681 0	370 0	14,801
	399 2,350	2,308	2,051	1,059	<b>681</b>	3 <b>70</b>	3,125 <b>17,926</b>
Net CSIP Increase	_,	_,,,,,	_,,	_,,,,,	001	0.0	4,971
22 Surface waters at RTP to AWT 0 0 0 0 0	0 0	0	0	0	0	0	0
23 Secondary effluent to AWT 409 369 409 0 0	0 0	0	0	409	396	409	2,401
·	355 367	367	355	0	0	0	2,166
	81 84	84	81	42	41	42	741
26 Feedwater to AWT 451 407 451 436 451 4	436 451	451	436	451	437	451	5,308
Subtotal- all waters (including secondary effluent) 912 1,062 1,481 2,719 2,730 2,8	835 2,800	2,759	2,487	1,510	1,118	821	23,234
							0 000
(2009-2013) <sup>15</sup> 1,785 1,219 1,141 420 88	49 27	34	114	859	1,314	1,759	0,009
(2009-2013) <sup>15</sup> 1,785 1,219 1,141 420 88 28 WASTE WATER EFFLUENT TO OCEAN OUTFALL WITH PROPOSED	49 27	34	114	859	1,314	1,759	8,809
(2009-2013) <sup>15</sup> 1,785 1,219 1,141 420 88 28 WASTE WATER EFFLUENT TO OCEAN OUTFALL WITH PROPOSED DIVERSIONS TO CSIP/AWT/RUWAP <sup>16</sup> 885 616 386 0 0	<ul><li>49</li><li>27</li><li>0</li><li>0</li></ul>	0	0	333	645	1,759 955	3,821
28 WASTE WATER EFFLUENT TO OCEAN OUTFALL WITH PROPOSED DIVERSIONS TO CSIP/AWT/RUWAP $^{16}$ 885 616 386 0 0 $^{29}$ NEW SUPPLIES IN EXCESS OF AWT DEMANDS FOR GWR $^{17}$ (409) (369) (409) 568 513 6					·		

#### Notes

- 1 Presumes all facilities associated with diversions are completed.
- 2 Table 2-1, p. 5, Schaaf & Wheeler Consulting Engineers. Revised Draft, Groundwater Replenishment Project, Salinas River Inflow Impacts, Prepared for Denise Duffy & Associates, February 2015.
- 3 Volume of effluent from City of Salinas agricultural wash water to be directed into ponds 1,2,3, and the aeration pond for storage.
- 4 Average monthly flow from Revised Draft, Groundwater Replenishment Project, Salinas River Inflow Impacts, Prepared for Denise Duffy & Associates, February 2015.
- 5 Rainfall from Revised Draft, Groundwater Replenishment Project, Salinas River Inflow Impacts, Prepared for Denise Duffy & Associates, February 2015. Pond area presumed to be Ponds 1,2, 3 + Aeration lagoon. No rainfall/evaporation or storage assigned to drying beds.
- 6 Table 3, Todd Groundwater, Draft Memorandum, Pure Water Monterey Groundwater Replenishment Project: Impacts of Changes in Percolation at the Salinas Industrial Wastewater Treatment Facility on Groundwater and the Salinas River, February 11, 2015.
- 7 Table 4, Ibid.
- 8 Ponds 1,2,3 and aeration basin hold up to 1,065 acre-feet (one foot of freeboard). If flow to ponds would exceed the maximum volume, it is presumed that excess flow can be diverted to the RIBs or drying beds or flow can be diverted to the RTP. Presume that pond storage goes to zero sometime during the year (shown here starting in July).
- 9 Table 4, Ibid. Also confirmed in MPWMD Industrial Ponds Percolation and Evaporation Technical Memorandum 2015-01, July 2015.
- 10 Max. diversion = 6 cfs. See REVISED DRAFT RECLAMATION DITCH YIELD STUDY, Schaaf and Wheeler, March 2015. Note that flow figures shown here are a combination of flow estimates in the S&W analysis made for the 2 cfs instream requirement Jan-May and 1 cfs instream requirement for June-Dec.
   11 Max. diversion = 3 cfs. See REVISED DRAFT RECLAMATION DITCH YIELD STUDY, Schaaf and Wheeler, March 2015. Figures shown here are the difference between the combined Davis Road/TS diversion with
- Seasonal Bypass. This presumes the preference is to remove flow at Davis Road first, rather than bypass flow to Tembaldero Slough.

  12 Includes secondary effluent wastewater currently used to produce recycled water at the Salinas Valley Reclamation Project (SVRP), and additional amounts which may be used during periods of low demand (<
- 12 Includes secondary effluent wastewater currently used to produce recycled water at the Salinas Valley Reclamation Project (SVRP), and additional amounts which may be used during periods of low demand (<5 mgd) with the proposed improvements to the SVRP.
- 13 New source waters not used by AWT will be available to SVRP for CSIP.
- 14 A drought reserve of up to 1,000 AF would be created over five years by producing 200 AFY additional product water from the GWR Project AWTF during winter months and storing the water in the Seaside Basin. This would establish a "water bank" that the CSIP can draw on in droughts. The drought reserve would allow flow at the RTP for the GWR Project to be temporarily reduced during critically dry periods, thus freeing up more of the newly available inflows to the RTP to be sent to the CSIP area. Extraction from the Seaside Basin would continue at the average rate to supply the Monterey Peninsula.
- 15 Average monthly RTP discharge, 2009-2013 (reported by MRWPCA).
- 16 Secondary treated municpal effluent not used for SVRP or the AWT.
- 17 Excess is calculated as Line 13 minus Lines 15 &~16
- ${\small 18}\ \ RUWAP\ supply\ comes\ from\ existing\ RTP\ inflows,\ demands\ reflect\ existing\ urban\ irrigation\ customers\ along\ trunk\ main.$

## Table 8B: Source Water Analysis for the Pure Water Monterey Groundwater Replenishment Project Diversion Pattern for a Normal Water Year with a Full Reserve, 600 AFY RUWAP Demand as AWT Product

All facilities built 1- average water year conditions - all flows in acre-f	eet											1	0/3/2017
<u>SOURCES</u>	<u>Jan</u>	<u>Feb</u>	Mar	<u>Apr</u>	May	<u>June</u>	<u>July</u>	Aug	<u>Sep</u>	<u>Oct</u>	Nov	Dec	<u>Total</u>
Existing RTP Inflows (Average 2009 to 2013)	1,798	1,678	1,867	1,796	1,850	1,799	1,893	1,888	1,813	1,844	1,762	1,776	21,764
New Source Water													
City of Salinas													
1 Salinas Agricultural Wash Water <sup>2</sup>	156	158	201	307	311	391	435	444	367	410	329	223	3,732
Agricultural Wash Water (AWW) to Ponds <sup>3</sup>	156	158	201	0	0	0	0	0	0	410	329	223	1,477
AWW directly to RTP	0	0	0	307	311	391	435	444	367	0	0	0	2,255
2 Salinas Urban Storm Water Runoff <sup>4</sup>	52	41	34	16	2	0	0	0	2	8	23	47	225
Urban runoff to ponds	52	41	34	0	0	0	0	0	0	8	23	47	205
Urban runoff to RTP  3 Rainfall (on SIWTF, 121 acre pond area) 5	0	0	<i>0</i> 21	<i>16</i> 11	2 3	<i>0</i> 1	<i>0</i> 0	<i>0</i> 0	<i>2</i> 2	<i>0</i> 6	0	<i>0</i> 24	<i>20</i>
4 Evaporation (from SIWTF, 121 acre pond area) <sup>6</sup>	26 (12)	24 (16)	(29)	(41)	3 (46)	(52)	U	U	2	(28)	14 (15)	(12)	132 (251)
5 Percolation <sup>7</sup>	(12)	(10)	(143)	(138)	(143)	(138)				(143)	(138)	(143)	(1,257)
6 SIWTF pond storage balance <sup>8</sup>	684	763	(143) 847	647	362	(138)	0	0	0	253	466	605	(1,237)
7 Recovery of flow from SIWTF storage ponds to RTP	084	703	0	32	100	172	0	0	0	255	0	003	304
8 AWW and Salinas Runoff to RTP	0	0	0	355	413	563	435	444	369	0	0	0	2,579
Water Rights Applications to SWRCB													
9 Blanco Drain <sup>9</sup>	0	0	0	252	225	274	277	244	184	0	0	0	1,456
10 Reclamation Ditch at Davis Road <sup>10</sup>	0	0	0	162	97	132	129	121	80	0	0	0	721
11 Tembladero Slough at Castroville <sup>11</sup>	0	0	0	0	0	0	0	0	0	0	0	0	0
12 City of Monterey - Diversion at Lake El Estero	0	0	0	0	0	0	0	0	0	0	0	0	0
13 Subtotal New Waters Available	0	0	0	769	735	969	841	809	633	0	0	0	4,756
Total Projected Water Supply	1,798	1,678	1,867	2,565	2,585	2,768	2,734	2,697	2,446	1,844	1,762	1,776	26,520
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DEMANDS	<u>Jan</u>	<u>Feb</u>	<u>Mar</u>	Apr	May	<u>June</u>	<u>July</u>	<u>Aug</u>	<u>Sep</u>	Oct	Nov	<u>Dec</u>	<u>Total</u>
Average SVRP deliveries to CSIP (2009-2013)  14 FIVE YEAR AVERAGE CSIP AREA WELL WATER USE (2009-2013)	13 448	459 195	726 304	1,376 412	1,763 324	1,750 606	1,866 519	1,854 504	1,698 300	984 75	448 233	18 352	12,955 4,272
TOTAL CSIP Demand	448 461	654	1,030	1,788	2,087	2,356	<b>2,385</b>	2,358	1,998	1,059	681	370	4,272 <b>17,227</b>
	.02		_,,	_,,	_,00;	_,000	_,000	_,000	_,,,,,	_,000	001	0.0	_,,,
15 FEEDWATER AMOUNT AT RTP TO GWR PROJECT AWTF	367	331	367	355	367	355	367	367	355	367	355	367	4,320
16 FEEDWATER TO ESTABLISH CSIP AREA DROUGHT RESERVE													
(200 AFY AWTF PRODUCT WATER) 14	0	0	0							0	0	0	0
17 FEEDWATER TO AWT FOR MCWD RUWAP <sup>18</sup>	42	38	42	81	84	81	84	84	81	42	41	42	741
18 TOTAL TO GWR ADVANCED WATER TREATMENT FACILITY	409	369	409	436	451	436	451	451	436	409	396	409	5,061
Total Projected Water Demand	870	1,024	1,439	2,224	2,537	2,792	2,836	2,808	2,434	1,468	1,077	779	22,288
•							-						
Use of Source Water	<u>Jan</u>	<u>Feb</u>	Mar	<u>Apr</u>	May	June	July	Λιισ	Sep	Oct	Nov	Dec	Total
19 Secondary effluent to SVRP for CSIP <sup>12</sup>	461	654	1,030	1,715	1,767	1,718	1,810	<u>Aug</u> 1,804	1,732	1,059	681	370	14,801
20 New sources available to CSIP <sup>13</sup>	0	0	0	414	368	614	474	442	278	0	0	0	2,590
21 Total Supply to CSIP	461	654	1,030	2,129	2,135	2,332	2,284	2,246	2,010	1,059	681	370	17,391
Net CSIP Increase													4,436
22 Surface waters at RTP to AWT	0	0	0	0	0	0	0	0	0	0	0	0	0
23 Secondary effluent to AWT	367	331	367	0	0	0	0	0	0	367	355	367	2,154
24 AWW and Salinas urban runoff to AWT	0	0	0	355	367	355	367	367	355	0	0	0	2,166
25 Secondary effluent to AWT for MCWD RUWAP	42	38	42	81	84	81	84	84	81	42	41	42	741
26 Feedwater to AWT	409	369	409	436	451	436	451	451	436	409	396	409	5,061
Subtotal- all waters (including secondary effluent)	870	1,024	1,439	2,565	2,585	2,768	2,734	2,697	2,446	1,468	1,077	779	22,452
27 FIVE YEAR AVERAGE WASTE WATER FEEL HEAT TO OCEAN OUTCOME													
27 FIVE YEAR AVERAGE WASTE WATER EFFLUENT TO OCEAN OUTFALL (2009-2013) <sup>15</sup>	1,785	1,219	1,141	420	88	49	27	34	114	859	1,314	1,759	8,809
(2009-2013) 28 WASTE WATER EFFLUENT TO OCEAN OUTFALL WITH PROPOSED	1,765	1,219	1,141	420	ŏŏ	49	21	54	114	639	1,314	1,/39	0,809
DIVERSIONS TO CSIP/AWT/RUWAP 16	928	654	428	0	0	0	0	0	0	375	685	998	4,068
29 NEW SUPPLIES IN EXCESS OF AWT DEMANDS FOR GWR <sup>17</sup>	(367)	(331)	(367)	414	368	614	474	442	278	(367)	(355)	(367)	436
30 AWT BRINE TO OCEAN OUTFALL	78	70	78	83	86	83	86	86	83	78	75	78	962

#### Notes

- 1 Presumes all facilities associated with diversions are completed.
- 2 Table 2-1, p. 5, Schaaf & Wheeler Consulting Engineers. Revised Draft, Groundwater Replenishment Project, Salinas River Inflow Impacts, Prepared for Denise Duffy & Associates, February 2015.
- 3 Volume of effluent from City of Salinas agricultural wash water to be directed into ponds 1,2,3, and the aeration pond for storage.
- 4 Average monthly flow from Revised Draft, Groundwater Replenishment Project, Salinas River Inflow Impacts, Prepared for Denise Duffy & Associates, February 2015.
- 5 Rainfall from Revised Draft, Groundwater Replenishment Project, Salinas River Inflow Impacts, Prepared for Denise Duffy & Associates, February 2015. Pond area presumed to be Ponds 1,2, 3 + Aeration lagoon. No rainfall/evaporation or storage assigned to drying beds.
- 6 Table 3, Todd Groundwater, Draft Memorandum, Pure Water Monterey Groundwater Replenishment Project: Impacts of Changes in Percolation at the Salinas Industrial Wastewater Treatment Facility on Groundwater and the Salinas River, February 11, 2015.
- 7 Table 4, Ibid.
- 8 Ponds 1,2,3 and aeration basin hold up to 1,065 acre-feet (one foot of freeboard). If flow to ponds would exceed the maximum volume, it is presumed that excess flow can be diverted to the RIBs or drying beds or flow can be diverted to the RTP. Presume that pond storage goes to zero sometime during the year (shown here starting in July).
- 9 Table 4, Ibid. Also confirmed in MPWMD Industrial Ponds Percolation and Evaporation Technical Memorandum 2015-01, July 2015.
- 10 Max. diversion = 6 cfs. See REVISED DRAFT RECLAMATION DITCH YIELD STUDY, Schaaf and Wheeler, March 2015. Note that flow figures shown here are a combination of flow estimates in the S&W analysis made for the 2 cfs instream requirement Jan-May and 1 cfs instream requirement for June-Dec.
   11 Max. diversion = 3 cfs. See REVISED DRAFT RECLAMATION DITCH YIELD STUDY, Schaaf and Wheeler, March 2015. Figures shown here are the difference between the combined Davis Road/TS diversion with
- Seasonal Bypass. This presumes the preference is to remove flow at Davis Road first, rather than bypass flow to Tembaldero Slough.

  12 Includes secondary effluent wastewater currently used to produce recycled water at the Salinas Valley Reclamation Project (SVRP), and additional amounts which may be used during periods of low demand (<
- 12 Includes secondary effluent wastewater currently used to produce recycled water at the Salinas Valley Reclamation Project (SVRP), and additional amounts which may be used during periods of low demand (<5 mgd) with the proposed improvements to the SVRP.
- 13 New source waters not used by AWT will be available to SVRP for CSIP.
- 14 A drought reserve of up to 1,000 AF would be created over five years by producing 200 AFY additional product water from the GWR Project AWTF during winter months and storing the water in the Seaside Basin. This would establish a "water bank" that the CSIP can draw on in droughts. The drought reserve would allow flow at the RTP for the GWR Project to be temporarily reduced during critically dry periods, thus freeing up more of the newly available inflows to the RTP to be sent to the CSIP area. Extraction from the Seaside Basin would continue at the average rate to supply the Monterey Peninsula.
- 15 Average monthly RTP discharge, 2009-2013 (reported by MRWPCA).
- 16 Secondary treated municpal effluent not used for SVRP or the AWT.
- 17 Excess is calculated as Line 13 minus Lines 15 &~16
- ${\small 18}\ \ RUWAP\ supply\ comes\ from\ existing\ RTP\ inflows,\ demands\ reflect\ existing\ urban\ irrigation\ customers\ along\ trunk\ main.$

Table 8C: Source Water Analysis for the Pure Water Monterey Groundwater Replenishment Project

Diversion Pattern for a Drought Year, Starting with a Full Drought Reserve, 600 AFY RUWAP Demand as AWT Product

All facilities built 1- average water year conditions - all flows in acre-	feet											1	0/3/2017
<u>SOURCES</u>	<u>Jan</u>	<u>Feb</u>	<u>Mar</u>	<u>Apr</u>	May	<u>June</u>	<u>July</u>	Aug	<u>Sep</u>	<u>Oct</u>	Nov	<u>Dec</u>	Total
Minimum Year RTP Inflows (2013)	1,725	1,494	1,645	1,657	1,722	1,675	1,748	1,773	1,715	1,690	1,634	1,612	20,090
Navy Carriag Waker													
New Source Water City of Salinas													
1 Salinas Agricultural Wash Water <sup>2</sup>	156	158	201	307	311	391	435	444	367	410	329	223	3,732
Agricultural Wash Water (AWW) to Ponds <sup>3</sup>	156	158	201	0	0	0	0	0	0	410	329	223	1,477
AWW directly to RTP	0	0	0	307	311	391	435	444	367	0	0	0	2,255
2 Salinas Urban Storm Water Runoff <sup>4</sup>	17	14	11	5	1	0	0	0	1	3	8	16	76
Urban runoff to ponds	17	14	11	0	0	0	0	0	0	3	8	16	69
Urban runoff to RTP	0	0	0	5	1	0	0	0	1	0	0	0	7
3 Rainfall (on SIWTF, 121 acre pond area) <sup>5</sup>	11	6	4	3	0	0	0	0	1	2	5	4	36
4 Evaporation (from SIWTF, 121 acre pond area) <sup>6</sup>	(12)	(16)	(29)	(41)	(46)	(52)				(28)	(15)	(12)	(251)
5 Percolation <sup>7</sup>	(143)	(129)	(143)	(138)	(143)	(138)				(143)	(138)	(143)	(1,257)
6 SIWTF pond storage balance <sup>8</sup>	550	584	628	452	163	(27)	0	0	0	245	433	521	
7 Recovery of flow from SIWTF storage ponds to RTP	0	0	0	0	100	0	0	0	0	0	0	0	100
8 AWW and Salinas Runoff to RTP	0	0	0	312	412	391	435	444	368	0	0	0	2,362
Water Rights Applications to SWRCB													
9 Blanco Drain <sup>9</sup>	0	0	246	252	225	274	277	244	184	168	133	0	2,003
10 Reclamation Ditch at Davis Road <sup>10</sup>	0	0	165	162	97	132	129	121	80	87	98	0	1,071
11 Tembladero Slough at Castroville 11	0	0	142	154	145	67	66	62	41	45	50	0	772
12 City of Monterey - Diversion at Lake El Estero 13 Subtotal New Waters Available	0 <b>0</b>	0 <b>0</b>	5 <b>53</b>	8 <b>80</b>	8 <b>79</b>	864	907	871	673	300	2 <b>81</b>	0 <b>0</b>	6,208
13 Subtotal New Waters Available	U	U	333	880	8/9	804	907	8/1	6/3	300	201	U	0,208
Total Projected Water Supply	1,725	1,494	2,198	2,537	2,601	2,539	2,655	2,644	2,388	1,990	1,915	1,612	26,297
DEMANDS  Mac Very SVRD delication to CSID (2012)	<u>Jan</u>	<u>Feb</u>	Mar 4 550	<u>Apr</u>	May	June 1 675	July 4.706	Aug	<u>Sep</u>	Oct	<u>Nov</u>	<u>Dec</u>	<u>Total</u>
Max Year SVRP deliveries to CSIP (2013)  14 PEAK CSIP AREA WELL WATER USE (10/2013-09/2014)	0 509	692 9	1,558 221	1,669 242	1,799 1,197	1,675 1,261	1,786 1,303	1,803 1,025	1,725 453	1,548 165	1,127 35	88 730	15,469 7,150
TOTAL CSIP Demand	<b>509</b>	<b>701</b>	1,779	1,911	2,996	2,936	3,089	2,828	2,178	1,713	1,162	818	<b>22,619</b>
TOTAL CON Demand	303	701	1,773	1,511	2,330	2,330	3,003	2,020	2,170	1,713	1,102	010	22,013
15 FEEDWATER AMOUNT AT RTP TO GWR PROJECT AWTF	367	331	367	133	137	133	137	137	133	367	355	367	2,963
16 FEEDWATER TO ESTABLISH CSIP AREA DROUGHT RESERVE													
(200 AFY AWTF PRODUCT WATER) 14	0	0	0							0	0	0	0
17 FEEDWATER TO AWT FOR MCWD RUWAP <sup>18</sup>	42	38	42	81	84	81	84	84	81	42	41	42	741
18 TOTAL TO GWR ADVANCED WATER TREATMENT FACILITY	409	369	409	213	221	213	221	221	213	409	396	409	3,704
Total Projected Water Demand	918	1,070	2,188	2,124	3,217	3,150	3,309	3,049	2,392	2,122	1,558	1,227	26,324
Use of Source Water	<u>Jan</u>	<u>Feb</u>	<u>Mar</u>	<u>Apr</u>	May	<u>June</u>	<u>July</u>	Aug	<u>Sep</u>	<u>Oct</u>	Nov	<u>Dec</u>	<u>Total</u>
19 Secondary effluent to SVRP for CSIP 12	509	701	1,603	1,576	1,638	1,594	1,665	1,690	1,634	1,580	1,162	818	16,170
New sources available to CSIP <sup>13</sup>	0	0	186	747	742	731	770	734	540	0	0	0	4,451
21 Total Supply to CSIP	509	701	1,789	2,323	2,380	2,326	2,435	2,424	2,175	1,580	1,162	818	20,620
Net CSIP Increase													5,151
22 Surface waters at RTP to AWT	0	0	367	0	0	0	0	0	0	300	281	0	948
23 Secondary effluent to AWT	367	331	0	0	0	0	0	0	0	67	74	367	1,206
24 AWW and Salinas urban runoff to AWT	0	0	0	133	137	133	137	137	133	0	0	0	809
25 Secondary effluent to AWT for MCWD RUWAP	42	38	42	81	84	81	84	84	81	42	41	42	741
26 Feedwater to AWT	409	369	409	213	221	213	221	221	213	409	396	409	3,704
Subtotal- all waters (including secondary effluent)	918	1,070	2,198	2,537	2,601	2,539	2,655	2,644	2,388	1,990	1,558	1,227	24,324
27 DRY YEAR WASTEWATER EFFLUENT TO OCEAN OUTFALL (2013) 15													
2 2. IN W. STEWNER EN LOUIT TO OCEAN COTTALE (2013)	1,725	802	87	0	0	0	0	0	0	142	507	1,607	4,870
28 WASTE WATER EFFLUENT TO OCEAN OUTFALL WITH PROPOSED	-,0		<del>= 1</del>	Ü	ŭ	Ü	ŭ	ŭ	ŭ	- · <b>-</b>		/==:	.,=.
DIVERSIONS TO CSIP/AWT/RUWAP 16	807	424	(0)	0	0	0	0	0	0	(0)	357	385	1,973
29 NEW SUPPLIES IN EXCESS OF AWT DEMANDS FOR GWR <sup>17</sup>	(367)	(331)	186	747	742	731	770	734	540	(67)	(74)	(367)	3,244
30 AWT BRINE TO OCEAN OUTFALL	78	70	78	41	42	41	42	42	41	78	75	78	704

#### Notes

- 1 Presumes all facilities associated with diversions are completed.
- 2 Table 2-1, p. 5, Schaaf & Wheeler Consulting Engineers. Revised Draft, Groundwater Replenishment Project, Salinas River Inflow Impacts, Prepared for Denise Duffy & Associates, February 2015.
- 3 Volume of effluent from City of Salinas agricultural wash water to be directed into ponds 1,2,3, and the aeration pond for storage.
- 4 Assume dry year at 1/3 the average monthly values from Revised Draft, Groundwater Replenishment Project, Salinas River Inflow Impacts, Prepared for Denise Duffy & Associates, February 2015.
- 5 Rainfall from Revised Draft, Groundwater Replenishment Project, Salinas River Inflow Impacts, Prepared for Denise Duffy & Associates, February 2015. Pond area presumed to be Ponds 1,2, 3 + Aeration lagoon. No rainfall/evaporation or storage assigned to drying beds.
- 6 Table 3, Todd Groundwater, Draft Memorandum, Pure Water Monterey Groundwater Replenishment Project: Impacts of Changes in Percolation at the Salinas Industrial Wastewater Treatment Facility on Groundwater and the Salinas River, February 11, 2015.
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- 8 Ponds 1,2,3 and aeration basin hold up to 1,065 acre-feet (one foot of freeboard). If flow to ponds would exceed the maximum volume, it is presumed that excess flow can be diverted to the RIBs or drying beds or flow can be diverted to the RTP. Presume that pond storage goes to zero sometime during the year (shown here starting in July).
- 9 Table 4, Ibid. Also confirmed in MPWMD Industrial Ponds Percolation and Evaporation Technical Memorandum 2015-01, July 2015.
- 10 Max. diversion = 6 cfs. See REVISED DRAFT RECLAMATION DITCH YIELD STUDY, Schaaf and Wheeler, March 2015. Note that flow figures shown here are a combination of flow estimates in the S&W analysis made for the 2 cfs instream requirement Jan-May and 1 cfs instream requirement for June-Dec.
- 11 Max. diversion = 3 cfs. See REVISED DRAFT RECLAMATION DITCH YIELD STUDY, Schaaf and Wheeler, March 2015. Figures shown here are the difference between the combined Davis Road/TS diversion with Seasonal Bypass. This presumes the preference is to remove flow at Davis Road first, rather than bypass flow to Tembaldero Slough.

  12 Includes secondary effluent wastewater currently used to produce recycled water at the Salinas Valley Reclamation Project (SVRP), and additional amounts which may be used during periods of low demand (<
- 12 Includes secondary effluent wastewater currently used to produce recycled water at the Salinas Valley Reclamation Project (SVRP), and additional amounts which may be used during periods of low demand (<5 mgd) with the proposed improvements to the SVRP.
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- 14 A drought reserve of up to 1,000 AF would be created over five years by producing 200 AFY additional product water from the GWR Project AWTF during winter months and storing the water in the Seaside Basin. This would establish a "water bank" that the CSIP can draw on in droughts. The drought reserve would allow flow at the RTP for the GWR Project to be temporarily reduced during critically dry periods, thus freeing up more of the newly available inflows to the RTP to be sent to the CSIP area. Extraction from the Seaside Basin would continue at the average rate to supply the Monterey Peninsula.
- 15 Monthly RTP discharge during critically dry year (2013), reported by MRWPCA
- 16 Secondary treated municipal effluent not used for SVRP or the AWT.
- 17 Excess is calculated as Line 13 minus Lines 15 &~16
- ${\small 18}\ \ RUWAP\ supply\ comes\ from\ existing\ RTP\ inflows,\ demands\ reflect\ existing\ urban\ irrigation\ customers\ along\ trunk\ main.$